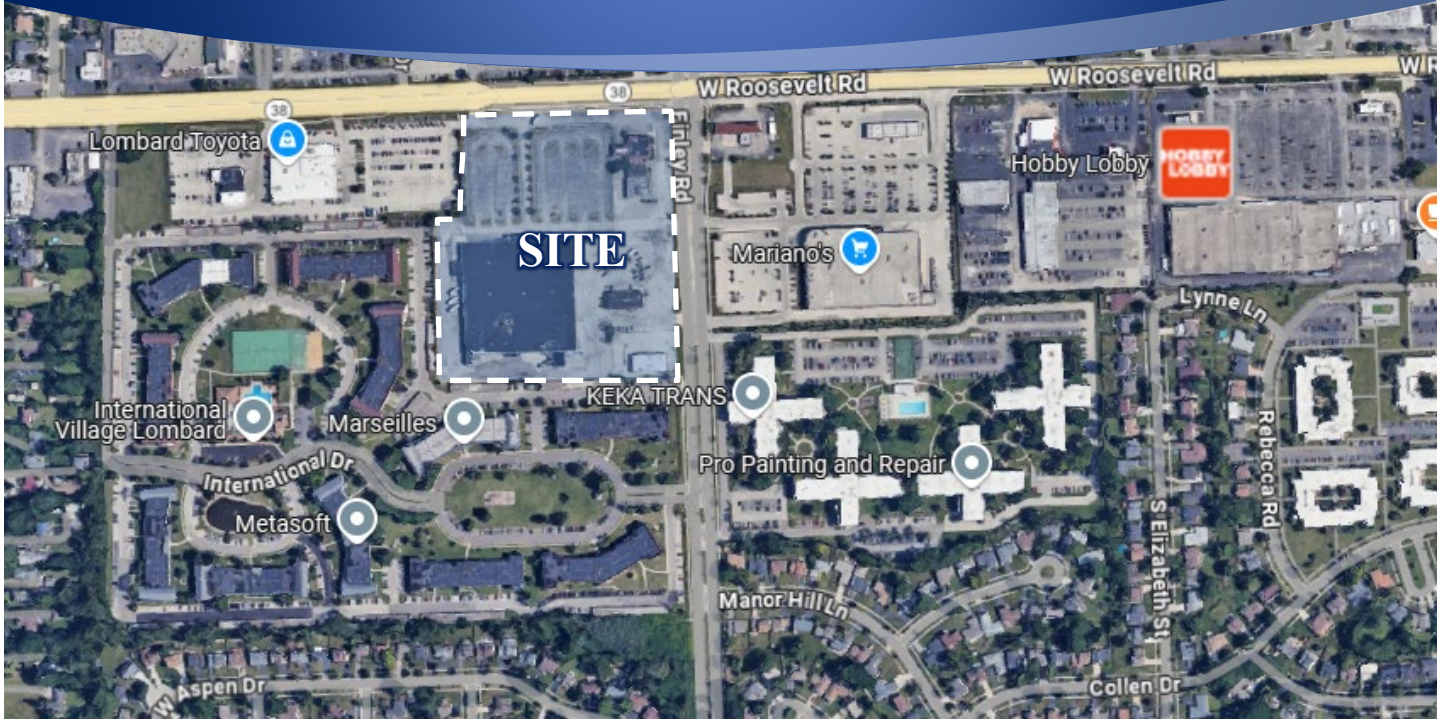


Traffic Impact Study

Burj Plaza Redevelopment

Lombard, Illinois



Prepared For:



June 12, 2025

1. Introduction

This report summarizes the methodologies, results, and findings of a traffic impact study conducted by Kenig, Lindgren, O'Hara, Aboona, Inc. (KLOA, Inc.) for the redevelopment of Burj Plaza located in Lombard, Illinois. The site is located in the southwest quadrant of the intersection of Roosevelt Road with Finley Road. As proposed, the site will be redeveloped to contain the following uses:

- 5,000 square-foot day care center with a maximum population of 72 children
- 3,565 square-foot fast food restaurant with drive-through
- 1,280 square-foot candy shop
- 4,782 square-foot smoking lounge
- 3,624 square feet of retail/service uses
- 2,978 square feet of personal care store
- Approximately 8,562 square feet of fast-food restaurants without drive-through
- Approximately 13,650 square feet of fast-casual restaurants
- Approximately 14,811 square feet of fine-dining restaurants
- Approximately 2,978 square feet of office space
- 60,000 square-foot banquet facility

Access to the site will continue to be provided via two existing right-in/right-out access drives and a full movement access drive off Finley Road, and via a full movement access drive off Roosevelt Road.

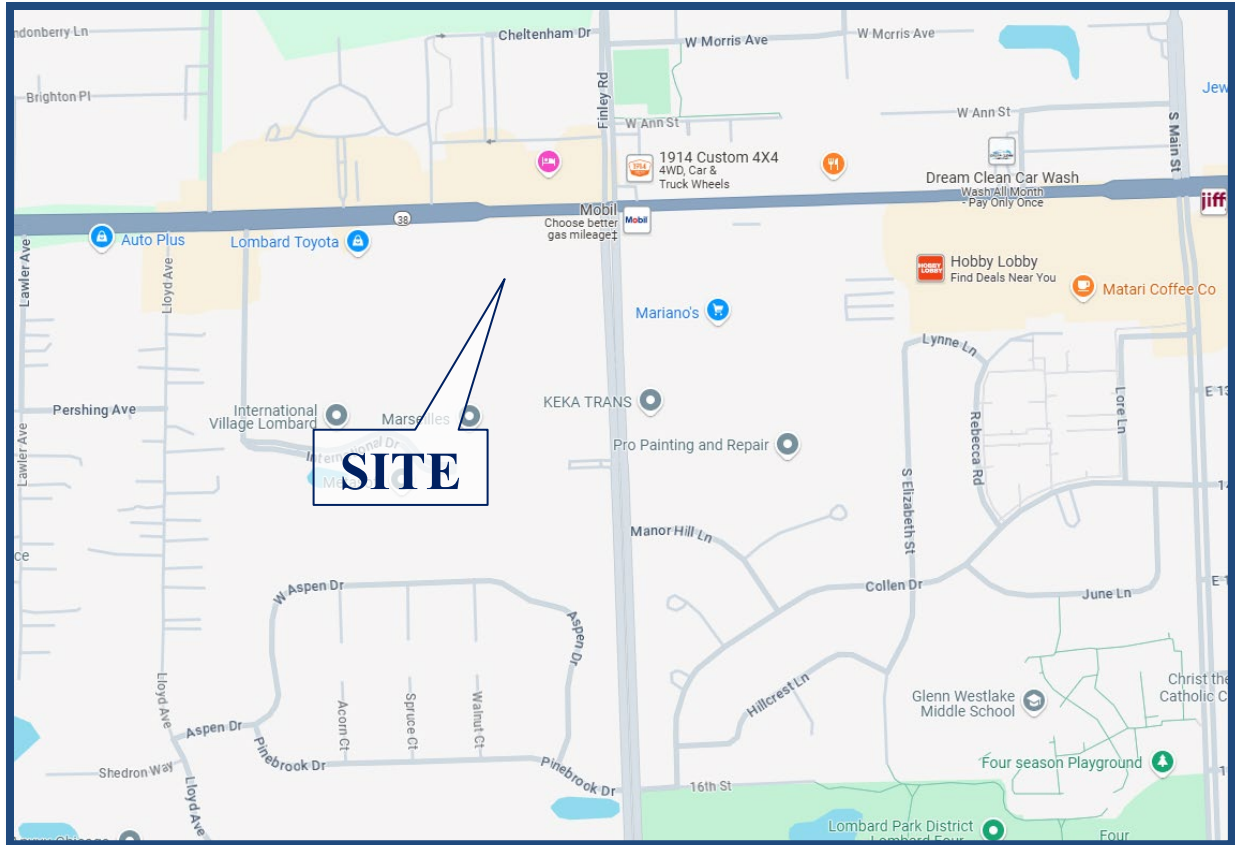
The purpose of this study was to examine background traffic conditions, assess the impact that the proposed redevelopment will have on traffic conditions in the area, and determine if any roadway or access improvements are necessary to accommodate traffic generated by the proposed development. **Figure 1** shows the location of the site in relation to the area roadway system. **Figure 2** shows an aerial view of the site.

The sections of this report present the following:

- Existing roadway conditions
- Description of the proposed development
- Directional distribution of the development traffic
- Vehicle trip generation for the redevelopment
- Future traffic conditions including access to the development
- Traffic analyses for the weekday morning, weekday evening, and Saturday midday peak hours
- Recommendations with respect to adequacy of the site access and adjacent roadway system
- A traffic signal warrant study performed at the intersection of Finley Road with Mariano's access drive/site access drive
- Parking evaluation

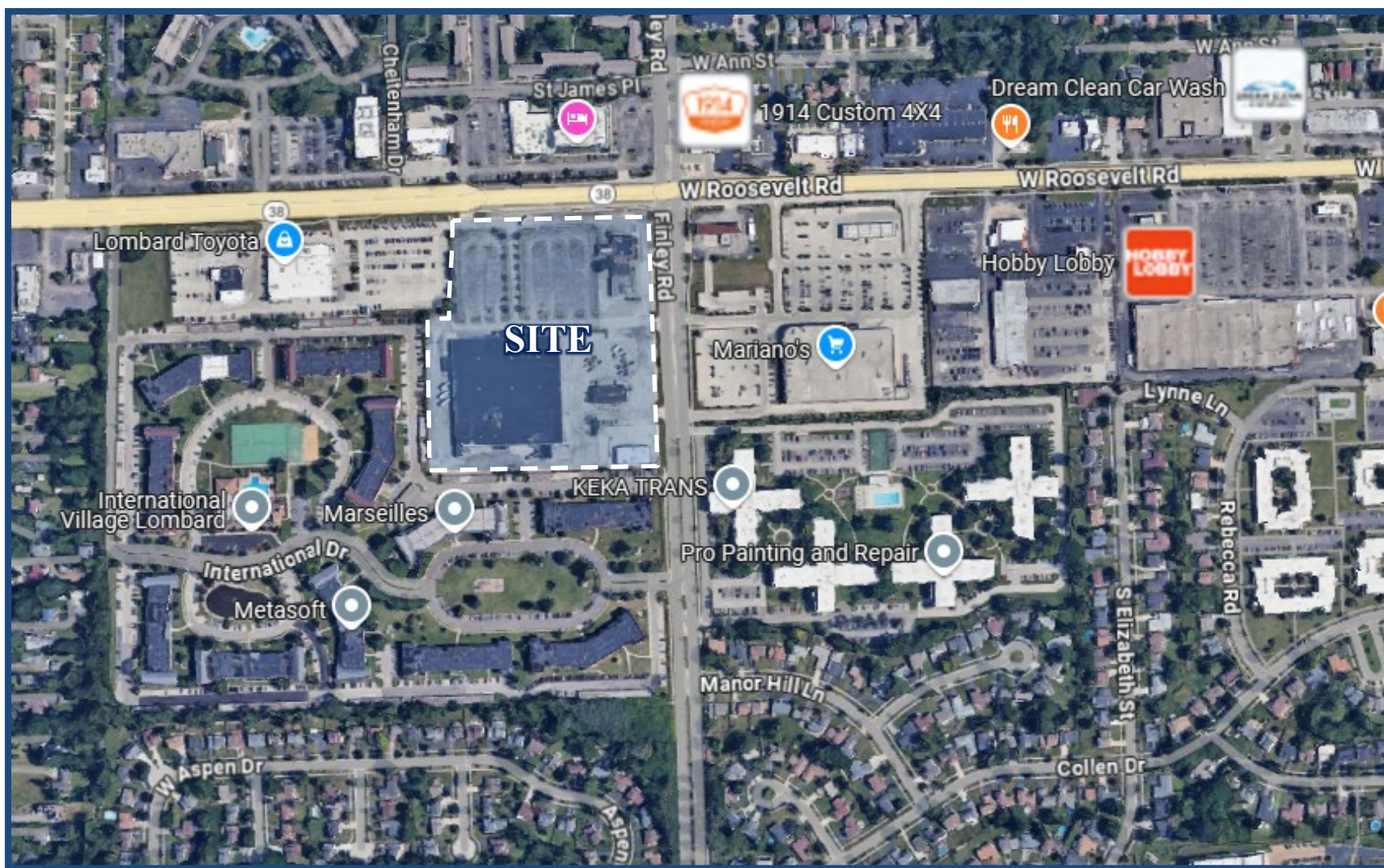
Traffic capacity analyses were conducted for the weekday morning, weekday evening, and Saturday midday peak hours for the following conditions:

1. Existing Conditions – Analyzes the capacity of the existing roadway system using peak hour traffic volumes from the traffic counts.
2. Year 2031 No-Build Conditions – Analyzes the capacity of the future roadway system using existing traffic volumes increased by an ambient area growth factor not attributable to any particular development.
3. Year 2031 Total Projected Conditions – Analyzes the capacity of the future roadway system assuming the projected traffic volumes that include the existing traffic volumes, ambient area growth not attributable to any particular development, and the traffic estimated to be generated by the proposed development.



Site Location

Figure 1



Aerial View of Site

Figure 2

2. Existing Conditions

Existing transportation conditions in the vicinity of the site were documented based on field visits conducted by KLOA, Inc. in order to obtain a database for projecting future conditions. The following provides a description of the geographical location of the site, physical characteristics of the area roadway system including lane usage and traffic control devices, and existing peak hour traffic volumes.

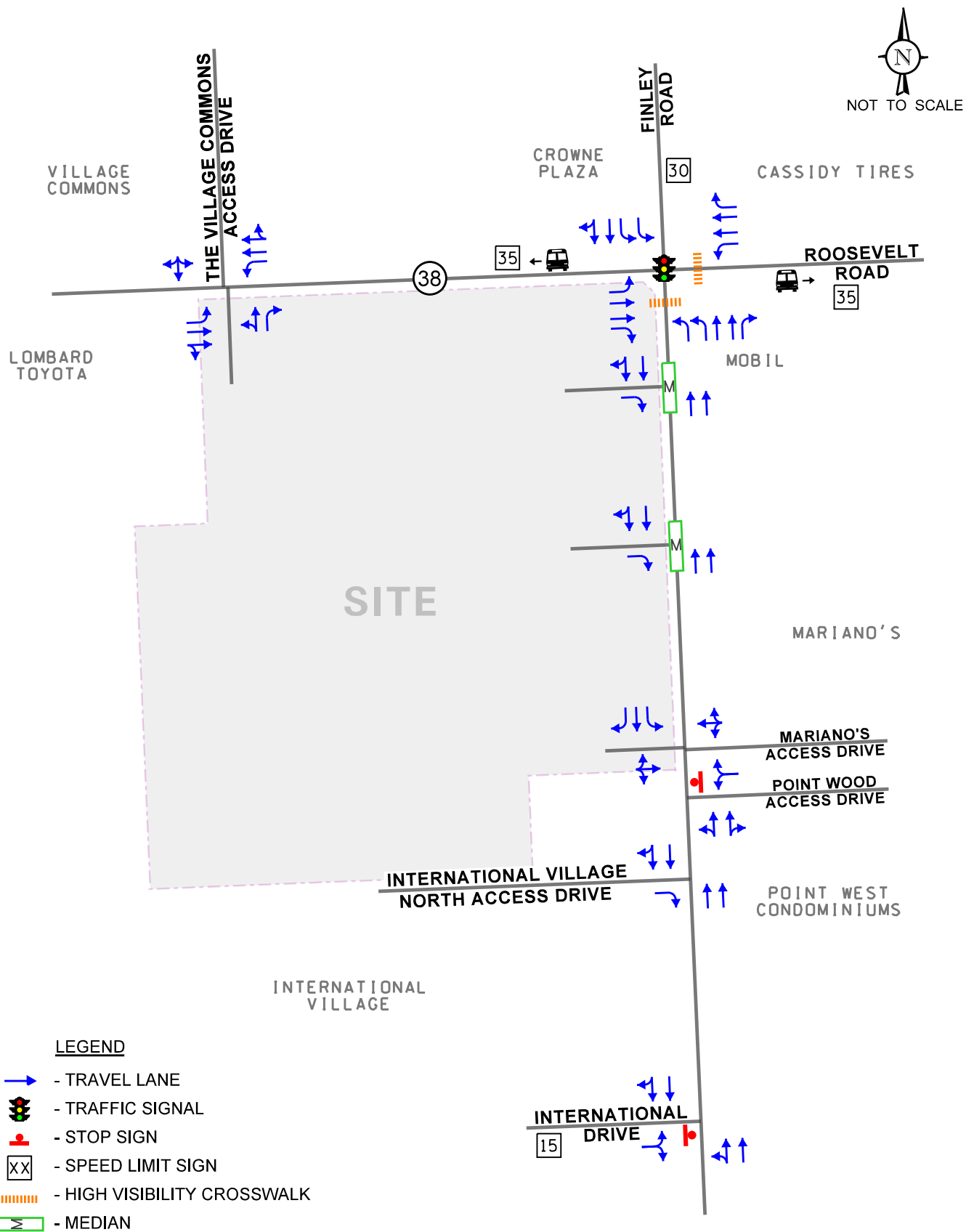
Site Location

The site is located in the southwest quadrant of the intersection of Roosevelt Road with Finley Road in Lombard, Illinois. Land uses in the vicinity of the site are a mixture of residential and commercial. Lombard Toyota is located to the west of the site, a Mobil fuel center and Mariano's grocery store are located to the east of the site, the International Village apartments are located to the west and south of the site, and the Crowne Plaza Hotel and the Village Commons strip retail center are located to the north of the site.

Existing Roadway System Characteristics

The characteristics of the existing roadways near the fuel center are described below and illustrated in **Figure 3**.

Roosevelt Road (IL Route 38) is an east-west other principal arterial roadway bordering the site to the north. It has two through lanes in each direction separated by a painted median with a posted speed limit of 35 mph. At its signalized intersection with Finley Road, Roosevelt Road provides an exclusive left-turn lane, two through lanes, and an exclusive right-turn lane on both approaches. High visibility crosswalks and pedestrian signals are provided on the east and south legs of this intersection. At its unsignalized intersection with the site full movement access drive/the Village Commons/Crowne Plaza access drive, Roosevelt Road provides an exclusive left-turn lane, a through lane and a shared through/right-turn lane on both approaches. Roosevelt Road is under the jurisdiction of the Illinois Department of Transportation (IDOT) and is designated as a Strategic Regional Arterial (SRA). It carries an Annual Average Daily Traffic (AADT) of approximately 34,300 vehicles (IDOT 2023) in the vicinity of the site.



Finley Road is a north-south roadway that provides two through lanes in each direction. Sidewalks are generally provided on both sides of the roadway and parking is prohibited. At its signalized intersection with Roosevelt Road, Finley Road provides dual left-turn lanes, two through lanes, and an exclusive right-turn lane on the northbound approach and dual left-turn lanes, a through lane and a shared through/right-turn lane on the southbound approach. At its unsignalized intersections with the two right/in/right-out access drives to the site, Finley Road provides two through lanes on the northbound approach and a through lane and a shared through/right-turn lane on the southbound approach. At its unsignalized intersection with the site full movement access drive/Mariano's access drive, Finley Road provides a shared left-turn/through lane and a shared through/right-turn lane on the northbound approach and an exclusive left-turn lane, a through lane, and a shared through/right-turn lane on the southbound approach. At its unsignalized intersection with International Drive, Roosevelt Road provides a shared left-turn/through lane and a through lane on the northbound approach and a through lane and a shared through/right-turn lane on the southbound approach. Finley Road is classified as a major collector roadway and carries an AADT of 6,800 vehicles (IDOT 2020) north of Roosevelt Road and is classified as a minor arterial roadway and carries an AADT of 18,700 vehicles (IDOT 2023) south of Roosevelt Road. Finley Road in the vicinity of the site, is under the jurisdiction of the Village of Lombard and has a posted speed limit of 35 miles per hour.

International Drive is an east-west private roadway that provides access to the International Village apartment development located on the west side of Finley Road providing one through lane in each direction. At its unsignalized intersection with Finley Road, International Drive provides one inbound lane and one outbound lane. International Drive has a posted speed limit of 15 miles per hour.

Existing Traffic Volumes

In order to determine current traffic conditions within the study area, KLOA, Inc. conducted peak period traffic counts utilizing Miovision Scout Collection Units at the following intersections:

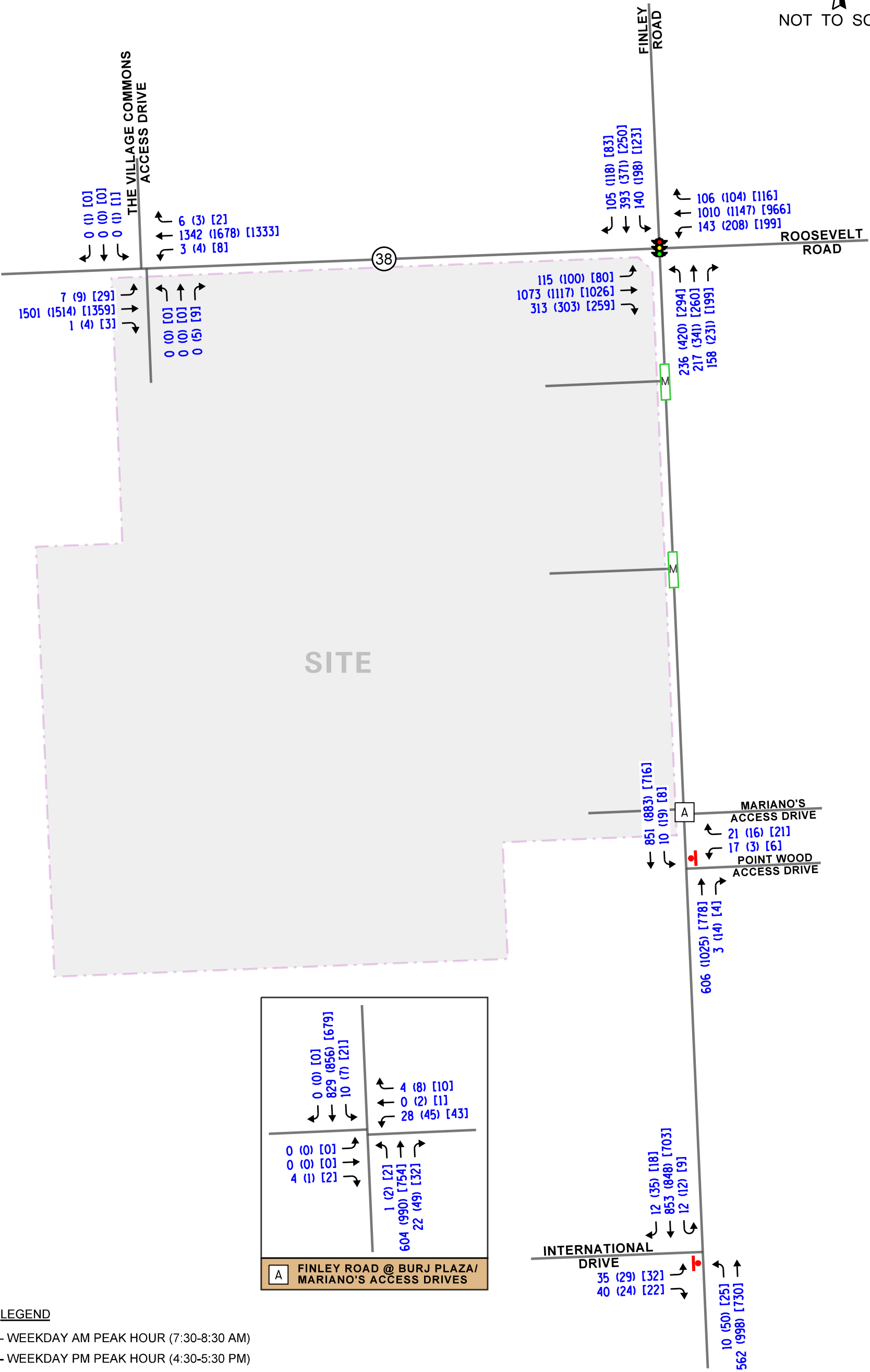
- Roosevelt Road (IL Route 38) with Finley Road
- Roosevelt Road with Burj Plaza Full Movement Access Drive/Village Commons/Crowne Plaza Access Drive
- Finley Road with Mariano's Access Drive/Burj Plaza Full Movement Access Drive
- Finley Road with International Drive

The traffic counts were conducted on Thursday, March 27, 2025 during the weekday morning (7:00 A.M. to 9:00 A.M.), weekday evening (2:00 P.M. to 6:00 P.M.), Saturday midday (12:00 P.M. to 2:00 P.M.) peak periods. The results of the traffic counts show that the peak hours of traffic generally occur between 7:30 A.M. and 8:30 A.M. during the weekday morning peak period, between 4:30 P.M. and 5:30 P.M. during the weekday evening peak period, and between 12:00 P.M. and 1:00 P.M. during the Saturday midday peak period.

The existing traffic volumes are illustrated in **Figure 4**. Copies of the traffic count summary sheets are included in the Appendix.



NOT TO SCALE



Crash Data Summary

KLOA, Inc. obtained crash data for the most recent available past five years (2019 to 2023) at the intersection of Roosevelt Road with Finley Road. A review of the crash data revealed that no fatalities were reported at this intersection during the review period. Summaries of the crash data are shown in **Table 1**.¹

Table 1
ROOSEVELT ROAD WITH FINLEY ROAD - CRASH SUMMARY

Year	Type of Crash Frequency						
	Angle	Object	Rear End	Sideswipe	Turning	Other	Total
2019	1	1	4	0	7	0	13
2020	0	0	4	0	6	0	10
2021	1	1	4	0	4	0	10
2022	2	0	3	2	10	0	17
2023	<u>1</u>	<u>1</u>	<u>3</u>	<u>1</u>	<u>7</u>	<u>0</u>	<u>13</u>
Total	5	3	18	3	34	0	63
Average/Year	1.0	<1.0	3.6	<1.0	6.8	--	12.6

¹ IDOT DISCLAIMER: The motor vehicle crash data referenced herein was provided by the Illinois Department of Transportation. Any conclusions drawn from analysis of the aforementioned data are the sole responsibility of the data recipient(s).

3. Traffic Characteristics of the Proposed Redevelopment

In order to properly evaluate future traffic conditions in the surrounding area, it was necessary to determine the traffic characteristics of the proposed redevelopment, including the directional distribution and volumes of traffic that it will generate.

Proposed Development Plan

As proposed, the site will be redeveloped to contain the following uses:

- 5,000 square-foot day care center with a maximum population of 72 children
- 3,565 square-foot fast food restaurant with drive-through
- 1,280 square-foot candy shop
- 4,782 square-foot smoking lounge
- 3,624 square feet of retail/service uses
- 2,978 square feet of personal care store
- Approximately 8,562 square feet of fast-food restaurants without drive-through
- Approximately 13,650 square feet of fast-casual restaurants
- Approximately 14,811 square feet of fine-dining restaurants
- Approximately 2,978 square feet of office space
- 60,000 square-foot banquet facility

Access to the site will continue to be provided via the following:

- An existing full movement access drive on the south side of Roosevelt Road located approximately 610 feet west of Finley Road. This access drive provides one inbound lane and two outbound lanes that will be striped for a shared left-turn/through lane and an exclusive right-turn lane. The outbound movements should be under stop sign control.
- An existing right-in/right-out access drive on the west side of Finley Road located approximately 185 feet south of Roosevelt Road. This access drive provides one inbound lane and one outbound lane. The outbound movement should be under stop sign control.
- An existing right-in/right-out access drive on the west side of Finley Road located approximately 395 feet south of Roosevelt Road. This access drive provides one inbound lane and one outbound lane. The outbound movement should be under stop sign control.
- An existing full movement access drive on the west side of Finley Road located approximately 665 feet south of Roosevelt Road. This access drive provides one inbound lane and two outbound lanes that will be striped for an exclusive left-turn lane and a shared through/right-turn lane. The outbound movements should be under stop sign control.

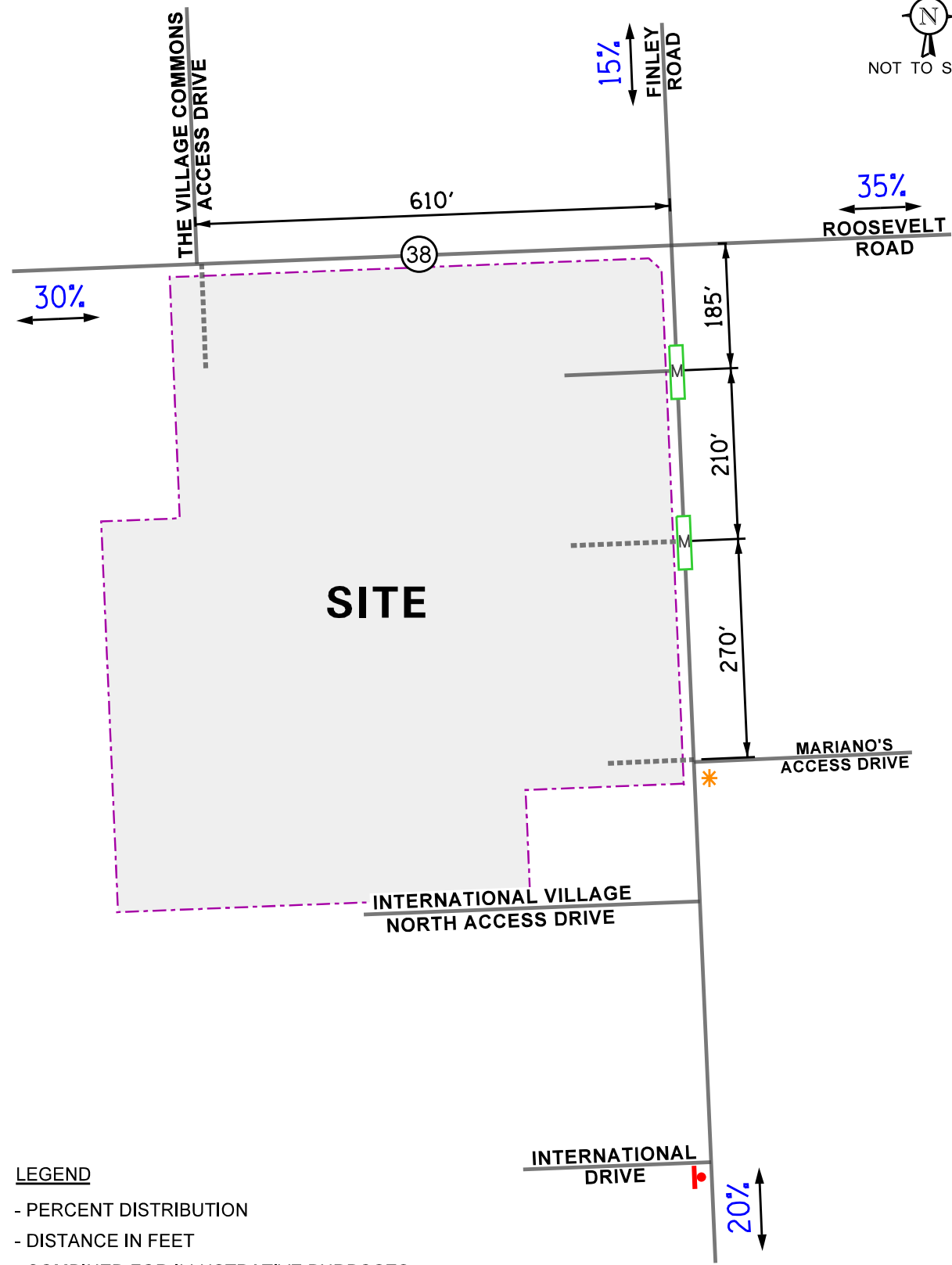
As part of the development, it is recommended that the existing raised median along Finley Road at its intersection with the Mariano's full access drive/site full access drive be reconstructed to provide an exclusive northbound left-turn lane. This turn lane should provide 145 feet of storage and 175 feet of taper. Furthermore, consideration should be given to consolidating the Mariano's access drive and the existing access drive to the Point West condominium development located south of Mariano's in order to provide a more efficient and safer intersection. This is a future consideration that should involve all parties as well as the Village.

Directional Distribution

The directional distribution of future site-generated trips on the roadway system is a function of several variables, including the operational characteristics of the roadway system and the ease with which drivers can travel over various sections of the roadway system. This is particularly true for pass-by traffic. **Figure 5** illustrates the directional distribution of the development-generated traffic. Figure 5 also shows the distance, in feet, between the study area intersections and the access drives.



NOT TO SCALE



LEGEND

- 00% - PERCENT DISTRIBUTION
- 00' - DISTANCE IN FEET
- * - COMBINED FOR ILLUSTRATIVE PURPOSES

Burj Plaza
Lombard, Illinois

Directional Distribution

Peak Hour Traffic Volumes

The number of peak hour trips estimated to be generated by the proposed development was based on vehicle trip generation rates contained in *Trip Generation Manual*, 11th Edition, published by the Institute of Transportation Engineers (ITE). The following rates were used to generate estimate the peak hour trips:

- The “Fast-Food Restaurant with Drive-Through” (Land-Use Code 934) rate was used for the approximately 3,565 square feet fast-food restaurant with drive-through.
- The “Strip Retail Plaza (<40k)” (LUC 822) rate was used for the candy store and the retail spaces.
- The “Brewery Tap Room” (LUC 971) rate was used for the smoking lounge.
- The “Day Care Center” (LUC 565) rate was used for the proposed day care.
- The “Fast-Food Restaurant Without Drive-Through” (LUC 933) rate was used for the quick service restaurants.
- The “Fast-Casual Restaurant” (LUC 930) rate was used for the proposed fast-casual restaurants.
- The “Fine-Dining Restaurant” (LUC 931) rate was used for the fine-dining restaurants.
- The “General Office Building” (LUC 710) rate was used for the proposed office spaces.
- The “Hair Salon” (LUC 918) rate was used for the proposed personal care space.

The banquet facility is proposed to have a maximum seating capacity of 1,500 seats. Based on information provided by the owner, a typical event is expected to accommodate 1,000 people on average during the evening peak hour. Based on KLOA, Inc.’s surveys at other banquet facilities, the typical average vehicle occupancy is three people per vehicle. Applying this vehicle occupancy rate to the 1,000 attendees translates into 333 trips. However, not all the guests arrive during the peak hour. As such, a conservative rate of two-thirds of the guests were assumed to arrive during the evening peak hour with the remaining one-third arriving before and after the peak hour. Additionally, the banquet will have a maximum of 23 staff.

It is important to note that surveys conducted by ITE have shown that approximately 50 percent of trips made to drive-through restaurants, 40 percent of trips made to fast-casual and fine-dining restaurants, and 20 percent of trips made to retail spaces are diverted from the existing traffic on the roadway system. This is particularly true during the weekday morning and weekday evening peak hours when traffic is diverted from the home-to-work and work-to-home trips. As such, the number of new passenger vehicle trips generated by the development was reduced to account for pass-by traffic. Further, it is anticipated that some of the patrons of the plaza will be patrons of restaurants as well. As such, the total trips that will be generated by these uses were reduced by 10 percent to account for the interaction between the various uses. **Table 2** summarizes the trips projected to be generated by the proposed development.

Table 2

ESTIMATED PEAK HOUR SITE-GENERATED TRAFFIC VOLUMES

ITE Land-Use Code	Type/Size	Weekday Morning Peak Hour			Weekday Evening Peak Hour			Saturday Midday Peak Hour		
		In	Out	Total	In	Out	Total	In	Out	Total
934	Fast-Food Restaurant with Drive-Through (3,565 s.f.)	81	78	159	61	57	118	100	97	197
822	Candy Store (1,280 s.f.)	--	--	--	4	4	8	4	4	8
971	Smoking Establishment (4,782 s.f.)	--	--	--	28	19	47	30	12	42
565	Day Care (72 Students)	30	26	56	26	29	55	5	3	8
822	Retail (3,624 s.f.)	5	4	9	19	19	38	12	12	24
918	Hair Salon (2,978 s.f.)	3	1	4	1	3	4	5	10	15
933	Fast Food Restaurant (8,562 s.f.)	5	5	10	142	142	284	229	238	467
930	Fast Casual Restaurant (13,650 s.f.)	10	10	20	94	77	171	245	201	446
931	Fine Dining Restaurant (14,811 s.f.)	5	5	10	77	38	115	93	65	158
710	General Office (2,978 s.f.)	7	1	8	2	7	9	1	1	2
	Banquet Facility (60,000 s.f. – 1,500 Seats)	10	0	10	220	15	235	10	10	20
	Sub Total	156	130	286	674	410	1,084	734	653	1,387
	<i>Interaction Reduction (10%)¹</i>	-5	-3	-8	-5	-7	-12	-5	-4	-9
	<i>Pass-By Reduction (50%)²</i>	-40	-40	-80	-71	-71	-142	-117	-117	-234
	<i>Pass-By Reduction (40%)³</i>	0	0	0	-57	-57	-114	-120	-120	-240
	<i>Pass-By Reduction (20%)⁴</i>	-1	-1	-2	-2	-2	-4	-1	-1	-2
	Total Pass-By	-41	-41	-82	-130	-130	-260	-238	-238	-476
	Total New Trips	110	86	196	539	273	812	491	411	902
1- 10 percent Interaction reduction was applied to all trips except the restaurant trips. 2- 50 percent pass-by reduction was applied to the fast-food restaurant trips. 3- 40 percent pass-by reduction was applied to the fast-casual and fine-dining restaurant trips 4- 20 percent pass-by reduction was applied to the retail trips.										

4. Projected Traffic Conditions

The total projected traffic volumes include the existing traffic volumes, increase in background traffic due to ambient growth, and the traffic estimated to be generated by the proposed subject fuel center.

Development Traffic Assignment

The estimated weekday morning, weekday evening, and Saturday midday traffic volumes that will be generated by the proposed development were assigned to the roadway system in accordance with the previously described directional distribution (Figure 5). **Figure 6** illustrates the traffic assignment of the development's new passenger vehicle trips and **Figure 7** illustrates the pass-by traffic assignment of the development.

Background (No-Build) Traffic Conditions

The existing traffic volumes (Figure 4) were increased by a regional growth factor to account for the increase in existing traffic related to regional growth in the area (i.e., not attributable to any planned development). Based on AADT projections provided by the Chicago Metropolitan Agency for Planning (CMAP), the existing traffic volumes were increased by an annually compounded growth rate of 0.43 percent per year for six years (buildout year plus five years) for a total of three percent. A copy of the CMAP letter is included in the Appendix.

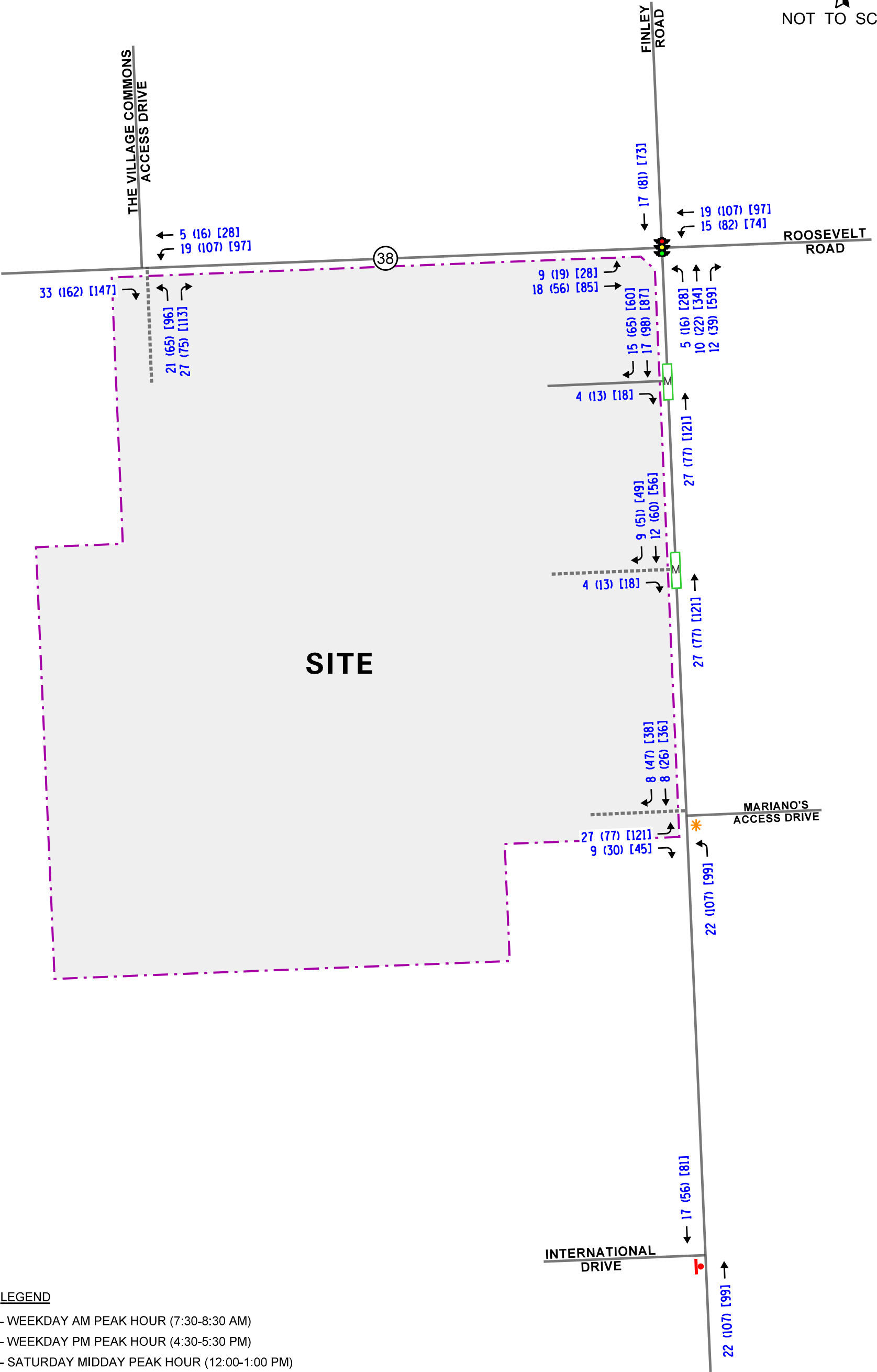
The projected Year 2031 no-build traffic volumes, which include the existing traffic volumes increased by the regional growth factor, are illustrated in **Figure 8**.

Total Projected Traffic Volumes

The development-generated traffic was added to the Year 2031 no-build traffic volumes to determine the projected Year 2031 total projected traffic volumes, as shown in **Figure 9**.

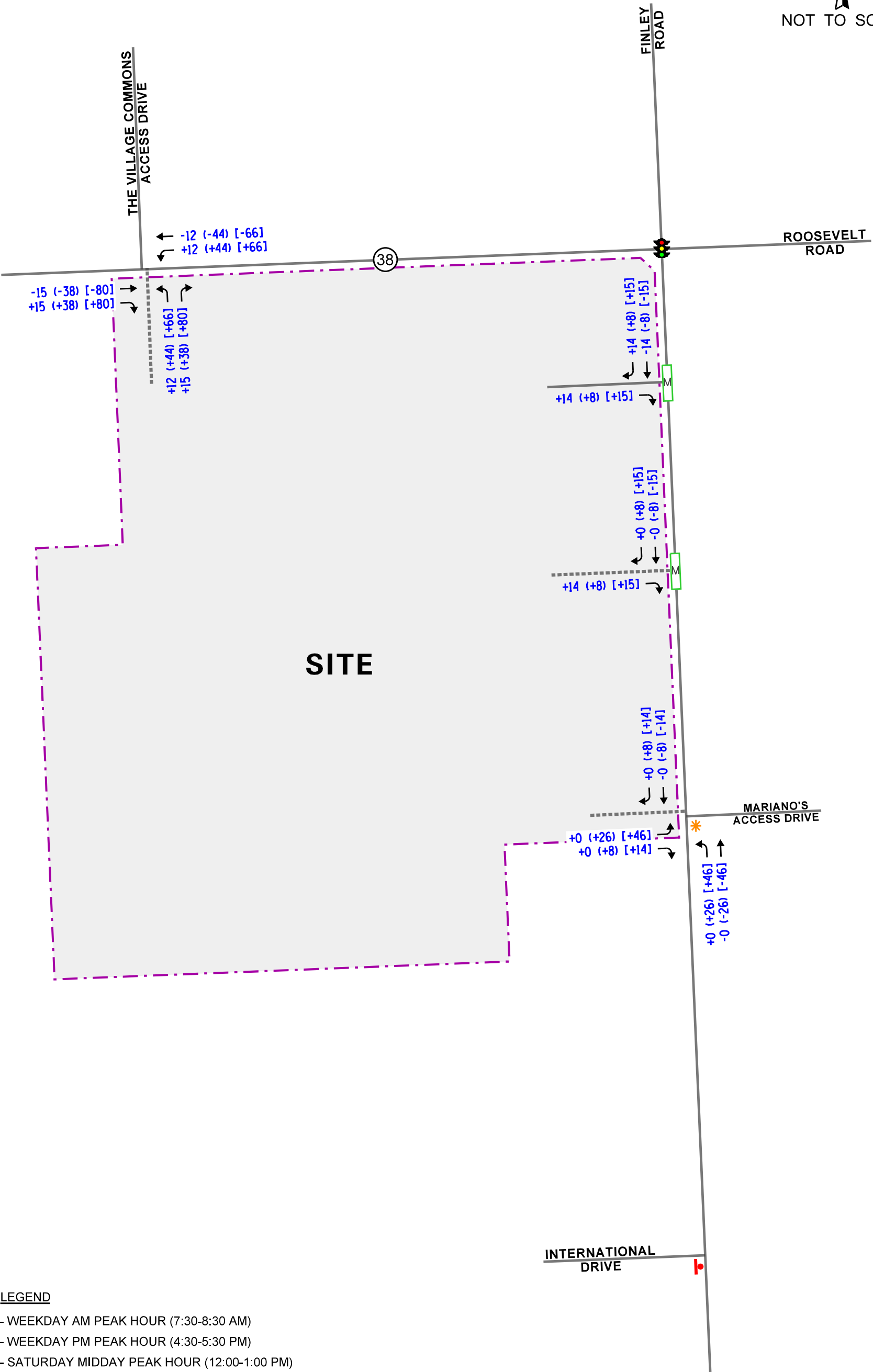


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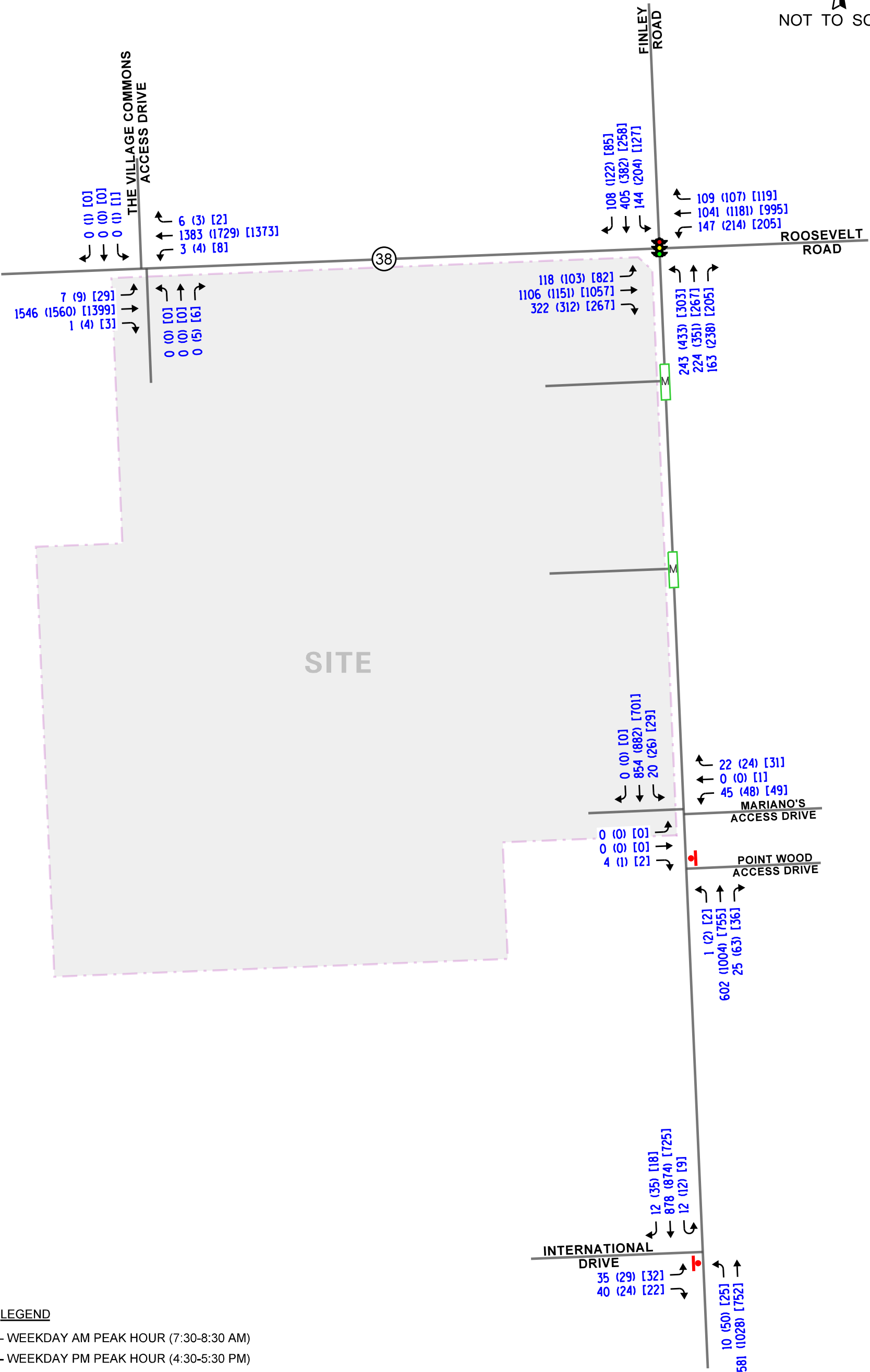


LEGEND

- 00 - WEEKDAY AM PEAK HOUR (7:30-8:30 AM)
- (00) - WEEKDAY PM PEAK HOUR (4:30-5:30 PM)
- [00] - SATURDAY MIDDAY PEAK HOUR (12:00-1:00 PM)
- * - COMBINED FOR ILLUSTRATIVE PURPOSES



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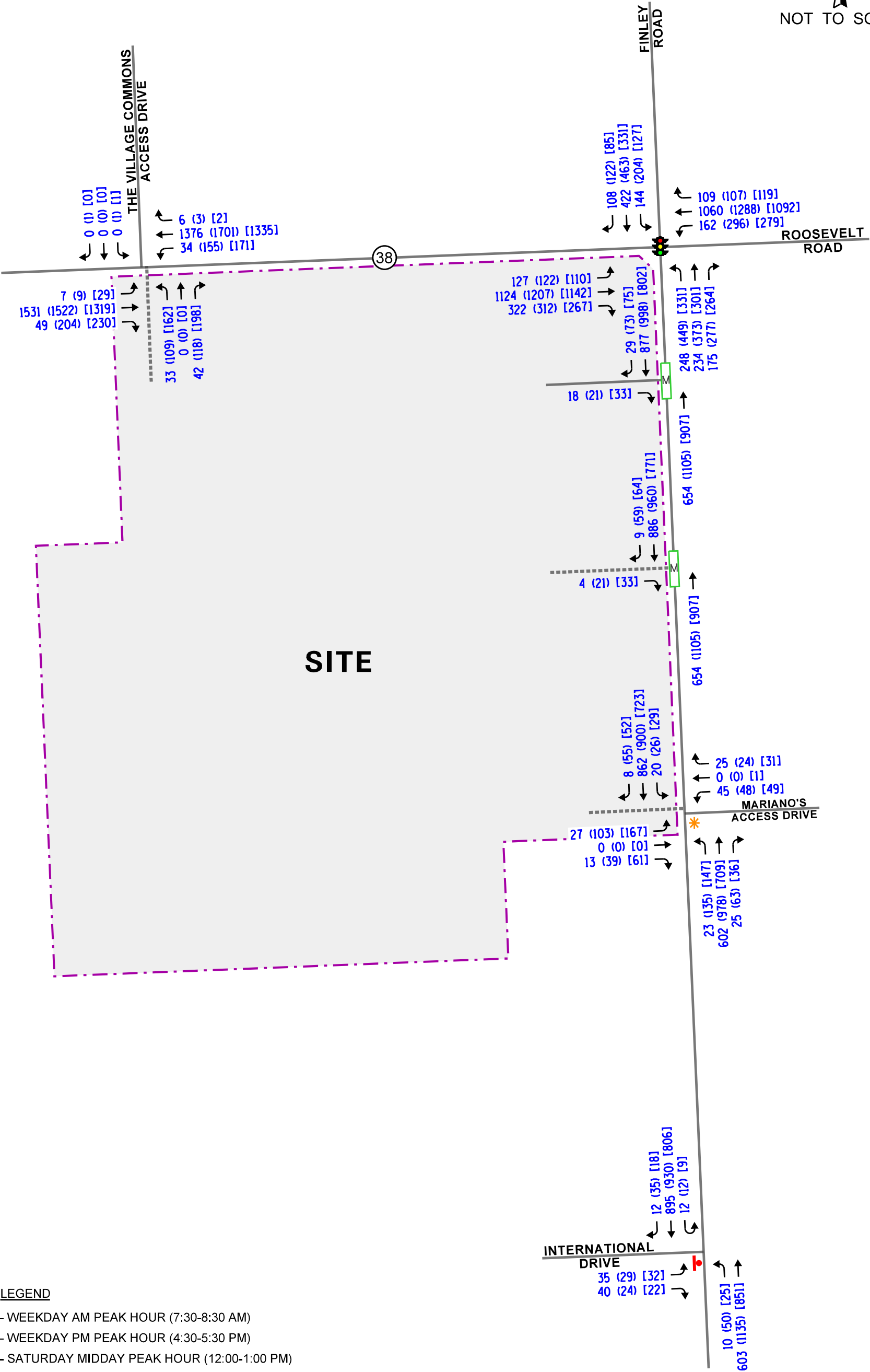


LEGEND

- 00 - WEEKDAY AM PEAK HOUR (7:30-8:30 AM)
- 000 - WEEKDAY PM PEAK HOUR (4:30-5:30 PM)
- 000 - SATURDAY MIDDAY PEAK HOUR (12:00-1:00 PM)



NOT TO SCALE



5. Traffic Analysis and Recommendations

The following provides an evaluation conducted for the weekday morning, weekday evening, and Saturday midday peak hours. The analysis includes conducting capacity analyses to determine how well the roadway system and access drive are projected to operate and whether any roadway improvements or modifications are required.

Traffic Analyses

Roadway and adjacent or nearby intersection analyses were performed for the weekday morning, weekday evening, and Saturday midday peak hours for the existing, Year 2031 no-build, and Year 2031 total projected traffic volumes.

The traffic analyses were performed using the methodologies outlined in the Transportation Research Board's *Highway Capacity Manual (HCM)*, 7th Edition and analyzed using Synchro/SimTraffic 12 software. The analysis for the traffic signal-controlled intersection was performed using actual cycle lengths and phasings to determine the average overall vehicle delay and levels of service.

The analyses for the unsignalized intersections determine the average control delay to vehicles at an intersection. Control delay is the elapsed time from a vehicle joining the queue at a stop sign (including the time required to decelerate to a stop) until its departure from the stop sign and resumption of free flow speed. The methodology analyzes each intersection approach controlled by a stop sign and considers traffic volumes on all approaches and lane characteristics.

The ability of an intersection to accommodate traffic flow is expressed in terms of level of service, which is assigned a letter from A to F based on the average control delay experienced by vehicles passing through the intersection. The *Highway Capacity Manual* definitions for levels of service and the corresponding control delay for signalized intersections and unsignalized intersections are included in the Appendix of this report.

Summaries of the traffic analysis results showing the level of service and overall intersection delay (measured in seconds) for the existing, Year 2031 no-build, and Year 2031 total projected conditions are presented in **Tables 3** through **6**. A discussion of each intersection follows. Summary sheets for the capacity analyses are included in the Appendix.

Table 3

CAPACITY ANALYSIS RESULTS – ROOSEVELT ROAD WITH FINLEY ROAD – SIGNALIZED
EXISTING INTERSECTION GEOMETRICS

Existing Conditions	Peak Hour	Eastbound			Westbound			Northbound			Southbound		Overall	
		L	T	R	L	T	R	L	T	R	L	T/R		
	Weekday Morning	C 22.2	C 34.4	B 14.9	C 26.7	C 29.9	B 12.4	E 67.8	D 45.4	C 33.3	E 68.6	E 66.0	D 38.2	
		C – 29.4			C – 28.1			D – 50.9			E – 66.6			
	Weekday Evening	C 21.9	D 37.1	B 13.1	D 36.9	C 32.9	B 13.2	E 69.5	D 49.7	D 36.6	E 73.9	E 70.1	D 42.5	
		C – 31.3			C – 32.1			E – 55.1			E – 71.2			
	Saturday Midday	B 12.6	C 26.5	B 10.8	B 18.9	C 21.7	A 9.5	E 65.0	D 48.3	D 35.2	E 62.8	E 61.8	C 32.1	
		C – 22.7			C – 20.2			D – 51.4			E – 62.1			
	No-Build Conditions	Weekday Morning	C 24.6	D 36.4	B 15.5	C 32.2	C 30.9	B 12.5	E 67.9	D 45.3	C 32.8	E 68.7	E 67.2	D 39.4
			C – 31.1			C – 29.5			D – 50.8			E – 67.5		
Weekday Evening		C 24.3	D 39.7	B 13.6	D 48.4	D 35.1	B 13.6	E 69.5	D 48.9	D 35.7	E 74.1	E 74.2	D 44.6	
		C – 33.4			D – 35.5			D – 54.6			E – 74.2			
Saturday Midday		B 13.1	C 27.6	B 11.3	C 21.4	C 22.2	A 9.5	E 65.5	D 48.4	C 34.9	E 62.8	E 62.6	C 32.8	
		C – 23.7			C – 20.9			D – 51.5			E – 62.6			
Projected Conditions	Weekday Morning	C 29.1	D 38.1	B 16.0	D 42.1	C 31.8	B 12.7	E 68.0	D 45.1	C 32.4	E 68.7	E 68.2	D 40.8	
		C – 32.8			C – 31.5			D – 50.4			E – 68.3			
	Weekday Evening	D 41.0	D 43.0	B 14.0	F 99+	D 39.2	B 14.3	E 69.7	D 49.1	D 36.2	E 74.1	E 79.8	D 51.4	
		D – 37.4			D – 50.5			D – 54.3			E – 78.4			
	Saturday Midday	B 16.7	C 34.0	B 13.4	D 52.9	C 24.5	B 10.0	E 67.5	D 48.1	C 32.8	E 62.8	E 68.7	D 38.0	
		C – 29.2			C – 28.6			D – 50.8			E – 67.3			
Letter denotes Level of Service Delay is measured in seconds.		L – Left Turn T – Through			R – Right Turn									

Table 4

CAPACITY ANALYSIS RESULTS – UNSIGNALIZED – EXISTING CONDITIONS

Intersection	Weekday Morning Peak Hour		Weekday Evening Peak Hour		Saturday Midday Peak Hour	
	LOS	Delay	LOS	Delay	LOS	Delay
Roosevelt Road with the Village Commons Access Drive/Full Movement Access Drive¹						
• Northbound Approach	A	0.1	B	10.6	B	10.1
• Southbound Approach	A	0.1	C	19.9	C	23.1
• Eastbound Left Turn	B	11.7	B	12.4	B	10.5
• Westbound Left Turn	B	12.1	B	11.1	B	10.5
Finley Road with Full Movement Access Drive/Mariano's Access Drive¹						
• Eastbound Approach	A	9.9	A	9.3	A	9.0
• Westbound Approach	B	14.6	C	25.0	C	19.2
• Northbound Left Turn	A	8.5	A	8.5	A	8.4
• Southbound Left Turn	A	9.0	B	10.9	A	9.7
Finley Road with International Drive²						
• Eastbound Approach	C	15.8	D	27.8	C	18.2
• Northbound Left Turn	A	8.7	A	9.0	A	8.5
LOS = Level of Service Delay is measured in seconds.			1 – Two-way stop control 2 – One-way stop control			

Table 5

CAPACITY ANALYSIS RESULTS – UNSIGNALIZED – NO-BUILD CONDITIONS

Intersection	Weekday Morning Peak Hour		Weekday Evening Peak Hour		Saturday Midday Peak Hour	
	LOS	Delay	LOS	Delay	LOS	Delay
Roosevelt Road with the Village Commons Access Drive/Full Movement Access Drive¹						
• Northbound Approach	A	0.1	B	10.7	B	10.2
• Southbound Approach	A	0.1	C	21.1	C	23.7
• Eastbound Left Turn	B	11.9	B	12.9	B	10.7
• Westbound Left Turn	B	12.4	B	11.4	B	10.7
Finley Road with Full Movement Access Drive/Mariano's Access Drive¹						
• Eastbound Approach	A	9.9	A	9.3	A	9.1
• Westbound Approach	B	14.9	D	25.3	C	19.8
• Northbound Left Turn	A	8.6	A	8.7	A	8.4
• Southbound Left Turn	A	9.0	B	11.1	A	9.8
Finley Road with International Drive²						
• Eastbound Approach	C	16.2	D	30.4	C	19.2
• Northbound Left Turn	A	8.7	A	9.1	A	8.6
LOS = Level of Service Delay is measured in seconds.			1 – Two-way stop control 2 – One-way stop control			

Table 6

CAPACITY ANALYSIS RESULTS – UNSIGNALIZED – TOTAL PROJECTED CONDITIONS

Intersection	Weekday Morning Peak Hour		Weekday Evening Peak Hour		Saturday Midday Peak Hour	
	LOS	Delay	LOS	Delay	LOS	Delay
Roosevelt Road with the Village Commons Access Drive/Full Movement Access Drive¹						
• Northbound Approach	C	24.0	F	73.0	F	99.0
• Southbound Approach	A	0.1	E	48.8	F	57.5
• Eastbound Left Turn	B	12.0	B	12.7	B	10.6
• Westbound Left Turn	B	13.6	C	18.0	C	16.1
Finley Road with North Right-In/Right-Out Access Drive²						
• Eastbound Approach	B	11.9	B	13.0	B	12.0
Finley Road with South Right-In/Right-Out Access Drive²						
• Eastbound Approach	B	11.7	B	12.6	B	11.7
Finley Road with Full Movement Access Drive/Mariano's Access Drive¹						
• Eastbound Approach	B	14.5	E	41.6	F	87.6
• Westbound Approach	C	15.7	E	47.8	D	34.8
• Northbound Left Turn	A	8.7	A	9.6	A	9.4
• Southbound Left Turn	A	9.0	B	10.9	A	9.6
Finley Road with International Drive²						
• Eastbound Approach	C	16.8	E	38.7	C	23.3
• Northbound Left Turn	A	8.8	A	9.3	A	8.9
LOS = Level of Service Delay is measured in seconds.			1 – Two-way stop control 2 – One-way stop control			

Discussion and Recommendations

The following summarizes how the intersections are projected to operate and identifies any roadway and traffic control improvements necessary to accommodate the fuel center-generated traffic.

Roosevelt Road with Finley Road

The results of the capacity analysis indicate that this signalized intersection currently operates at an overall Level of Service (LOS) D during the weekday morning and weekday evening peak hours and LOS C during the Saturday midday peak hour. The eastbound and westbound approaches currently operate at LOS C during all three peak hours, the northbound approach operates at LOS D during all three peak hours, and the southbound approach operates at LOS E during all three peak hours. It should be noted that the intersection operates better than the capacity analyses indicate as, per IDOT requirements, the capacity analyses were performed assuming no right turns on red although they are permitted on all approaches.

Under Year 2031 no-build conditions, the intersection is projected to operate at the existing levels of service with increases in delay of less than two seconds over existing conditions. All approaches are projected to operate at LOS D or better during all three peak hours except for the southbound approach that is projected to continue operating at LOS E during the peak hours.

Under Year 2031 total projected conditions, the intersection is projected to operate at LOS D during the weekday morning, weekday evening, and Saturday midday peak hours with increases in delay of less than nine seconds over no-build conditions. The eastbound, westbound, and northbound approaches are projected to operate at LOS D or better during all three peak hours. The southbound approach is projected to continue to operate at a LOS E during all three peak hours with a Volume to Capacity ratio (v/c) of less than one. The maximum 95th percentile queue for the northbound approach is projected to be approximately 300 feet during the weekday evening peak hour and will not extend to the location of the full movement access drive on Finley Road. It should be noted that based on the results of the capacity analyses, the queues on the eastbound approach during the weekday morning and evening peak hours will extend to the location of the full movement access drive on Roosevelt Road. However, they will clear the intersection with each green phase of the signal. As such, this intersection has adequate reserve capacity to accommodate the traffic estimated to be generated by Burj Plaza and no roadway improvements or traffic control adjustments will be required.

Roosevelt Road with Site Access Drive/Village Commons Access Drive

The results of the capacity analysis indicate that the northbound and southbound approaches currently operate at LOS C or better during the weekday morning, weekday evening, and Saturday midday peak hours while the eastbound and westbound left-turn movements operate at LOS B during all three peak hours.

Under Year 2031 no-build conditions, all approaches and their critical movements are projected to operate at the same existing levels of service during all three peak hours with increases in delay of less than two seconds over existing conditions.

Burj Plaza Redevelopment Lombard, Illinois

Under Year 2031 total conditions, the northbound and southbound approaches are projected to operate at LOS E or better during all three peak hours except for the northbound approach that is projected to operate at LOS F with a v/c of less than one during the weekday evening and Saturday midday peak hours. The lower level of service of F is common and expected when an access drive intersects a major arterial such as Roosevelt Road. The eastbound and westbound left-turn movements are projected to operate at LOS C or better during all three peak hours with 95th percentile queues of one to two vehicles during all three peak hours that can be accommodated via the existing left-turn lanes along Roosevelt Road. The 95th percentile queue for the northbound approach is projected to be approximately one or two vehicles during the weekday morning peak hour, six vehicles during the weekday evening peak hour, and ten vehicles during the Saturday midday peak hour. A review of the site plan indicates that there will be stacking for approximately six vehicles before reaching the first east-west drive aisle. In order to ensure that vehicles do not block this east-west drive aisle, a “Do Not Block Intersection” sign should be provided on the north-south access road facing south at its intersection with the east-west drive aisle. As such, this intersection has adequate reserve capacity to accommodate the traffic estimated to be generated by Burj Plaza.

Finley Road with Mariano's Access Drive/Site Full Movement Access Drive

The results of the capacity analysis indicate that currently, all approaches and their critical movements operate at LOS C or better during the weekday morning, weekday evening, and Saturday midday peak hours.

Under Year 2031 no-build conditions, all approaches and their critical movements are projected to operate at LOS C or better during all three peak hours except for the westbound approach that is projected to operate at LOS D during the weekday evening peak hour.

Under Year 2031 total projected conditions, the eastbound and westbound approaches are projected to operate at LOS B during the weekday morning peak hour and LOS E during the weekday evening peak hour. The eastbound approach is projected to operate at LOS F during the Saturday midday peak hour with a v/c of less than one, and the westbound approach is projected to operate at LOS D during the Saturday midday peak hour. The lower levels of service E/F are common and expected when an access road intersects a major road such as Finley Road. Additionally, the northbound and southbound left-turn movements are projected to operate at LOS B or better during all three peak hours with 95th percentile queues of one to two vehicles that can be accommodated within the left-turn lanes along Finley Road. As such, this intersection has adequate reserve capacity to accommodate the traffic estimated to be generated by Burj Plaza and will ensure efficient and flexible access to the site.

Finley Road with International Drive

The results of the capacity analysis indicate that currently the eastbound approach operates at LOS C during the weekday morning and Saturday midday peak hours and LOS D during the weekday evening peak hour. The northbound left turn operates at LOS A during all three peak hours.

Under Year 2031 no-build conditions, the eastbound approach and the northbound left-turn movement are projected to continue operating at the same existing levels of service during all three peak hours with increases in delay of less than three seconds over existing conditions.

Under Year 2031 total projected conditions, the eastbound approach is projected to operate at LOS C during the weekday morning and Saturday midday peak hours and LOS E during the weekday evening peak hour with increases in delay of less than eight seconds over no-build conditions. As such, this intersection will be adequate to accommodate the traffic estimated to be generated by Burj Plaza and no roadway improvements or traffic control adjustments will be required.

Finley Road with Site North and South Right-In/Right-Out Access Drives

The results of the capacity analysis indicate that the eastbound approach is projected to operate at LOS B during the weekday morning, weekday evening, and Saturday midday peak hours at both north and south right-in/right-out access drives. As such, these intersections have adequate reserve capacity to accommodate the traffic estimated to be generated by Burj Plaza and will ensure efficient access to the site.

6. Traffic Signal Warrant Evaluation

As previously indicated, traffic counts were conducted at the intersection of Finley Road with Burj Plaza full movement access drive/Mariano's access drive from 7:00 A.M. to 5:00 P.M. These counts were utilized to determine if a traffic signal is warranted under future conditions. Installation of a traffic signal requires that one or more of the nine signal warrants outlined in the *Manual on Uniform Traffic Control Devices* (MUTCD 2009) is met. For the purposes of this study, Signal Warrant 1 (Eight Hour Vehicular Volume) and Signal Warrant 3 (Peak Hour) were utilized.

Warrant 1, (Eight-Hour Vehicular Volume) has two conditions. Per the MUTCD, Condition A (Minimum Vehicular Volume) is intended for application at locations where a large volume of intersecting traffic is the principal reason to consider installing a traffic control signal. Condition B (Interruption of Continuous Traffic) is intended for application at locations where Condition A is not satisfied and where the traffic volumes on a major street are so heavy that traffic on the minor intersecting street suffers excessive delay or conflict in entering or crossing the major street.

Given that Finley Road provides two-lanes in each approach and Burj Plaza access drive will provide a two-lane approach, the required number of vehicles per hour on the major street is 600 (total of both approaches) and the required number of vehicles per hour on the minor street is 200 (one direction only) for Condition A. For Condition B, the required number of vehicles per hour on the major street is 900 (total of both approaches) and the required number of vehicles per hour on a two-lane minor street is 100 (one direction only).

Table 7 summarizes the future (Year 2031 total projected) major street (Finley Road) and minor street (Burj Plaza access drive) traffic volumes at the intersection. It should be noted that the right-turn volume on the minor street approach (Burj Plaza access drive) was reduced by 60 percent per IDOT's traffic signal warrant requirements.

The following summarizes the results of the traffic signal warrant study based on the Year 2031 total projected conditions:

- Warrant 1A (Eight-Hour Volumes): none of the hourly volumes will satisfy the minimum volume requirement when eight hours are required.
- Warrant 1B (Eight-Hour Volumes) will be met for five hours of the required eight hours. As such, the Year 2031 total projected traffic volumes will not meet Warrant 1B per IDOT requirements.

Based on the above, a traffic signal will not be warranted at the intersection of Finley Road with the Burj Plaza access drive.

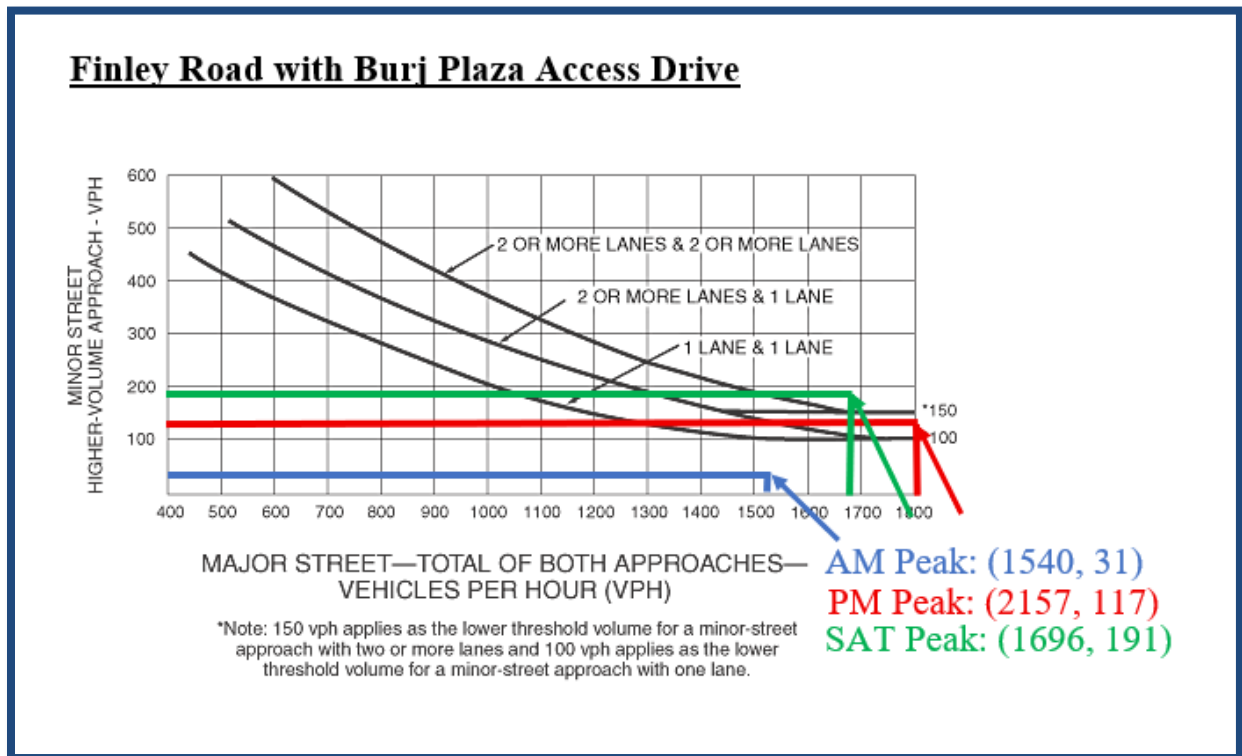
Table 7

FINLEY ROAD WITH BURJ PLAZA ACCESS DRIVE - YEAR 2031 TOTAL PROJECTED TRAFFIC VOLUMES

Time	Major Approach Total (Finley Road)	Minor Approach (Burj Plaza Access Drive)	Single Lane Approach Minimum Met?	
			Condition A	Condition B
7:00 AM	1,323	32	No	No
8:00 AM	1,494	39	No	No
9:00 AM	1,115	30	No	No
10:00 AM	1,157	34	No	No
11:00 AM	1,387	100	No	Yes
12:00 PM	1,479	155	No	Yes
1:00 PM	1,450	152	No	Yes
2:00 PM	1,534	106	No	Yes
3:00 PM	1,726	108	No	Yes
4:00 PM	2,029	96	No	No
5:00 PM	1,970	124	No	Yes
For Burj Plaza Access Drive: Warrant 1A requires a major approach volume of 600 vehicles and a minor approach volume of 200 vehicles. Warrant 1B requires a major approach volume of 900 vehicles and a minor approach volume of 100 vehicles.				

KLOA, Inc. also evaluated whether a traffic signal could be warranted based on Warrant 3 – Peak Hour. Warrant 3 indicates that the plotted point representing the vehicles per hour on the major street (total of both approaches) and the corresponding vehicles per hour on the higher-volume minor-street approach (one direction only) for one hour (any four consecutive 15-minute periods) of an average day falls above the applicable curve in Figure 4C-3 for the combination of approach lanes.

Figure 10 illustrates the projected peak hour volumes on the major and minor roadways versus the warrant thresholds for the intersection of Finley Road with Mariano's access drive/Burj Plaza access drive.



Traffic Signal Warrant Graph – Finley Road with Burj Plaza Access Drive

Figure 10

The results of the analysis indicate that the peak hour warrant is met on Saturday. While it appears that a traffic signal will be warranted based on one peak hour, as stated in the MUTCD, “the satisfaction of a traffic signal warrant or warrants shall not in itself require the installation of a traffic signal control.” Given that only one-hour is potentially met, it is KLOA, Inc’s recommendation that future traffic counts after the development is fully operational be conducted to determine if other warrants are met.

Parking Evaluation

As proposed, the development is to contain the following uses with approximately 688 parking spaces:

- 5,000 square-foot day care center with a maximum population of 72 children
- 3,565 square-foot fast food restaurant with drive-through
- 1,280 square-foot candy shop
- 4,782 square-foot smoking lounge
- 3,624 square feet of retail/service uses
- 2,978 square feet of personal care store
- Approximately 8,562 square feet of fast-food restaurants without drive-through
- Approximately 13,650 square feet of fast-casual restaurants
- Approximately 14,811 square feet of fine-dining restaurants
- Approximately 2,978 square feet of office space
- 60,000 square-foot banquet facility

Based on the Village of Lombard Code of Ordinances, the proposed uses are required to provide parking spaces based on the following rates:

- The day care center is required to provide two parking spaces per 1,000 square feet of gross floor area which translates into approximately ten parking spaces. The day care will be located in the southeast corner of the plaza and has its own parking spaces.
- The fast-food restaurants are required to provide nine parking spaces per 1,000 square feet of gross floor dining area which translates into approximately 74 parking spaces (6,200 square-foot of gross dining area).
- Retail uses within shopping centers are required to provide three parking spaces per 1,000 square feet of gross floor area. As such, the candy shop and the retail spaces should provide approximately 15 parking spaces.
- Given that there is no specific requirement for a smoking establishment, the Village has typically applied in the past the parking requirement for a fast-food restaurant. A review of the architectural plans indicates that the smoking establishment has an indoor seating area of 2,000 square feet plus an additional 668 square feet of outdoor seating area for a total seating area of 2,668 square feet. Based on this, the smoking establishment should provide 24 parking spaces.

- Personal care spaces are required to provide three parking spaces per 1,000 square feet of gross floor area which translates into nine parking spaces.
- Sit-down restaurants are required to provide approximately 12 parking spaces for every 1,000 square feet of gross floor dining area. As such, the fast-casual restaurants and fine-dining restaurants should provide 115 parking spaces in total (9,600 square foot of gross floor dining area).
- Office spaces are required to provide three parking spaces per 1,000 square feet of gross floor area which translates into nine spaces.
- Banquet facilities are required to provide one parking space per three seats plus one space per employee which translates into approximately 360 parking spaces for an average event with 1,000 guests and 523 parking spaces for the full occupancy of the banquet.

As such, the total number of parking spaces required per the Village code will be approximately 606 and 769 parking spaces assuming different occupancy of the banquet. While the proposed number of parking spaces falls short of meeting the Village's parking requirement when assuming a 1,500-guest event, it should be noted that these type of events usually occur on Friday and Saturday nights when most of the other uses within the plaza will be either closed or generating minimal parking demand.

In order to determine the adequacy of the proposed parking supply, an hourly shared parking analysis was conducted based on information published in the ITE *Parking Generation Manual*, 6th Edition. The shared parking tables per use based on the Village of Lombard parking requirements and the ITE hourly distributions are shown in **Tables 8 and 9**.

Table 8

SHARED PARKING ANALYSIS – VILLAGE RATES (1,000 ATTENDEES)

Time	Fast-Food Restaurants	Fine Dining Restaurants	Fast Casual Restaurants	Retail	Hair Salon	Banquet (1,000 attendees)	Smoking Lounge	Office	Total
7:00 AM	0	0	0	0	0	0	0	2	2
8:00 AM	0	43	2	3	2	72	0	5	127
9:00 AM	0	50	5	5	3	72	0	8	143
10:00 AM	15	55	6	8	5	72	0	9	170
11:00 AM	30	60	8	9	5	72	0	9	193
12:00 PM	74	67	36	14	9	72	14	8	294
1:00 PM	69	58	29	15	9	72	13	1	266
2:00 PM	60	39	15	11	7	72	14	9	227
3:00 PM	40	30	12	11	7	143	15	9	267
4:00 PM	25	27	10	10	6	196	16	8	298
5:00 PM	30	42	17	11	7	250	21	6	384
6:00 PM	28	49	28	12	0	356	24	2	499
7:00 PM	15	64	23	11	0	356	24	0	493
8:00 PM	25	51	13	9	0	356	23	0	477
Provided									688

Table 9
SHARED PARKING ANALYSIS – VILLAGE RATES (1,500 ATTENDEES)

Time	Fast-Food Restaurants	Fine Dining Restaurants	Fast Casual Restaurants	Retail	Hair Salon	Banquet (1,500 attendees)	Smoking Lounge	Office	Total
7:00 AM	0	0	0	0	0	0	0	2	2
8:00 AM	0	43	2	3	2	105	0	5	160
9:00 AM	0	50	5	5	3	105	0	8	176
10:00 AM	15	55	6	8	5	105	0	9	203
11:00 AM	30	60	8	9	5	105	0	9	226
12:00 PM	74	67	36	14	9	105	14	8	327
1:00 PM	69	58	29	15	9	105	13	1	299
2:00 PM	60	39	15	11	7	105	14	9	260
3:00 PM	40	30	12	11	7	210	15	9	334
4:00 PM	25	27	10	10	6	288	16	8	390
5:00 PM	30	42	17	11	7	367	21	6	501
6:00 PM	28	49	28	12	0	523	24	2	666
7:00 PM	15	64	23	11	0	523	24	0	660
8:00 PM	25	51	13	9	0	523	23	0	644
Provided									688

As can be seen from Tables 8 and 9, the development is projected to have an overall peak parking demand of 499 parking spaces based on the Village of Lombard ordinances and assuming 1,000 attendees for the banquet and 666 parking spaces based on the Village of Lombard Ordinances and assuming 1,500 attendees for the banquet. This peak parking demand is the result of the variations in the peak accumulation of parked vehicles due to time differences in the activity patterns of adjacent or proximate land uses (by hour, by day, by season). One of the main principles of the shared parking methodology is the relationships among land use activities in a development and as such, the proposed 688 off-street parking spaces will be adequate in accommodating the estimated peak parking demand for the development.

7. Conclusion

Based on the preceding analyses and recommendations, the following conclusions have been made:

The development is to contain the following uses with approximately 688 parking spaces:

- 5,000 square-foot day care center with a maximum population of 72 children
 - 3,565 square-foot fast food restaurant with drive-through
 - 1,280 square-foot candy shop
 - 4,782 square-foot smoking lounge
 - 3,624 square feet of retail/service uses
 - 2,978 square feet of personal care store
 - Approximately 8,562 square feet of fast-food restaurants without drive-through
 - Approximately 13,650 square feet of fast-casual restaurants
 - Approximately 14,811 square feet of fine-dining restaurants
 - Approximately 2,978 square feet of office space
 - 60,000 square-foot banquet facility
- The traffic projected to be generated by Burj Plaza will be reduced due to the volume of pass-by traffic that will be diverted from the existing traffic on the adjacent roadways as well as interaction with the other proposed uses on site.
 - Access to the development will be provided via the existing four access drives described as follows:
 - An existing full movement access drive on the south side of Roosevelt Road located approximately 610 feet west of Finley Road. This access drive provides one inbound lane and two outbound lanes that will be striped for a shared left-turn/through lane and an exclusive right-turn lane. The outbound movements should be under stop sign control.
 - An existing right-in/right-out access drive on the west side of Finley Road located approximately 185 feet south of Roosevelt Road. This access drive provides one inbound lane and one outbound lane. The outbound movement should be under stop sign control.
 - An existing right-in/right-out access drive on the west side of Finley Road located approximately 395 feet south of Roosevelt Road. This access drive provides one inbound lane and one outbound lane. The outbound movement should be under stop sign control.
 - An existing full movement access drive on the west side of Finley Road located approximately 665 feet south of Roosevelt Road. This access drive provides one inbound lane and two outbound lanes that will be striped for an exclusive left-turn lane and a shared through/right-turn lane. The outbound movements should be under stop sign control.

- As part of the development, it is recommended that the existing raised median along Finley Road at its intersection with the Mariano's full access drive/site full access drive be reconstructed to provide an exclusive northbound left-turn lane. This turn lane should provide 145 feet of storage and 175 feet of taper.
- Consideration should be given to consolidating the Mariano's access drive and the existing access road to the Point West condominium development located south of Mariano's in order to provide a more efficient and safer intersection. This is a future consideration that should involve all parties as well as the Village.
- The existing roadway system will generally have sufficient reserve capacity to accommodate the traffic that will be generated by Burj Plaza.
- The proposed access system will be adequate in accommodating the traffic projected to be generated by Burj Plaza.
- While a traffic signal appears to be warranted during one hour on a Saturday, it is KLOA, Inc.'s recommendation that future traffic counts after the development is fully operational be conducted to determine if other warrants are met.
- The number of parking spaces will be adequate to meet the parking needs of Burj Plaza.

Appendix

Traffic Count Summary Sheets

Site Plan

ITE Trip Generation Summary Sheets

CMAP 2050 Projections Letter

Level of Service Criteria

Capacity Analysis Summary Sheets

Traffic Count Summary Sheets



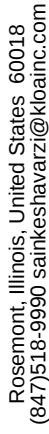
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Count Name: Finley Road with International
Drive TMC
Site Code:
Start Date: 03/27/2025
Page No: 1

Turning Movement Data

Start Time	International Drive Eastbound					Finley Road Northbound					Finley Road Southbound				
	U-Turn	Left	Right	Peds	App. Total	U-Turn	Left	Thru	Peds	App. Total	U-Turn	Thru	Right	Peds	Int. Total
7:00 AM	0	3	7	0	10	0	3	114	0	117	0	133	3	0	263
7:15 AM	0	10	9	0	19	0	2	126	0	128	2	142	2	0	293
7:30 AM	0	10	6	0	16	0	2	158	0	160	6	195	1	0	378
7:45 AM	0	9	13	0	22	0	3	111	0	114	2	232	2	0	372
Hourly Total	0	32	35	0	67	0	10	509	0	519	10	702	8	0	1306
8:00 AM	0	10	9	0	19	0	1	140	0	141	4	205	4	0	373
8:15 AM	0	6	12	0	18	0	4	148	0	152	0	221	5	0	396
8:30 AM	0	8	8	1	16	0	4	158	0	162	4	208	5	0	395
8:45 AM	0	11	7	0	18	0	5	126	0	131	0	200	3	0	352
Hourly Total	0	35	36	1	71	0	14	572	0	586	8	834	17	0	1516
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2:00 PM	0	9	5	0	14	0	4	195	0	199	2	162	6	0	383
2:15 PM	0	14	3	0	17	0	9	185	0	194	1	174	4	0	390
2:30 PM	0	7	8	0	15	0	7	175	0	182	1	159	5	1	362
2:45 PM	0	7	4	0	11	0	9	186	0	195	1	171	2	1	380
Hourly Total	0	37	20	0	57	0	29	741	0	770	5	666	17	2	1515
3:00 PM	0	5	6	0	11	0	7	171	0	178	1	188	5	0	383
3:15 PM	0	8	10	1	18	0	7	225	0	232	2	196	6	0	454
3:30 PM	0	4	5	1	9	0	7	212	1	219	2	179	9	1	418
3:45 PM	0	6	9	0	15	0	8	198	0	206	2	184	4	0	411
Hourly Total	0	23	30	2	53	0	29	806	1	835	7	747	24	1	1666
4:00 PM	0	4	6	0	10	0	11	290	0	301	1	182	3	0	497
4:15 PM	0	6	3	0	9	0	7	212	0	219	0	198	8	0	434
4:30 PM	0	7	6	0	13	0	10	241	0	251	2	202	7	0	475
4:45 PM	0	4	11	1	15	0	8	229	0	237	6	235	12	1	505
Hourly Total	0	21	26	1	47	0	36	972	0	1008	9	817	30	1	1911
5:00 PM	0	9	4	0	13	0	17	260	0	277	4	222	6	0	522
5:15 PM	0	9	3	0	12	0	15	248	0	263	0	189	10	2	474
5:30 PM	0	4	9	0	13	0	12	222	0	234	1	188	10	1	446
5:45 PM	0	10	12	0	22	0	14	178	0	192	3	223	10	0	450
Hourly Total	0	32	28	0	60	0	58	908	0	966	8	822	36	3	1892
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
12:00 PM	0	8	5	0	13	1	5	168	0	174	3	160	5	0	355
12:15 PM	0	10	6	0	16	1	4	199	0	204	2	197	4	1	423
12:30 PM	0	6	5	0	11	0	5	189	0	194	1	167	3	0	376
12:45 PM	1	7	6	2	14	0	9	174	0	183	3	179	6	0	385

Hourly Total	1	31	22	2	2	54	2	23	730	0	755	9	703	18	1	730	1539
1:00 PM	0	6	2	0	0	8	0	4	179	0	183	1	157	1	0	159	350
1:15 PM	0	10	6	2	2	16	0	8	179	0	187	2	146	7	0	155	358
1:30 PM	0	9	5	0	0	14	0	12	168	0	180	3	177	15	0	195	389
1:45 PM	0	1	7	0	0	8	0	8	162	0	170	2	160	11	0	173	351
Hourly Total	0	26	20	2	2	46	0	32	688	0	720	8	640	34	0	682	1448
Grand Total	1	237	217	8	8	455	2	231	5926	1	6159	64	5931	184	8	6179	12793
Approach %	0.2	52.1	47.7	-	-	-	0.0	3.8	96.2	-	-	1.0	96.0	3.0	-	-	-
Total %	0.0	1.9	1.7	-	-	3.6	0.0	1.8	46.3	-	48.1	0.5	46.4	1.4	-	48.3	-
Lights	1	231	203	-	-	435	2	229	5821	-	6052	64	5855	181	-	6100	12587
% Lights	100.0	97.5	93.5	-	-	95.6	100.0	99.1	98.2	-	98.3	100.0	98.7	98.4	-	98.7	98.4
Buses	0	5	10	-	-	15	0	0	23	-	23	0	13	1	-	14	52
% Buses	0.0	2.1	4.6	-	-	3.3	0.0	0.0	0.4	-	0.4	0.0	0.2	0.5	-	0.2	0.4
Single-Unit Trucks	0	0	4	-	-	4	0	2	66	-	68	0	52	2	-	54	126
% Single-Unit Trucks	0.0	0.0	1.8	-	-	0.9	0.0	0.9	1.1	-	1.1	0.0	0.9	1.1	-	0.9	1.0
Articulated Trucks	0	0	0	-	-	0	0	0	16	-	16	0	11	0	-	11	27
% Articulated Trucks	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.3	-	0.3	0.0	0.2	0.0	-	0.2	0.2
Bicycles on Road	0	1	0	-	-	1	0	0	0	-	0	0	0	0	-	0	1
% Bicycles on Road	0.0	0.4	0.0	-	-	0.2	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	-	8	-	-	-	-	-	1	-	-	-	-	8	-	-
% Pedestrians	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	100.0	-	-



Site Code:
Start Date: 03/27/2025
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Turning Movement Peak Hour Data (7:30 AM)

Start Time	International Drive Eastbound						Finley Road Northbound						Finley Road Southbound					
	U-Turn	Left	Right	Peds	App. Total		U-Turn	Left	Thru	Peds	App. Total		U-Turn	Thru	Right	Peds	App. Total	Int. Total
7:30 AM	0	10	6	0	16		0	2	158	0	160		6	195	1	0	202	378
7:45 AM	0	9	13	0	22		0	3	111	0	114		2	232	2	0	236	372
8:00 AM	0	10	9	0	19		0	1	140	0	141		4	205	4	0	213	373
8:15 AM	0	6	12	0	18		0	4	148	0	152		0	221	5	0	226	396
Total	0	35	40	0	75		0	10	557	0	567		12	853	12	0	877	1519
Approach %	0.0	46.7	53.3	-	-		0.0	1.8	98.2	-	-		1.4	97.3	1.4	-	-	-
Total %	0.0	2.3	2.6	-	4.9		0.0	0.7	36.7	-	37.3		0.8	56.2	0.8	-	57.7	-
PHF	0.000	0.875	0.769	-	0.852		0.000	0.625	0.881	-	0.886		0.500	0.919	0.600	-	0.929	0.959
Lights	0	34	35		69		0	10	538	-	548		12	836	10		858	1475
% Lights	-	97.1	87.5	-	92.0		-	100.0	96.6	-	96.6		100.0	98.0	83.3	-	97.8	97.1
Buses	0	1	5	-	6		0	0	5	-	5		0	4	1	-	5	16
% Buses	-	2.9	12.5	-	8.0		-	0.0	0.9	-	0.9		0.0	0.5	8.3	-	0.6	1.1
Single-Unit Trucks	0	0	0	-	0		0	0	12	-	12		0	9	1	-	10	22
% Single-Unit Trucks	-	0.0	0.0	-	0.0		-	0.0	2.2	-	2.1		0.0	1.1	8.3	-	1.1	1.4
Articulated Trucks	0	0	0	-	0		0	0	2	-	2		0	4	0	-	4	6
% Articulated Trucks	-	0.0	0.0	-	0.0		-	0.0	0.4	-	0.4		0.0	0.5	0.0	-	0.5	0.4
Bicycles on Road	0	0	0	-	0		0	0	0	-	0		0	0	0	-	0	0
% Bicycles on Road	-	0.0	0.0	-	0.0		-	0.0	0.0	-	0.0		0.0	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	-	0	-		-	-	-	0	-		-	-	-	0	-	-
% Pedestrians	-	-	-	-	-		-	-	-	-	-		-	-	-	-	-	-



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Turning Movement Peak Hour Data (4:30 PM)

Start Time	International Drive Eastbound						Finley Road Northbound						Finley Road Southbound					
	U-Turn	Left	Right	Peds	App. Total		U-Turn	Left	Thru	Peds	App. Total		U-Turn	Thru	Right	Peds	App. Total	Int. Total
4:30 PM	0	7	6	0	13		0	10	241	0	251		2	202	7	0	211	475
4:45 PM	0	4	11	1	15		0	8	229	0	237		6	235	12	1	253	505
5:00 PM	0	9	4	0	13		0	17	260	0	277		4	222	6	0	232	522
5:15 PM	0	9	3	0	12		0	15	248	0	263		0	189	10	2	199	474
Total	0	29	24	1	53		0	50	978	0	1028		12	848	35	3	895	1976
Approach %	0.0	54.7	45.3	-	-		0.0	4.9	95.1	-	-		1.3	94.7	3.9	-	-	-
Total %	0.000	1.5	1.2	-	2.7		0.0	2.5	49.5	-	52.0		0.6	42.9	1.8	-	45.3	-
PHF	0.000	0.806	0.545	-	0.883		0.000	0.735	0.940	-	0.928		0.500	0.902	0.729	-	0.884	0.946
Lights	0	29	23	-	52		0	49	968	-	1017		12	843	35	-	890	1959
% Lights	-	100.0	95.8	-	98.1		-	98.0	99.0	-	98.9		100.0	99.4	100.0	-	99.4	99.1
Buses	0	0	0	-	0		0	0	0	-	0		0	0	0	-	0	0
% Buses	-	0.0	0.0	-	0.0		-	0.0	0.0	-	0.0		0.0	0.0	0.0	-	0.0	0.0
Single-Unit Trucks	0	0	1	-	1		0	1	5	-	6		0	4	0	-	4	11
% Single-Unit Trucks	-	0.0	4.2	-	1.9		-	2.0	0.5	-	0.6		0.0	0.5	0.0	-	0.4	0.6
Articulated Trucks	0	0	0	-	0		0	0	5	-	5		0	1	0	-	1	6
% Articulated Trucks	-	0.0	0.0	-	0.0		-	0.0	0.5	-	0.5		0.0	0.1	0.0	-	0.1	0.3
Bicycles on Road	0	0	0	-	0		0	0	0	-	0		0	0	0	-	0	0
% Bicycles on Road	-	0.0	0.0	-	0.0		-	0.0	0.0	-	0.0		0.0	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	-	1	-		-	-	-	0	-		-	-	-	3	-	-
% Pedestrians	-	-	-	100.0	-		-	-	-	-	-		-	-	-	100.0	-	-



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Turning Movement Peak Hour Data (12:00 PM)

Start Time	International Drive Eastbound						Finley Road Northbound						Finley Road Southbound					
	U-Turn	Left	Right	Peds	App. Total		U-Turn	Left	Thru	Peds	App. Total		U-Turn	Thru	Right	Peds	App. Total	Int. Total
12:00 PM	0	8	5	0	13		1	5	168	0	174		3	160	5	0	168	355
12:15 PM	0	10	6	0	16		1	4	199	0	204		2	197	4	1	203	423
12:30 PM	0	6	5	0	11		0	5	189	0	194		1	167	3	0	171	376
12:45 PM	1	7	6	2	14		0	9	174	0	183		3	179	6	0	188	385
Total	1	31	22	2	54		2	23	730	0	755		9	703	18	1	730	1539
Approach %	1.9	57.4	40.7	-	-		0.3	3.0	96.7	-	-		1.2	96.3	2.5	-	-	-
Total %	0.1	2.0	1.4	-	3.5		0.1	1.5	47.4	-	49.1		0.6	45.7	1.2	-	47.4	-
PHF	0.250	0.775	0.917	-	0.844		0.500	0.639	0.917	-	0.925		0.750	0.892	0.750	-	0.899	0.910
Lights	1	31	22	-	54		2	23	723	-	748		9	700	18	-	727	1529
% Lights	100.0	100.0	100.0	-	100.0		100.0	100.0	99.0	-	99.1		100.0	99.6	100.0	-	99.6	99.4
Buses	0	0	0	-	0		0	0	0	-	0		0	0	0	-	0	0
% Buses	0.0	0.0	0.0	-	0.0		0.0	0.0	0.0	-	0.0		0.0	0.0	0.0	-	0.0	0.0
Single-Unit Trucks	0	0	0	-	0		0	0	5	-	5		0	3	0	-	3	8
% Single-Unit Trucks	0.0	0.0	0.0	-	0.0		0.0	0.0	0.7	-	0.7		0.0	0.4	0.0	-	0.4	0.5
Articulated Trucks	0	0	0	-	0		0	0	2	-	2		0	0	0	-	0	2
% Articulated Trucks	0.0	0.0	0.0	-	0.0		0.0	0.0	0.3	-	0.3		0.0	0.0	0.0	-	0.0	0.1
Bicycles on Road	0	0	0	-	0		0	0	0	-	0		0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	0.0	-	0.0		0.0	0.0	0.0	-	0.0		0.0	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	-	2	-		-	-	-	0	-		-	-	-	1	-	-
% Pedestrians	-	-	-	100.0	-		-	-	-	-	-		-	-	-	100.0	-	-



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Count Name: Roosevelt Road with Access Drive
TMC
Site Code:
Start Date: 03/27/2025
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Turning Movement Data

Start Time	Roosevelt Road Eastbound						Roosevelt Road Westbound						Access Drive Northbound						Access Drive Southbound							
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	Int. Total	
7:00 AM	0	0	311	0	0	311	0	0	242	0	0	242	0	0	0	0	0	0	0	0	0	0	0	0	553	
7:15 AM	0	3	385	0	0	388	1	2	287	0	0	290	0	0	0	1	0	1	0	0	0	0	0	0	679	
7:30 AM	0	1	419	0	0	420	0	0	365	1	0	366	0	0	0	0	0	0	0	0	0	0	0	0	786	
7:45 AM	0	2	398	1	0	401	0	2	334	1	0	337	0	0	0	0	0	0	0	0	0	0	0	0	738	
Hourly Total	0	6	1513	1	0	1520	1	4	1228	2	0	1235	0	0	0	1	0	1	0	0	0	0	0	0	2756	
8:00 AM	0	1	322	0	0	323	0	0	331	1	0	332	0	0	0	0	0	0	0	0	0	0	0	0	655	
8:15 AM	0	3	362	0	0	365	0	1	321	3	0	325	0	0	0	0	0	0	0	0	0	0	0	0	690	
8:30 AM	0	4	384	0	0	388	0	0	374	1	0	375	0	0	0	0	0	0	0	0	0	0	0	0	763	
8:45 AM	1	7	333	0	0	341	0	5	280	1	0	286	0	0	0	0	0	0	0	0	0	0	0	0	627	
Hourly Total	1	15	1401	0	0	1417	0	6	1306	6	0	1318	0	0	0	0	0	0	0	0	0	0	0	0	2735	
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
2:00 PM	1	0	317	1	0	319	0	2	303	3	0	308	0	1	0	3	1	4	0	0	0	1	1	1	632	
2:15 PM	0	1	336	1	0	338	1	2	328	1	0	332	0	0	0	0	0	0	0	0	0	0	0	0	670	
2:30 PM	1	0	329	0	0	330	0	1	285	1	1	287	0	1	0	0	0	1	0	0	0	0	1	0	618	
2:45 PM	2	1	349	1	0	353	0	0	351	1	0	352	0	0	0	2	1	2	0	0	0	0	0	0	707	
Hourly Total	4	2	1331	3	0	1340	1	5	1267	6	1	1279	0	2	0	5	2	7	0	0	0	1	2	1	1	2627
3:00 PM	0	0	359	1	0	360	0	1	425	0	0	426	0	0	0	2	0	2	0	0	0	0	0	0	788	
3:15 PM	0	0	357	0	0	357	0	6	413	4	0	423	0	0	0	2	0	2	0	0	0	0	0	0	782	
3:30 PM	0	5	301	0	0	306	0	3	440	0	0	443	0	0	0	5	0	5	0	0	0	0	1	0	754	
3:45 PM	0	1	356	0	0	357	2	1	383	2	0	388	0	0	0	1	0	1	0	0	0	1	0	1	747	
Hourly Total	0	6	1373	1	0	1380	2	11	1661	6	0	1680	0	0	0	10	0	10	0	0	0	1	1	1	1	3071
4:00 PM	0	5	383	0	0	388	0	2	453	1	0	456	0	1	0	1	0	2	0	1	0	1	0	2	848	
4:15 PM	0	3	376	1	0	380	0	0	403	2	0	405	0	0	0	1	1	1	0	0	0	0	0	0	786	
4:30 PM	0	0	383	1	0	384	0	1	384	0	0	385	0	0	0	1	0	1	0	0	0	0	0	0	770	
4:45 PM	0	3	368	1	0	372	0	1	451	3	0	455	0	0	0	1	1	1	0	0	0	1	0	1	829	
Hourly Total	0	11	1510	3	0	1524	0	4	1691	6	0	1701	0	1	0	4	2	5	0	1	0	2	0	3	3233	
5:00 PM	0	3	357	2	0	362	0	1	441	0	0	442	0	0	0	2	0	2	0	0	0	0	0	0	806	
5:15 PM	0	3	398	0	0	401	0	1	412	0	0	413	0	0	0	1	0	1	0	1	0	0	0	1	816	
5:30 PM	0	0	351	0	0	351	0	1	392	0	0	393	0	0	0	0	0	0	0	0	0	1	0	1	745	
5:45 PM	0	1	353	0	0	354	0	1	381	1	0	383	0	0	0	1	0	1	0	0	0	0	0	0	738	
Hourly Total	0	7	1459	2	0	1468	0	4	1626	1	0	1631	0	0	0	4	0	4	0	1	0	1	0	2	1	3105
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
12:00 PM	0	4	327	0	0	331	0	0	298	0	0	298	0	0	0	2	0	2	0	1	0	0	0	1	632	
12:15 PM	0	22	325	0	0	347	0	4	353	0	0	357	0	0	0	0	0	0	0	0	0	0	0	0	704	
12:30 PM	0	2	353	0	0	355	0	2	333	2	0	337	0	0	0	2	0	2	0	0	0	0	0	0	694	



Count Name: Roosevelt Road with Access Drive
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Turning Movement Peak Hour Data (7:30 AM)

[illegible]



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Count Name: Roosevelt Road with Access Drive
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Turning Movement Peak Hour Data (4:30 PM)

Start Time	Roosevelt Road Eastbound						Roosevelt Road Westbound						Access Drive Northbound						Access Drive Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
4:30 PM	0	0	383	1	0	384	0	1	384	0	0	385	0	0	0	1	1	0	1	0	0	0	0	0	770
4:45 PM	0	3	368	1	0	372	0	1	451	3	0	455	0	0	0	1	1	1	0	0	0	1	0	1	829
5:00 PM	0	3	357	2	0	362	0	1	441	0	0	442	0	0	0	2	0	2	0	0	0	0	0	0	806
5:15 PM	0	3	398	0	0	401	0	1	412	0	0	413	0	0	0	1	0	1	0	1	0	0	0	1	816
Total	0	9	1506	4	0	1519	0	4	1688	3	0	1695	0	0	0	5	1	5	0	1	0	1	0	2	3221
Approach %	0.0	0.6	99.1	0.3	-	-	0.0	0.2	99.6	0.2	-	-	0.0	0.0	0.0	100.0	-	-	0.0	50.0	0.0	50.0	-	-	-
Total %	0.0	0.3	46.8	0.1	-	47.2	0.0	0.1	52.4	0.1	-	52.6	0.0	0.0	0.0	0.2	-	0.2	0.0	0.0	0.0	0.0	-	0.1	-
PHF	0.000	0.750	0.946	0.500	-	0.947	0.000	1.000	0.936	0.250	-	0.931	0.000	0.000	0.000	0.625	-	0.625	0.000	0.250	0.000	0.250	-	0.500	0.971
Lights	0	9	1491	4	-	1504	0	4	1671	3	-	1678	0	0	0	5	-	5	0	1	0	1	-	2	3189
% Lights	-	100.0	99.0	100.0	-	99.0	-	100.0	99.0	100.0	-	99.0	-	-	-	100.0	-	100.0	-	100.0	-	100.0	-	100.0	99.0
Buses	0	0	6	0	-	6	0	0	1	0	-	1	0	0	0	0	-	0	0	0	0	0	-	0	7
% Buses	-	0.0	0.4	0.0	-	0.4	-	0.0	0.1	0.0	-	0.1	-	-	-	0.0	-	0.0	-	0.0	-	0.0	-	0.0	0.2
Single-Unit Trucks	0	0	5	0	-	5	0	0	10	0	-	10	0	0	0	0	-	0	0	0	0	0	-	0	15
% Single-Unit Trucks	-	0.0	0.3	0.0	-	0.3	-	0.0	0.6	0.0	-	0.6	-	-	-	0.0	-	0.0	-	0.0	-	0.0	-	0.0	0.5
Articulated Trucks	0	0	4	0	-	4	0	0	6	0	-	6	0	0	0	0	-	0	0	0	0	0	-	0	10
% Articulated Trucks	-	0.0	0.3	0.0	-	0.3	-	0.0	0.4	0.0	-	0.4	-	-	-	0.0	-	0.0	-	0.0	-	0.0	-	0.0	0.3
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	-	-	0.0	-	0.0	-	0.0	-	0.0	-	0.0	0.0
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	1	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	-	-	-



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Count Name: Roosevelt Road with Access Drive
TMC
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Turning Movement Peak Hour Data (12:00 PM)

Start Time	Roosevelt Road Eastbound						Roosevelt Road Westbound						Access Drive Northbound						Access Drive Southbound						Int. Total	
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total		
12:00 PM	0	4	327	0	0	331	0	0	298	0	0	298	0	0	0	2	0	2	0	1	0	0	0	0	1	632
12:15 PM	0	22	325	0	0	347	0	4	353	0	0	357	0	0	0	0	0	0	0	0	0	0	0	0	0	704
12:30 PM	0	2	353	0	0	355	0	2	333	2	0	337	0	0	0	2	0	2	0	0	0	0	0	0	0	694
12:45 PM	0	1	342	3	0	346	0	2	314	0	1	316	0	0	0	1	1	1	0	0	0	0	2	0	0	663
Total	0	29	1347	3	0	1379	0	8	1298	2	1	1308	0	0	0	5	1	5	0	1	0	0	2	1	1	2693
Approach %	0.0	2.1	97.7	0.2	-	-	0.0	0.6	99.2	0.2	-	-	0.0	0.0	0.0	100.0	-	-	0.0	100.0	0.0	0.0	0.0	-	-	-
Total %	0.0	1.1	50.0	0.1	-	51.2	0.0	0.3	48.2	0.1	-	48.6	0.0	0.0	0.0	0.2	-	0.2	0.0	0.0	0.0	0.0	-	0.0	0.0	-
PHF	0.000	0.330	0.954	0.250	-	0.971	0.000	0.500	0.919	0.250	-	0.916	0.000	0.000	0.000	0.625	-	0.625	0.000	0.250	0.000	0.000	-	0.250	0.956	-
Lights	0	29	1334	3	-	1366	0	7	1281	2	-	1290	0	0	0	5	-	5	0	1	0	0	-	1	1	2662
% Lights	-	100.0	99.0	100.0	-	99.1	-	87.5	98.7	100.0	-	98.6	-	-	-	100.0	-	100.0	-	100.0	-	-	-	-	100.0	98.8
Buses	0	0	1	0	-	1	0	0	2	0	-	2	0	0	0	0	-	0	0	0	0	0	-	0	0	3
% Buses	-	0.0	0.1	0.0	-	0.1	-	0.0	0.2	0.0	-	0.2	-	-	-	0.0	-	0.0	-	0.0	-	-	-	-	0.0	0.1
Single-Unit Trucks	0	0	8	0	-	8	0	0	11	0	-	11	0	0	0	0	-	0	0	0	0	0	-	0	0	19
% Single-Unit Trucks	-	0.0	0.6	0.0	-	0.6	-	0.0	0.8	0.0	-	0.8	-	-	-	0.0	-	0.0	-	0.0	-	-	-	-	0.0	0.7
Articulated Trucks	0	0	4	0	-	4	0	0	3	0	-	3	0	0	0	0	-	0	0	0	0	0	-	0	0	7
% Articulated Trucks	-	0.0	0.3	0.0	-	0.3	-	0.0	0.2	0.0	-	0.2	-	-	-	0.0	-	0.0	-	0.0	-	-	-	-	0.0	0.3
Bicycles on Road	0	0	0	0	-	0	0	1	1	0	-	2	0	0	0	0	-	0	0	0	0	0	-	0	0	2
% Bicycles on Road	-	0.0	0.0	0.0	-	0.0	-	12.5	0.1	0.0	-	0.2	-	-	-	0.0	-	0.0	-	0.0	-	-	-	-	0.0	0.1
Pedestrians	-	-	-	-	0	-	-	-	-	-	1	-	-	-	-	-	-	1	-	-	-	-	2	-	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-



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Count Name: Roosevelt Road with Finley Road
TMC
Site Code:
Start Date: 03/27/2025
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Turning Movement Data

Start Time	Roosevelt Road Eastbound						Roosevelt Road Westbound						Finley Road Northbound						Finley Road Southbound						
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	Int. Total
7:00 AM	0	22	228	38	0	288	0	35	173	18	0	226	0	48	48	29	0	125	0	35	60	21	0	116	755
7:15 AM	0	37	305	57	1	399	0	32	213	29	0	274	0	58	57	33	0	148	0	24	46	24	0	94	915
7:30 AM	0	40	282	57	0	379	0	25	267	27	1	319	0	66	68	51	0	185	0	54	101	35	0	190	1073
7:45 AM	0	37	298	85	0	420	0	46	266	26	0	338	0	46	47	30	0	123	0	23	89	29	0	141	1022
Hourly Total	0	136	1113	237	1	1486	0	138	919	100	1	1157	0	218	220	143	0	581	0	136	296	109	0	541	3765
8:00 AM	0	16	208	90	0	314	0	37	231	21	0	289	0	71	49	42	0	162	0	32	76	19	0	127	892
8:15 AM	0	22	241	81	0	344	0	35	225	32	2	292	0	53	53	35	0	141	0	31	101	22	4	154	931
8:30 AM	0	18	301	92	0	411	0	34	275	25	1	334	0	76	41	37	0	154	0	34	71	32	0	137	1036
8:45 AM	0	11	216	80	2	307	0	30	202	26	0	258	0	47	55	53	0	155	0	38	90	22	0	150	870
Hourly Total	0	67	966	343	2	1376	0	136	933	104	3	1173	0	247	198	167	0	612	0	135	338	95	4	568	3729
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2:00 PM	0	19	250	64	0	333	0	42	239	29	1	310	0	74	48	52	0	174	0	28	56	16	0	100	917
2:15 PM	0	23	236	68	0	327	0	42	254	37	1	333	1	64	64	46	0	175	0	27	65	17	0	109	944
2:30 PM	0	23	243	59	0	325	0	42	231	28	1	301	0	67	79	51	1	197	0	41	65	20	0	126	949
2:45 PM	0	26	256	68	0	350	0	43	234	36	0	313	0	95	65	49	0	209	0	33	67	24	0	124	996
Hourly Total	0	91	985	259	0	1335	0	169	958	130	3	1257	1	300	256	198	1	755	0	129	253	77	0	459	3806
3:00 PM	0	25	252	74	0	351	0	52	303	28	1	383	0	71	65	47	0	183	0	50	71	44	0	165	1082
3:15 PM	0	33	250	78	0	361	0	50	284	35	1	369	0	92	74	66	0	232	0	41	70	33	0	144	1106
3:30 PM	0	28	219	68	1	315	0	53	285	35	2	373	0	93	72	44	0	209	0	58	75	45	0	178	1075
3:45 PM	0	18	276	72	0	366	0	48	304	28	0	380	0	83	69	53	0	205	0	58	71	27	0	156	1107
Hourly Total	0	104	997	292	1	1393	0	203	1176	126	4	1505	0	339	280	210	0	829	0	207	287	149	0	643	4370
4:00 PM	0	33	280	72	2	385	0	46	284	40	0	370	0	118	103	75	0	296	0	41	62	44	0	147	1198
4:15 PM	0	26	267	72	0	365	0	52	273	38	0	363	0	98	78	50	0	226	0	41	84	30	0	155	1109
4:30 PM	0	26	294	75	0	395	0	51	255	21	0	327	0	106	77	55	0	238	0	50	89	30	0	169	1129
4:45 PM	0	25	257	82	0	364	0	55	312	26	0	393	0	103	76	64	0	243	0	43	102	41	0	186	1186
Hourly Total	0	110	1098	301	2	1509	0	204	1124	125	0	1453	0	425	334	244	0	1003	0	175	337	145	0	657	4622
5:00 PM	0	23	248	74	0	345	0	53	301	27	0	381	0	107	112	59	0	278	0	63	96	24	0	183	1187
5:15 PM	0	26	318	72	0	416	0	49	275	30	3	354	0	104	76	53	0	233	0	42	84	23	0	149	1152
5:30 PM	0	25	262	68	0	355	0	52	249	29	0	330	0	92	84	58	0	234	0	30	78	22	0	130	1049
5:45 PM	0	22	268	82	0	372	1	61	263	28	1	353	0	67	81	45	0	193	0	42	85	30	0	157	1075
Hourly Total	0	96	1096	296	0	1488	1	215	1088	114	4	1418	0	370	353	215	0	938	0	177	343	99	0	619	4463
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
12:00 PM	0	21	259	62	0	342	0	44	255	36	0	335	0	46	61	48	0	155	0	21	50	12	0	83	915
12:15 PM	0	22	214	71	0	307	0	50	247	29	0	326	0	102	67	59	0	228	1	25	81	21	0	128	989
12:30 PM	1	16	292	57	0	366	2	51	260	20	3	333	0	65	62	50	0	177	1	33	56	24	0	114	990

12:45 PM	0	20	261	69	3	350	0	52	204	31	4	287	0	81	54	42	0	177	0	42	63	26	0	131	945
Hourly Total	1	79	1026	259	3	1365	2	197	966	116	7	1281	0	294	244	199	0	737	2	121	250	83	0	456	3839
1:00 PM	0	18	206	49	0	273	0	38	197	18	0	253	1	81	53	60	0	195	0	26	55	21	0	102	823
1:15 PM	0	13	252	56	0	321	0	43	271	32	2	346	0	69	50	47	0	166	0	46	49	22	0	117	950
1:30 PM	0	23	240	68	0	331	0	67	208	25	0	300	0	65	64	56	0	185	1	23	47	18	0	89	905
1:45 PM	0	25	285	69	0	379	0	55	240	29	0	324	0	65	44	38	0	147	0	27	47	19	0	93	943
Hourly Total	0	79	983	242	0	1304	0	203	916	104	2	1223	1	280	211	201	0	693	1	122	198	80	0	401	3621
Grand Total	1	762	8264	2229	9	11256	3	1465	8080	919	24	10467	2	2473	2096	1577	1	6148	3	1202	2302	837	4	4344	32215
Approach %	0.0	6.8	73.4	19.8	-	-	0.0	14.0	77.2	8.8	-	-	0.0	40.2	34.1	25.7	-	-	0.1	27.7	53.0	19.3	-	-	-
Total %	0.0	2.4	25.7	6.9	-	34.9	0.0	4.5	25.1	2.9	-	32.5	0.0	7.7	6.5	4.9	-	19.1	0.0	3.7	7.1	2.6	-	13.5	-
Lights	1	734	8083	2185	-	11003	3	1452	7893	901	-	10249	2	2423	2069	1551	-	6045	3	1177	2272	806	-	4258	31555
% Lights	100.0	96.3	97.8	98.0	-	97.8	100.0	99.1	97.7	98.0	-	97.9	100.0	98.0	98.7	98.4	-	98.3	100.0	97.9	98.7	96.3	-	98.0	98.0
Buses	0	13	57	8	-	78	0	5	53	8	-	66	0	14	10	4	-	28	0	13	5	18	-	36	208
% Buses	0.0	1.7	0.7	0.4	-	0.7	0.0	0.3	0.7	0.9	-	0.6	0.0	0.6	0.5	0.3	-	0.5	0.0	1.1	0.2	2.2	-	0.8	0.6
Single-Unit Trucks	0	11	66	29	-	106	0	7	86	8	-	101	0	30	14	14	-	58	0	10	17	10	-	37	302
% Single-Unit Trucks	0.0	1.4	0.8	1.3	-	0.9	0.0	0.5	1.1	0.9	-	1.0	0.0	1.2	0.7	0.9	-	0.9	0.0	0.8	0.7	1.2	-	0.9	0.9
Articulated Trucks	0	4	58	7	-	69	0	1	48	1	-	50	0	6	3	7	-	16	0	2	6	3	-	11	146
% Articulated Trucks	0.0	0.5	0.7	0.3	-	0.6	0.0	0.1	0.6	0.1	-	0.5	0.0	0.2	0.1	0.4	-	0.3	0.0	0.2	0.3	0.4	-	0.3	0.5
Bicycles on Road	0	0	0	0	-	0	0	0	0	1	-	1	0	0	0	1	-	1	0	0	2	0	-	2	4
% Bicycles on Road	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	0.1	-	0.0	0.0	0.0	0.0	0.1	-	0.0	0.0	0.0	0.1	0.0	-	0.0	0.0
Pedestrians	-	-	-	-	9	-	-	-	-	-	24	-	-	-	-	-	1	-	-	-	-	-	4	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-



Site Code:
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Turning Movement Peak Hour Data (7:30 AM)

[illegible]



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Count Name: Roosevelt Road with Finley Road
TMC
Site Code:
Start Date: 03/27/2025
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Turning Movement Peak Hour Data (4:30 PM)

Start Time	Roosevelt Road Eastbound						Roosevelt Road Westbound						Finley Road Northbound						Finley Road Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
4:30 PM	0	26	294	75	0	395	0	51	255	21	0	327	0	106	77	55	0	238	0	50	89	30	0	169	1129
4:45 PM	0	25	257	82	0	364	0	55	312	26	0	393	0	103	76	64	0	243	0	43	102	41	0	186	1186
5:00 PM	0	23	248	74	0	345	0	53	301	27	0	381	0	107	112	59	0	278	0	63	96	24	0	183	1187
5:15 PM	0	26	318	72	0	416	0	49	275	30	3	354	0	104	76	53	0	233	0	42	84	23	0	149	1152
Total	0	100	1117	303	0	1520	0	208	1143	104	3	1455	0	420	341	231	0	992	0	198	371	118	0	687	4654
Approach %	0.0	6.6	73.5	19.9	-	-	0.0	14.3	78.6	7.1	-	-	0.0	42.3	34.4	23.3	-	-	0.0	28.8	54.0	17.2	-	-	-
Total %	0.0	2.1	24.0	6.5	-	32.7	0.0	4.5	24.6	2.2	-	31.3	0.0	9.0	7.3	5.0	-	21.3	0.0	4.3	8.0	2.5	-	14.8	-
PHF	0.000	0.962	0.878	0.924	-	0.913	0.000	0.945	0.916	0.867	-	0.926	0.000	0.981	0.761	0.902	-	0.892	0.000	0.786	0.909	0.720	-	0.923	0.980
Lights	0	99	1106	300	-	1505	0	208	1133	102	-	1443	0	415	338	228	-	981	0	198	368	117	-	683	4612
% Lights	-	99.0	99.0	99.0	-	99.0	-	100.0	99.1	98.1	-	99.2	-	98.8	99.1	98.7	-	98.9	-	100.0	99.2	99.2	-	99.4	99.1
Buses	0	0	6	0	-	6	0	0	1	0	-	1	0	0	0	0	-	0	0	0	0	0	-	0	7
% Buses	-	0.0	0.5	0.0	-	0.4	-	0.0	0.1	0.0	-	0.1	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.2
Single-Unit Trucks	0	0	2	2	-	4	0	0	4	2	-	6	0	3	2	2	-	7	0	0	1	1	-	2	19
% Single-Unit Trucks	-	0.0	0.2	0.7	-	0.3	-	0.0	0.3	1.9	-	0.4	-	0.7	0.6	0.9	-	0.7	-	0.0	0.3	0.8	-	0.3	0.4
Articulated Trucks	0	1	3	1	-	5	0	0	5	0	-	5	0	2	1	1	-	4	0	0	1	0	-	1	15
% Articulated Trucks	-	1.0	0.3	0.3	-	0.3	-	0.0	0.4	0.0	-	0.3	-	0.5	0.3	0.4	-	0.4	-	0.0	0.3	0.0	-	0.1	0.3
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	1	0	-	1	1
% Bicycles on Road	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.3	0.0	-	0.1	0.0
Pedestrians	-	-	-	-	0	-	-	-	-	-	3	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	-	-	-	-	-	-	-	-



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Count Name: Roosevelt Road with Finley Road
TMC
Site Code:
Start Date: 03/27/2025
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Turning Movement Peak Hour Data (12:00 PM)

Start Time	Roosevelt Road Eastbound							Roosevelt Road Westbound							Finley Road Northbound							Finley Road Southbound							Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total		U-Turn	Left	Thru	Right	Peds	App. Total		U-Turn	Left	Thru	Right	Peds	App. Total		U-Turn	Left	Thru	Right	Peds	App. Total		
12:00 PM	0	21	259	62	0	342		0	44	255	36	0	335		0	46	61	48	0	155		0	21	50	12	0	83	915	
12:15 PM	0	22	214	71	0	307		0	50	247	29	0	326		0	102	67	59	0	228		1	25	81	21	0	128	989	
12:30 PM	1	16	292	57	0	366		2	51	260	20	3	333		0	65	62	50	0	177		1	33	56	24	0	114	990	
12:45 PM	0	20	261	69	3	350		0	52	204	31	4	287		0	81	54	42	0	177		0	42	63	26	0	131	945	
Total	1	79	1026	259	3	1365		2	197	966	116	7	1281		0	294	244	199	0	737		2	121	250	83	0	456	3839	
Approach %	0.1	5.8	75.2	19.0	-	-		0.2	15.4	75.4	9.1	-	-		0.0	39.9	33.1	27.0	-	-		0.4	26.5	54.8	18.2	-	-	-	
Total %	0.0	2.1	26.7	6.7	-	35.6		0.1	5.1	25.2	3.0	-	33.4		0.0	7.7	6.4	5.2	-	19.2		0.1	3.2	6.5	2.2	-	11.9	-	
PHF	0.250	0.898	0.878	0.912	-	0.932		0.250	0.947	0.929	0.806	-	0.956		0.000	0.721	0.910	0.843	-	0.808		0.500	0.720	0.772	0.798	-	0.870	0.969	
Lights	1	77	1017	258	-	1353		2	196	953	115	-	1266		0	293	241	196	-	730		2	121	250	82	-	455	3804	
% Lights	100.0	97.5	99.1	99.6	-	99.1		100.0	99.5	98.7	99.1	-	98.8		-	99.7	98.8	98.5	-	99.1		100.0	100.0	100.0	98.8	-	99.8	99.1	
Buses	0	0	1	0	-	1		0	0	1	0	-	1		0	0	0	0	-	0		0	0	0	1	-	1	3	
% Buses	0.0	0.0	0.1	0.0	-	0.1		0.0	0.0	0.1	0.0	-	0.1		-	0.0	0.0	0.0	-	0.0		0.0	0.0	0.0	1.2	-	0.2	0.1	
Single-Unit Trucks	0	2	4	1	-	7		0	1	8	1	-	10		0	1	3	2	-	6		0	0	0	0	-	0	23	
% Single-Unit Trucks	0.0	2.5	0.4	0.4	-	0.5		0.0	0.5	0.8	0.9	-	0.8		-	0.3	1.2	1.0	-	0.8		0.0	0.0	0.0	0.0	-	0.0	0.6	
Articulated Trucks	0	0	4	0	-	4		0	0	4	0	-	4		0	0	0	0	-	0		0	0	0	0	-	0	8	
% Articulated Trucks	0.0	0.0	0.4	0.0	-	0.3		0.0	0.0	0.4	0.0	-	0.3		-	0.0	0.0	0.0	-	0.0		0.0	0.0	0.0	0.0	-	0.0	0.2	
Bicycles on Road	0	0	0	0	-	0		0	0	0	0	-	0		0	0	0	1	-	1		0	0	0	0	-	0	1	
% Bicycles on Road	0.0	0.0	0.0	0.0	-	0.0		0.0	0.0	0.0	0.0	-	0.0		-	0.0	0.0	0.5	-	0.1		0.0	0.0	0.0	0.0	-	0.0	0.0	
Pedestrians	-	-	-	-	3	-		-	-	-	-	7	-		-	-	-	-	0	-		-	-	-	-	0	-	-	
% Pedestrians	-	-	-	-	100.0	-		-	-	-	-	100.0	-		-	-	-	-	-	-		-	-	-	-	-	-	-	

Study Name Finley Road with Marianos Access Drive TMC
Start Date Thursday, March 27, 2025 7:00 AM
End Date Saturday, March 29, 2025 2:00 PM
Site Code

Report Summary

		Eastbound								Westbound								Northbound								Southbound								Northwestbound								Crosswalk			
Time Period	Class.	U	L	T	BR	R	I	O	U	HL	L	T	R	I	O	U	L	T	R	HR	I	O	U	L	BL	T	R	I	O	U	HL	BL	BR	HR	I	O	Total	EB	Westria	Total					
Peak 1	Lights	0	0	0	0	2	2	1	0	0	27	0	3	30	32	0	1	569	22	3	595	849	0	10	9	803	0	822	593	0	17	0	21	0	38	12	1487	EB	0	0					
Specified Period	%	0%	0%	0%	0%	50%	50%	100%	0%	0%	96%	0%	75%	94%	100%	0%	100%	98%	100%	100%	98%	98%	0%	100%	90%	98%	0%	98%	98%	0%	100%	0%	100%	0%	100%	92%	98%	0%	0	0					
7:30 AM - 8:30 AM	Buses	0	0	0	0	1	1	0	0	0	1	0	0	1	0	0	0	6	0	0	6	5	0	0	1	3	0	4	6	0	0	0	0	0	0	0	1	12	WB	1	1				
One Hour Peak	%	0%	0%	0%	0%	25%	25%	0%	0%	0%	4%	0%	0%	3%	0%	0%	0%	1%	0%	0%	1%	1%	0%	0%	10%	0%	0%	0%	1%	0%	0%	0%	0%	0%	8%	1%	100%	0	0						
7:30 AM - 8:30 AM	igle-Unit Truc	0	0	0	0	0	0	0	0	0	0	0	1	1	0	0	0	6	0	0	6	10	0	0	0	10	0	10	7	0	0	0	0	0	0	0	0	17	NB	0	0				
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	25%	3%	0%	0%	0%	1%	0%	0%	1%	1%	0%	0%	0%	1%	0%	1%	1%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%	0	0				
	ticulated Truc	0	0	0	0	1	1	0	0	0	0	0	0	0	0	0	0	2	0	0	2	4	0	0	0	3	0	3	2	0	0	0	0	0	0	0	0	6	SB	0	0				
	%	0%	0%	0%	0%	25%	25%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0	0				
	cycles on Ro	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	NWB	2	2			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	100%	0	0			
	Total	0	0	0	0	4	4	1	0	0	28	0	4	32	32	0	1	583	22	3	609	868	0	10	10	819	0	839	608	0	17	0	21	0	38	13	1522	3	3						
	PHF	0	0	0	0	1	1	0.25	0	0	0.88	0	0.5	0.89	0.89	0	0.25	0.87	0.69	0.38	0.88	0.94	0	0.62	0.5	0.94	0	0.93	0.86	0	0.61	0	0.75	0	0.79	0.65	0.97								
	Approach %						0%	0%						2%	2%						40%	57%						55%	40%					2%	1%										
Peak 2	Lights	0	0	0	0	1	1	4	0	1	44	2	8	55	55	0	2	966	49	14	1031	892	0	6	19	844	0	869	989	0	3	0	15	0	18	34	1974	EB	0	0					
Specified Period	%	0%	0%	0%	0%	100%	100%	100%	0%	100%	100%	100%	100%	100%	98%	0%	100%	99%	100%	100%	99%	100%	0%	86%	100%	100%	0%	99%	99%	0%	100%	0%	94%	0%	95%	100%	99%	0%	0	0					
4:30 PM - 5:30 PM	Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	WB	9	9				
One Hour Peak	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0	100%					
4:30 PM - 5:30 PM	igle-Unit Truc	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	4	0	0	4	3	0	1	0	3	0	4	5	0	0	0	1	0	1	0	9	NB	1	1					
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	2%	0%	0%	0%	0%	0%	0%	0%	0%	0%	14%	0%	0%	0%	0%	1%	0%	0%	0%	6%	0%	5%	0%	0%	100%						
	ticulated Truc	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	4	1	0	0	0	1	0	1	4	0	0	0	0	0	0	0	5	SB	0	0					
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0	0					
	cycles on Ro	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	NWB	14	14			
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	100%	0	0			
	Total	0	0	0	0	1	1	4	0	1	44	2	8	55	56	0	2	974	49	14	1039	896	0	7	19	848	0	874	998	0	3	0	16	0	19	34	1988	24	24						
	PHF	0	0	0	0	0.25	0.25	1	0	0.25	0.92	0.5	1	0.92	0.64	0	0.5	0.93	0.68	0.7	0.94	0.89	0	0.44	0.68	0.87	0	0.87	0.93	0	0.38	0	0.67	0	0.79	0.77	0.97								
	Approach %						0%	0%						3%	3%						52%	45%						44%	50%					1%	2%										

Study Name	Finley Road with Marianos Access Drive TMC
Start Date	Thursday, March 27, 2025 7:00 AM
End Date	Saturday, March 29, 2025 2:00 PM
Site Code	

Time Period	Class.	Eastbound										Westbound										Northbound										Southbound										Northwestbound										Crosswalk	
		U	L	T	BR	R	I	O	U	HL	L	T	R	I	O	U	L	T	R	HR	I	O	U	L	BL	T	R	I	O	U	HL	BL	BR	HR	I	O	Total	desria	Total														
Peak 1	Lights	0	0	0	0	2	2	3	0	1	42	1	7	51	53	0	2	713	32	4	751	720	2	19	8	670	0	699	743	0	6	0	21	2	29	13	1532	EB	0	0													
Specified Period	%	0%	0%	0%	0%	100%	100%	100%	0%	100%	100%	100%	88%	98%	100%	0%	100%	99%	100%	100%	99%	100%	100%	100%	100%	100%	100%	99%	0%	100%	0%	100%	100%	100%	100%	99%	0%	0	0														
12:00 PM - 1:00 PM	Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	WB	8	8													
One Hour Peak	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	100%															
12:00 PM - 1:00 PM	igle-Unit Truc	0	0	0	0	0	0	0	0	0	0	0	1	1	0	0	0	4	0	0	4	2	0	0	0	2	0	2	5	0	0	0	0	0	0	0	7	NB	3	3													
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	13%	2%	0%	0%	0%	1%	0%	0%	1%	0%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	100%															
	ticultated Truc	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2	0	0	0	0	0	0	2	0	0	0	0	0	0	0	0	2	SB	0	0													
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%															
	ycles on Ro:	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	NWB	7	7													
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	100%														
	Total	0	0	0	0	2	2	3	0	1	42	1	8	52	53	0	2	719	32	4	757	722	2	19	8	672	0	701	750	0	6	0	21	2	29	13	1541	100%	18														
	PHF	0	0	0	0	0.5	0.38	0	0.25	0.81	0.25	0.5	0.87	0.74	0	0.5	0.93	0.8	1	0.93	0.86	0.25	0.59	0.4	0.84	0	0.85	0.92	0	0.3	0	0.75	0.5	0.72	0.54	0.9																	
	Approach %						0%	0%						3%	3%						49%	47%						45%	49%					2%	1%																		

Site Plan

ITE Trip Generation Summary Sheets

Day Care Center (565)

Vehicle Trip Ends vs: Students

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban

Number of Studies: 75

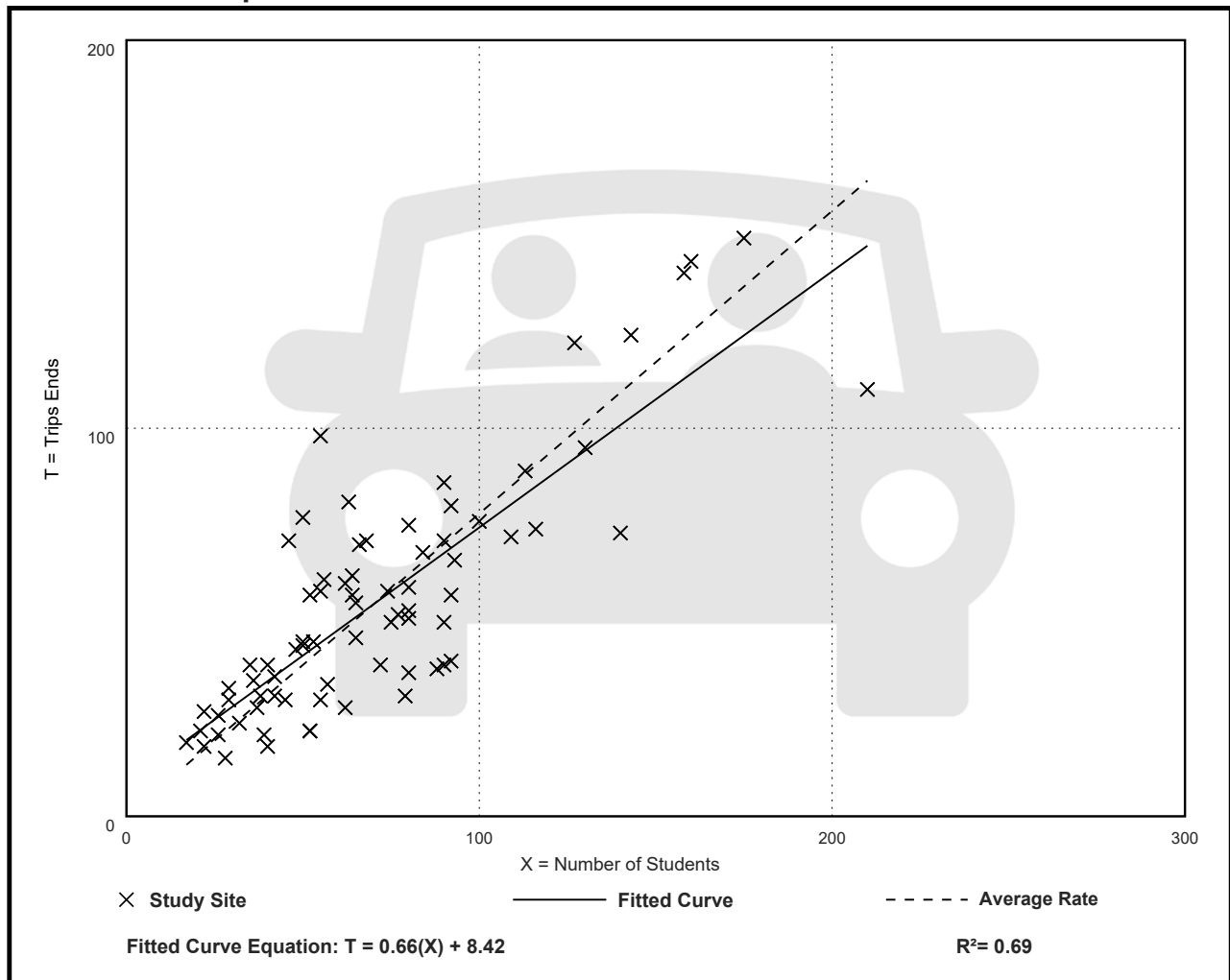
Avg. Num. of Students: 71

Directional Distribution: 53% entering, 47% exiting

Vehicle Trip Generation per Student

Average Rate	Range of Rates	Standard Deviation
0.78	0.39 - 1.78	0.25

Data Plot and Equation



Day Care Center (565)

Vehicle Trip Ends vs: Students

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 4 and 6 p.m.

Setting/Location: General Urban/Suburban

Number of Studies: 75

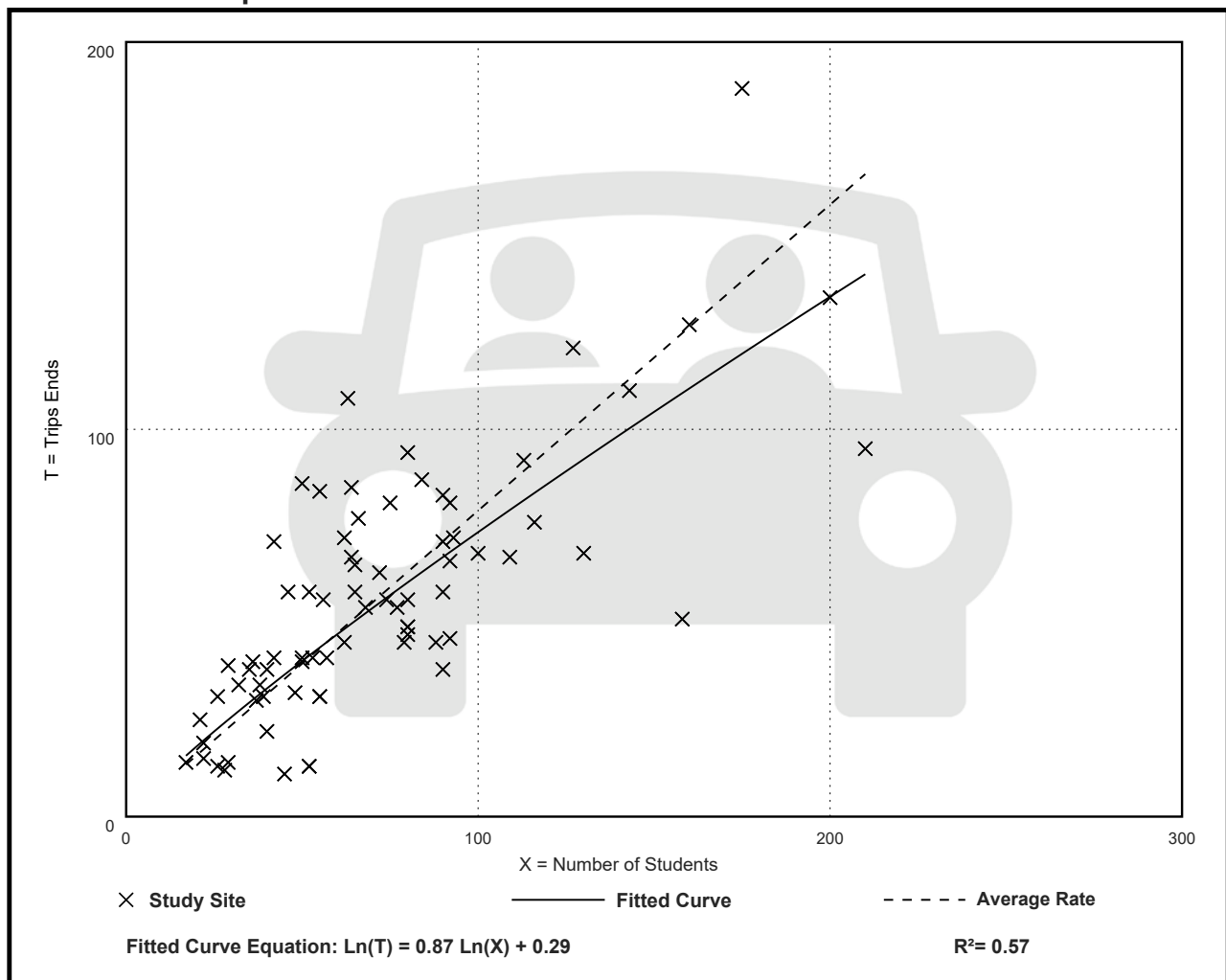
Avg. Num. of Students: 72

Directional Distribution: 47% entering, 53% exiting

Vehicle Trip Generation per Student

Average Rate	Range of Rates	Standard Deviation
0.79	0.24 - 1.72	0.30

Data Plot and Equation



Day Care Center (565)

Vehicle Trip Ends vs: Students

On a: Saturday, Peak Hour of Generator

Setting/Location: General Urban/Suburban

Number of Studies: 5

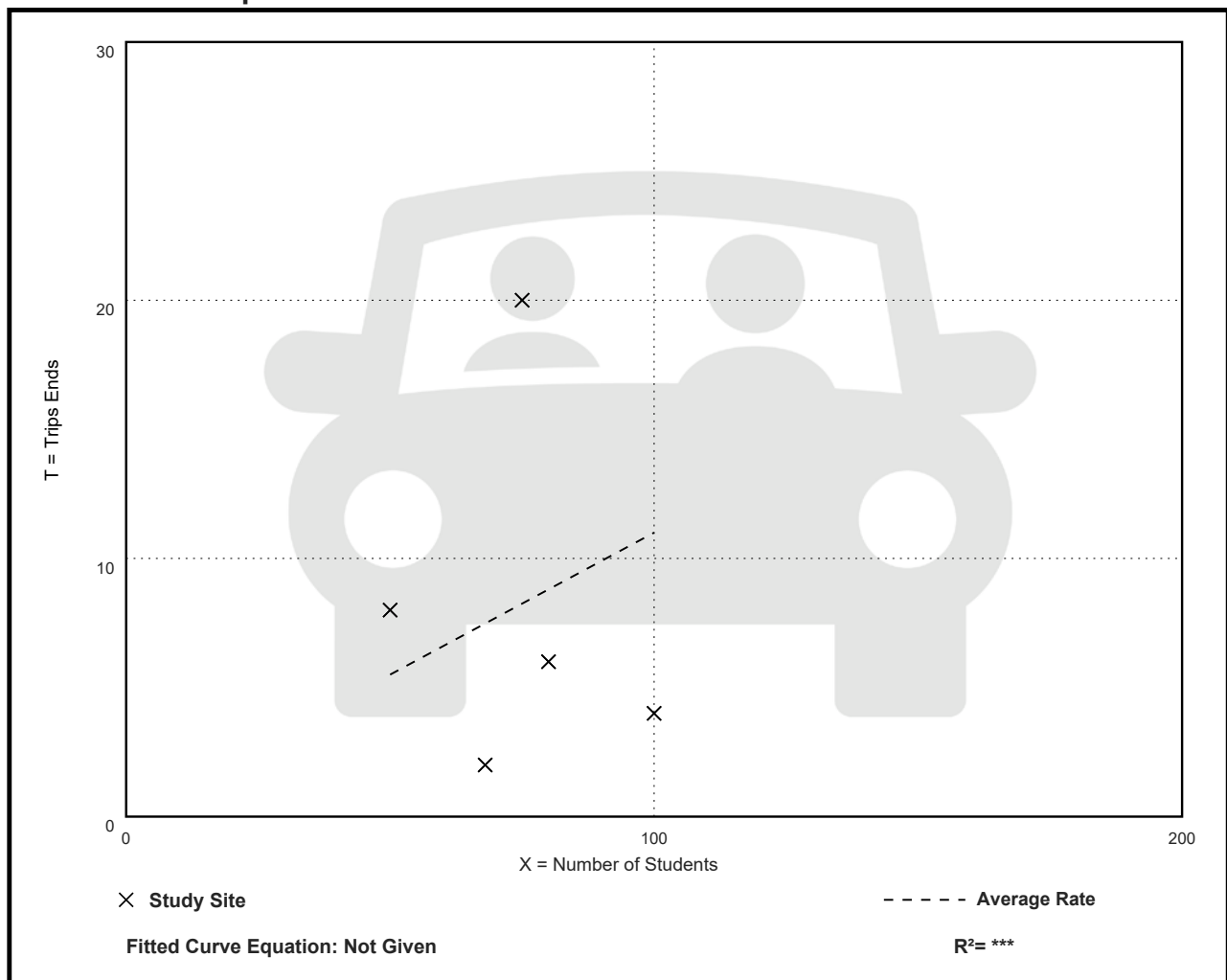
Avg. Num. of Students: 75

Directional Distribution: 63% entering, 37% exiting

Vehicle Trip Generation per Student

Average Rate	Range of Rates	Standard Deviation
0.11	0.03 - 0.27	0.10

Data Plot and Equation



General Office Building (710)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban

Number of Studies: 221

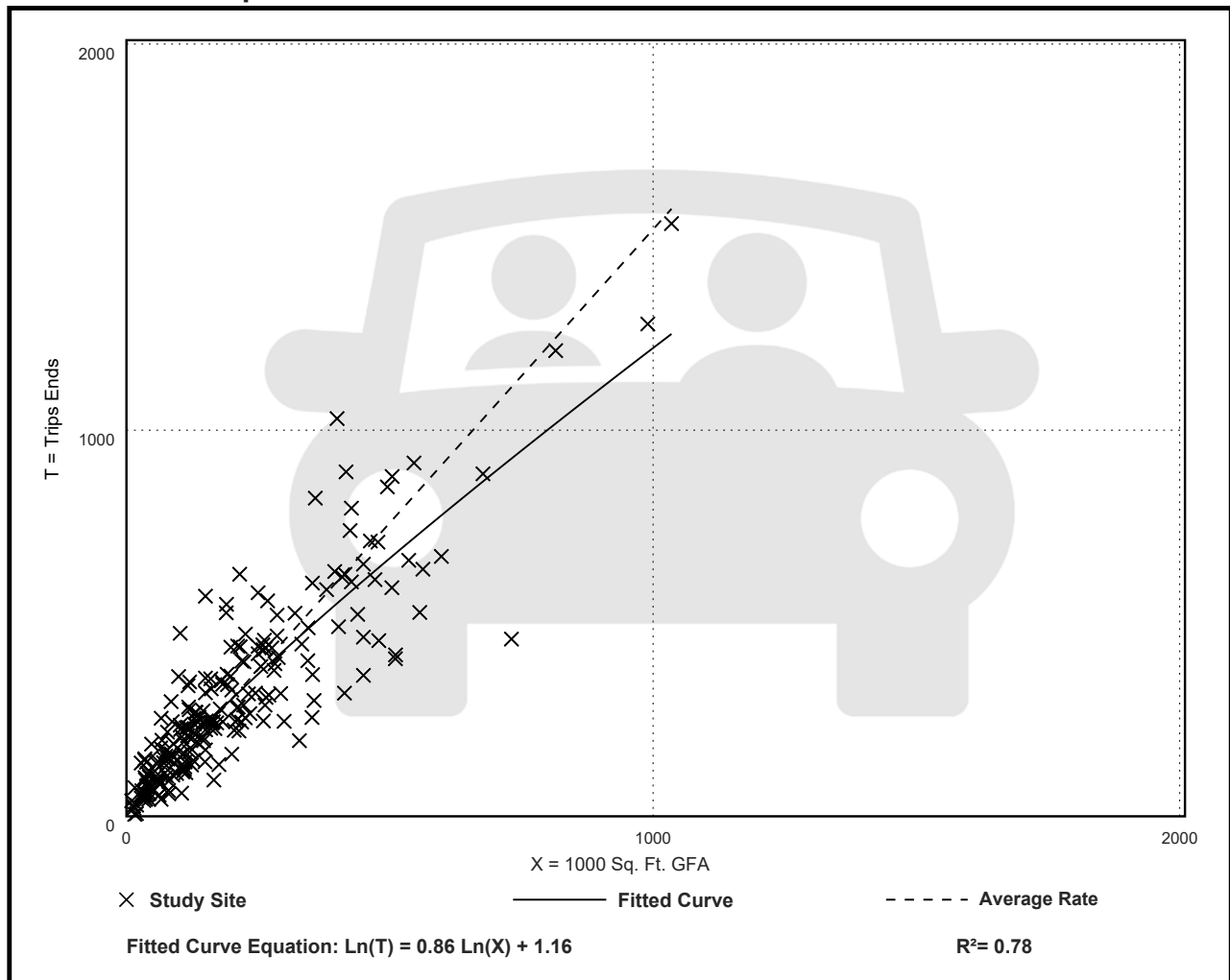
Avg. 1000 Sq. Ft. GFA: 201

Directional Distribution: 88% entering, 12% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
1.52	0.32 - 4.93	0.58

Data Plot and Equation



General Office Building (710)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 4 and 6 p.m.

Setting/Location: General Urban/Suburban

Number of Studies: 232

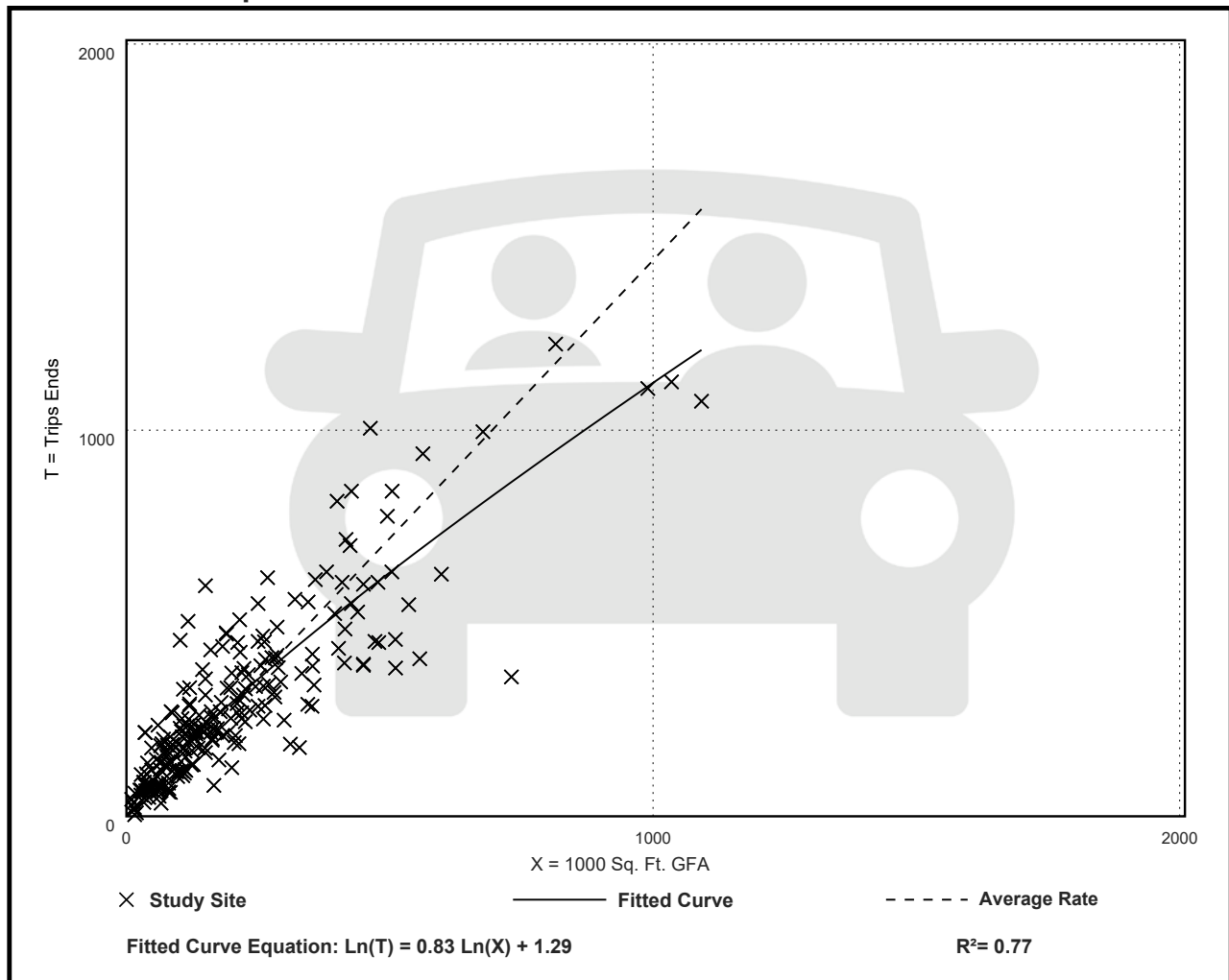
Avg. 1000 Sq. Ft. GFA: 199

Directional Distribution: 17% entering, 83% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
1.44	0.26 - 6.20	0.60

Data Plot and Equation



General Office Building (710)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Saturday, Peak Hour of Generator

Setting/Location: General Urban/Suburban

Number of Studies: 3

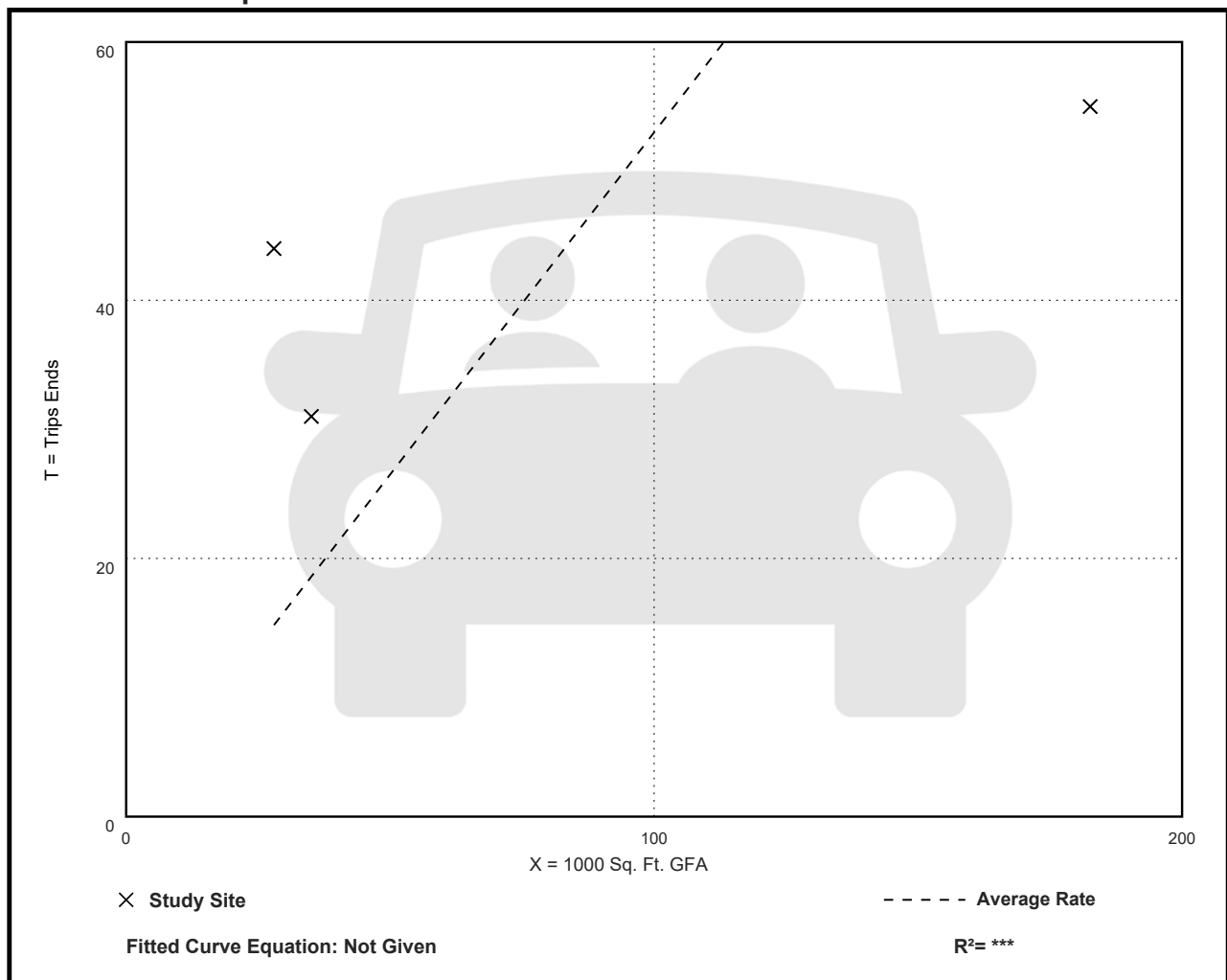
Avg. 1000 Sq. Ft. GFA: 82

Directional Distribution: 54% entering, 46% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
0.53	0.30 - 1.57	0.52

Data Plot and Equation



General Office Building (710)

Vehicle Trip Ends vs: Employees

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban

Number of Studies: 153

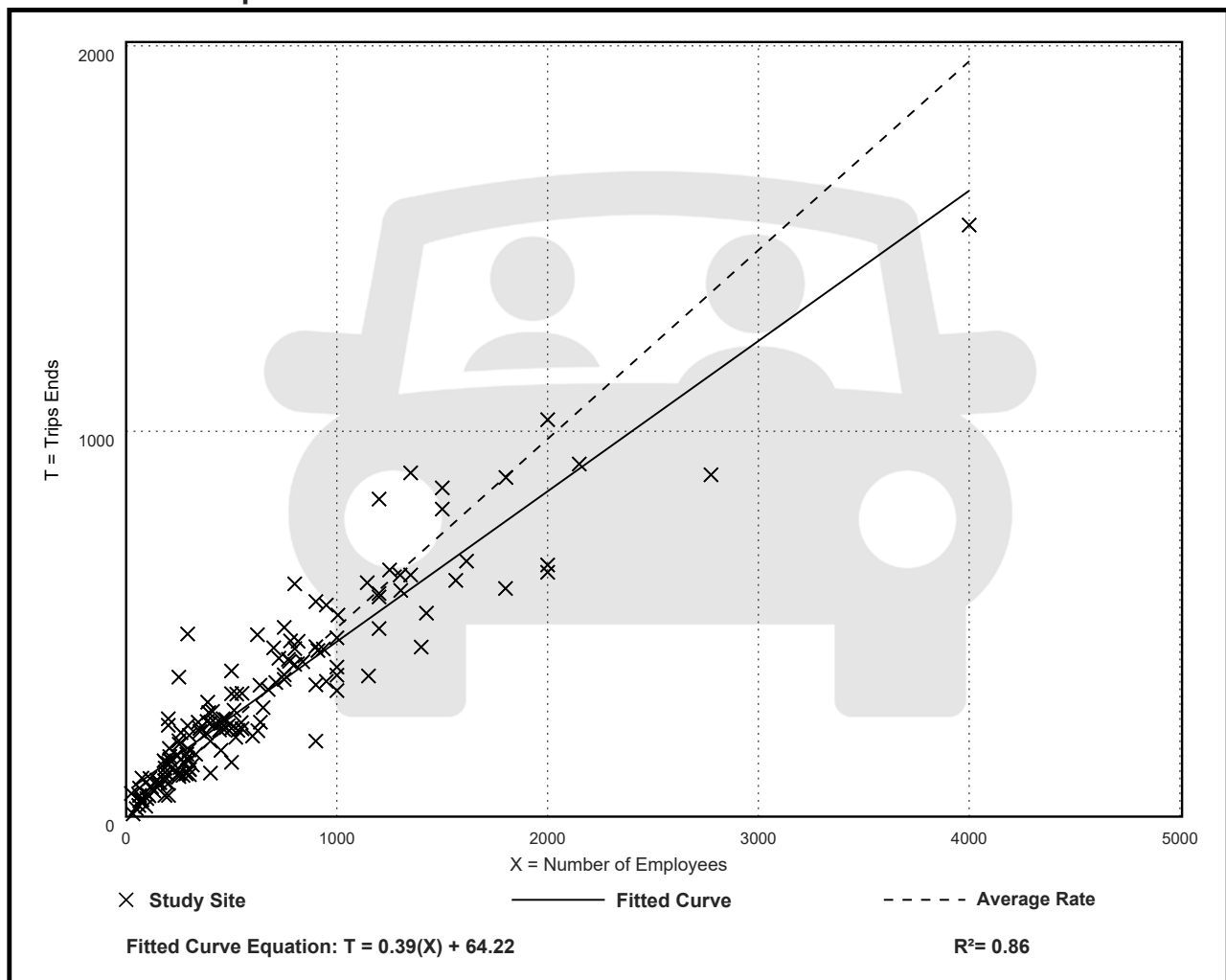
Avg. Num. of Employees: 618

Directional Distribution: 88% entering, 12% exiting

Vehicle Trip Generation per Employee

Average Rate	Range of Rates	Standard Deviation
0.49	0.21 - 2.40	0.16

Data Plot and Equation



General Office Building (710)

Vehicle Trip Ends vs: Employees

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 4 and 6 p.m.

Setting/Location: General Urban/Suburban

Number of Studies: 163

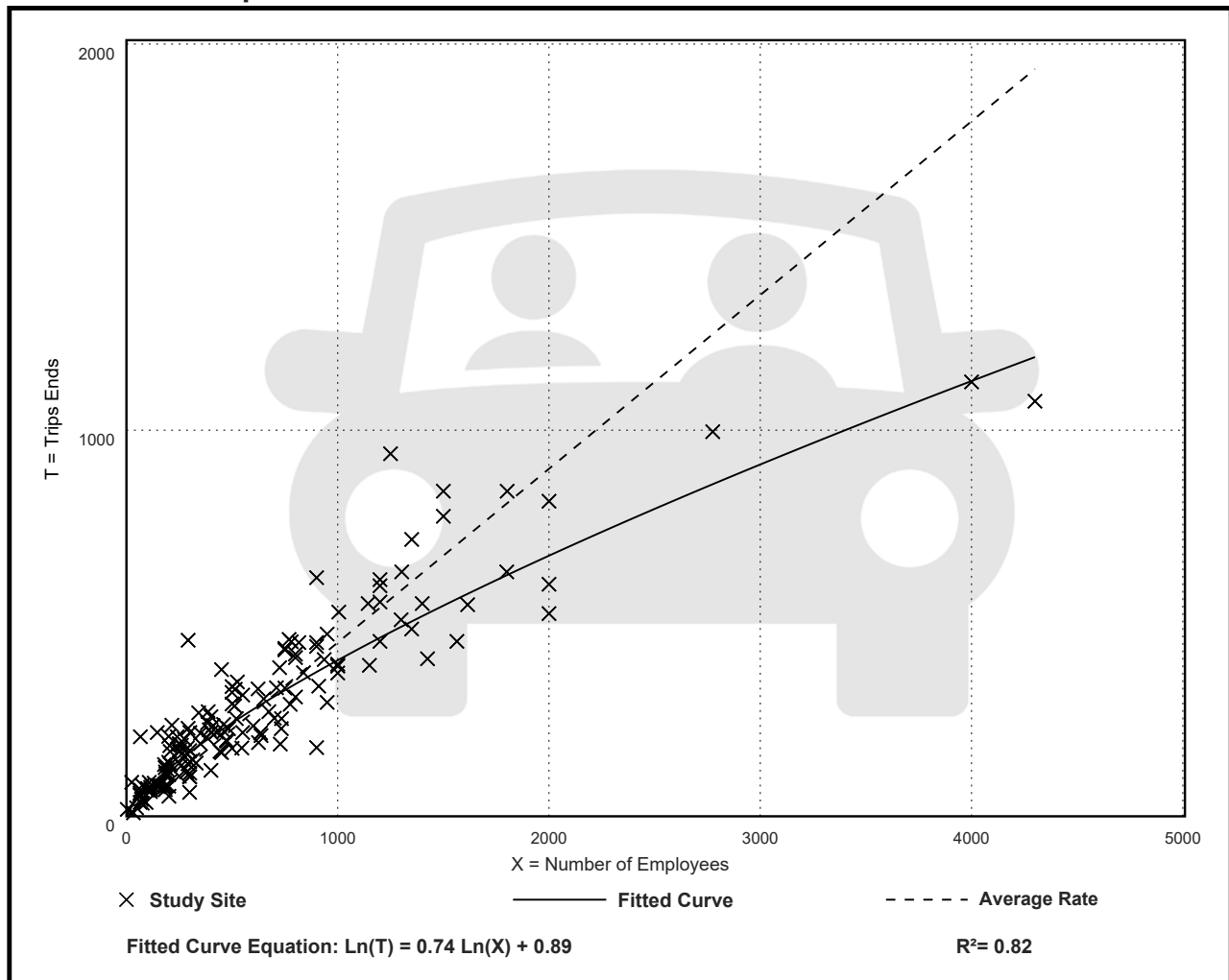
Avg. Num. of Employees: 614

Directional Distribution: 17% entering, 83% exiting

Vehicle Trip Generation per Employee

Average Rate	Range of Rates	Standard Deviation
0.45	0.20 - 4.50	0.18

Data Plot and Equation



General Office Building (710)

Vehicle Trip Ends vs: Employees

On a: Saturday, Peak Hour of Generator

Setting/Location: General Urban/Suburban

Number of Studies: 2

Avg. Num. of Employees: 550

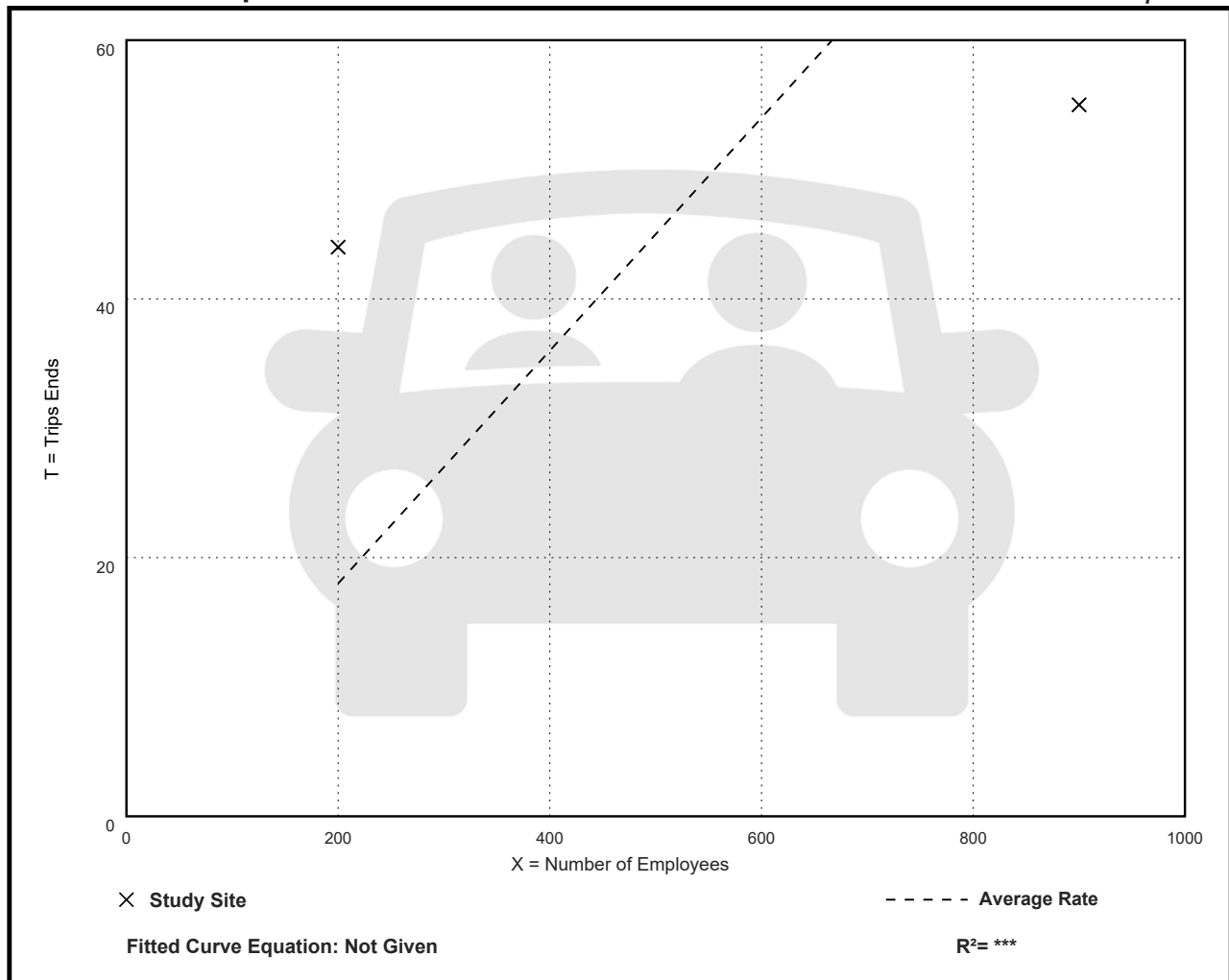
Directional Distribution: 54% entering, 46% exiting

Vehicle Trip Generation per Employee

Average Rate	Range of Rates	Standard Deviation
0.09	0.06 - 0.22	***

Data Plot and Equation

Caution – Small Sample Size



General Office Building (710)

Walk+Bike+Transit Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban

Number of Studies: 21

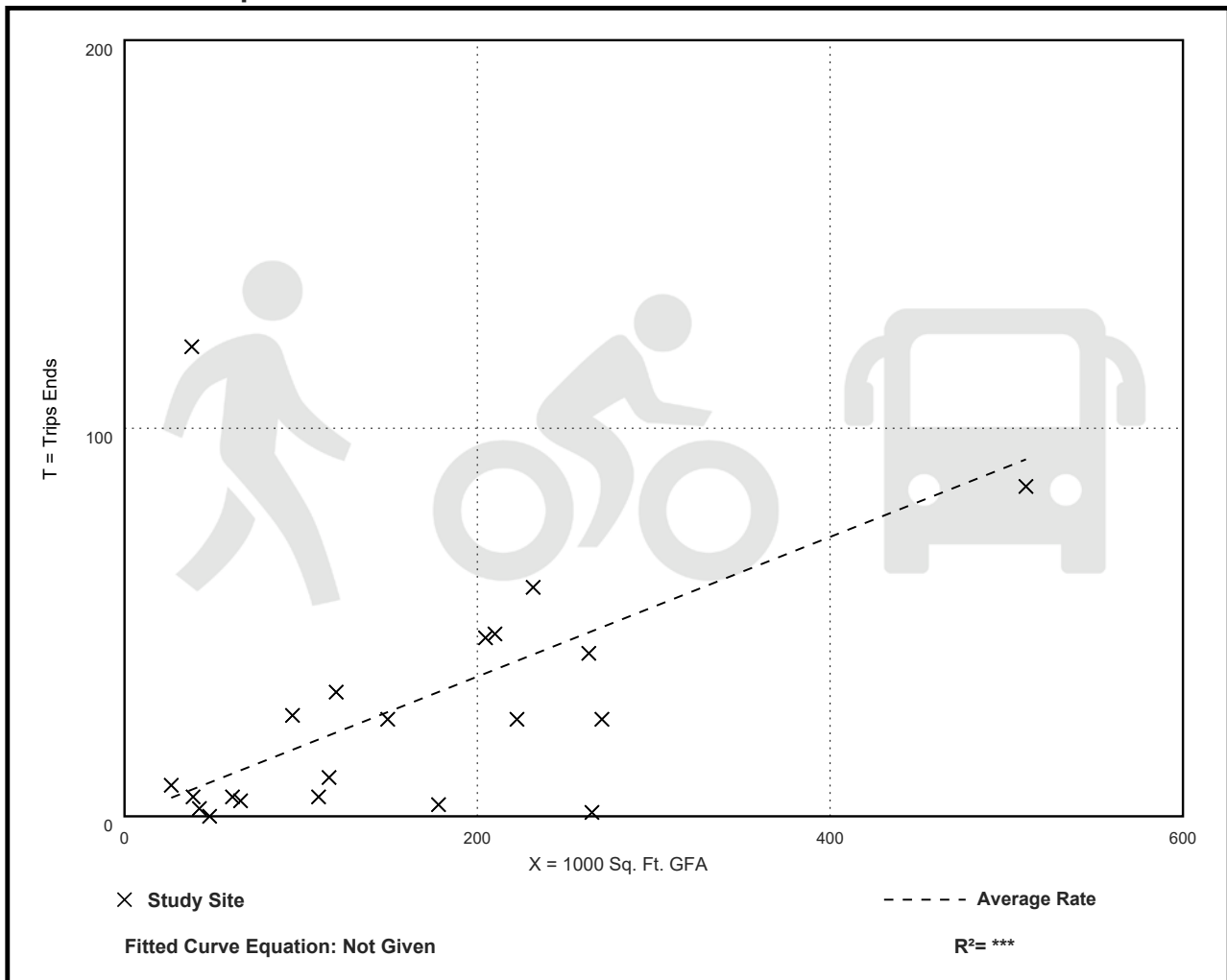
Avg. 1000 Sq. Ft. GFA: 156

Directional Distribution: Not Available

Walk+Bike+Transit Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
0.18	0.00 - 3.17	0.34

Data Plot and Equation



General Office Building (710)

Walk+Bike+Transit Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 4 and 6 p.m.

Setting/Location: General Urban/Suburban

Number of Studies: 19

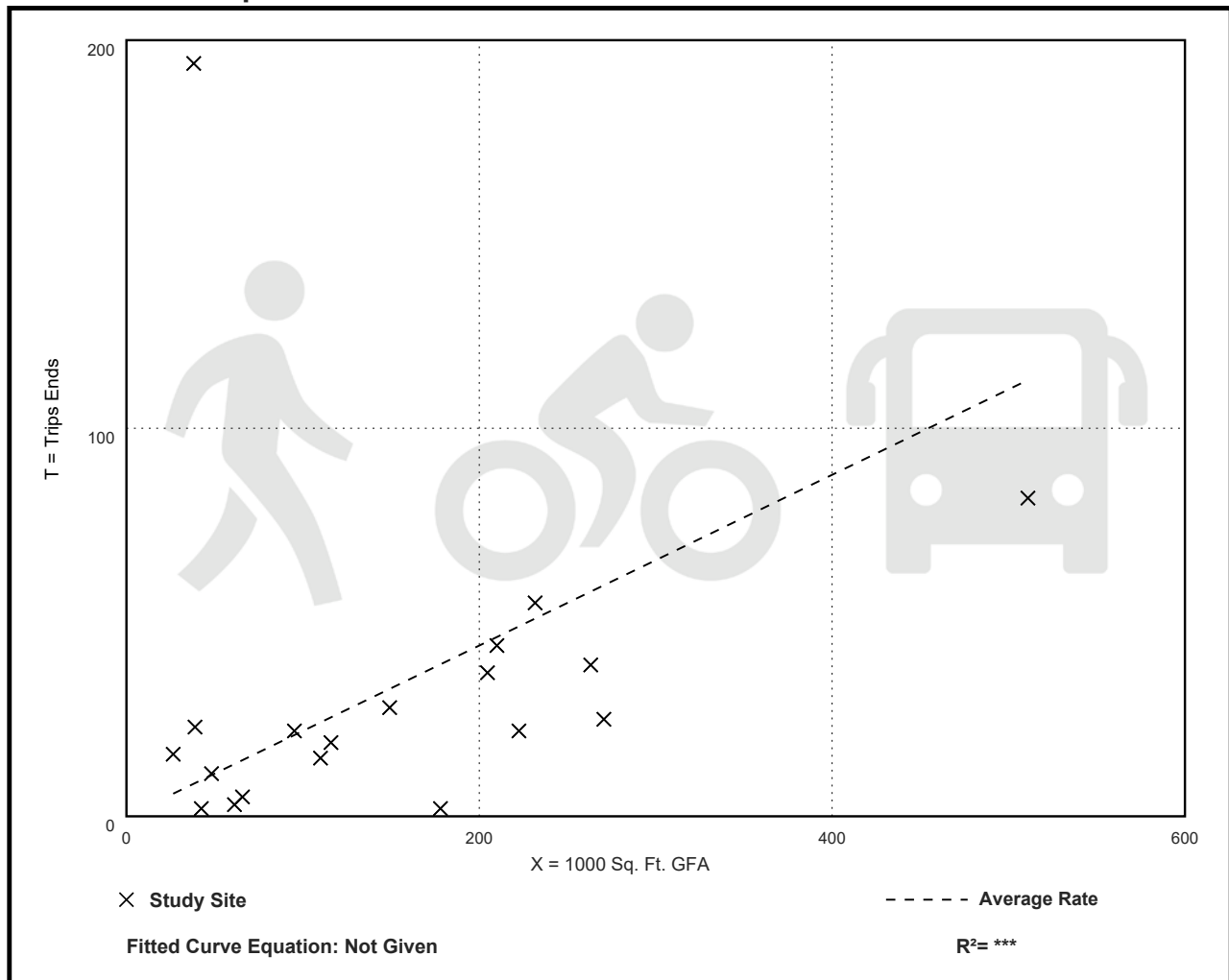
Avg. 1000 Sq. Ft. GFA: 152

Directional Distribution: Not Available

Walk+Bike+Transit Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
0.22	0.01 - 5.09	0.59

Data Plot and Equation



General Office Building (710)

Walk+Bike+Transit Trip Ends vs: Employees

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban

Number of Studies: 7

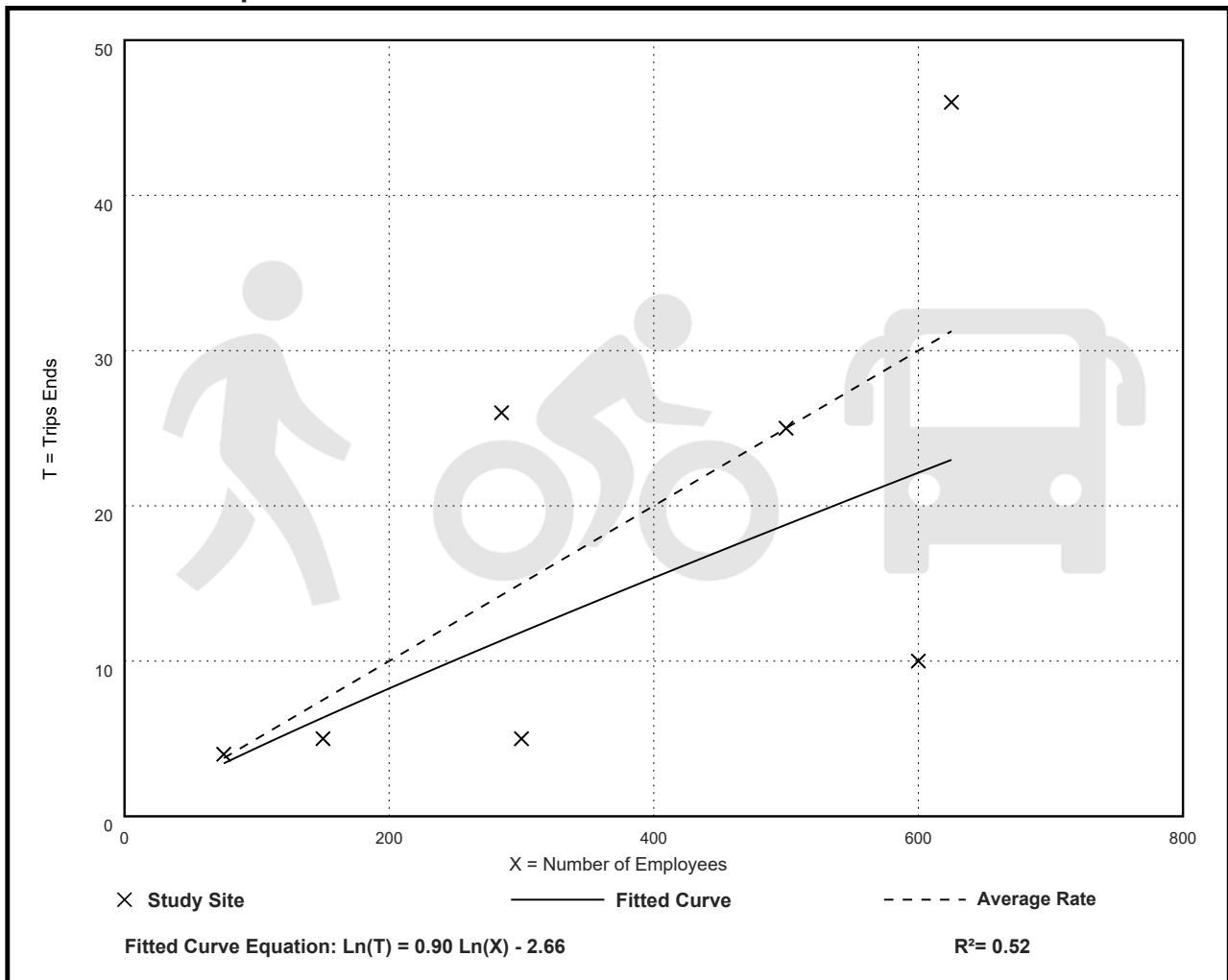
Avg. Num. of Employees: 362

Directional Distribution: Not Available

Walk+Bike+Transit Trip Generation per Employee

Average Rate	Range of Rates	Standard Deviation
0.05	0.02 - 0.09	0.03

Data Plot and Equation



General Office Building (710)

Walk+Bike+Transit Trip Ends vs: Employees

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 4 and 6 p.m.

Setting/Location: General Urban/Suburban

Number of Studies: 7

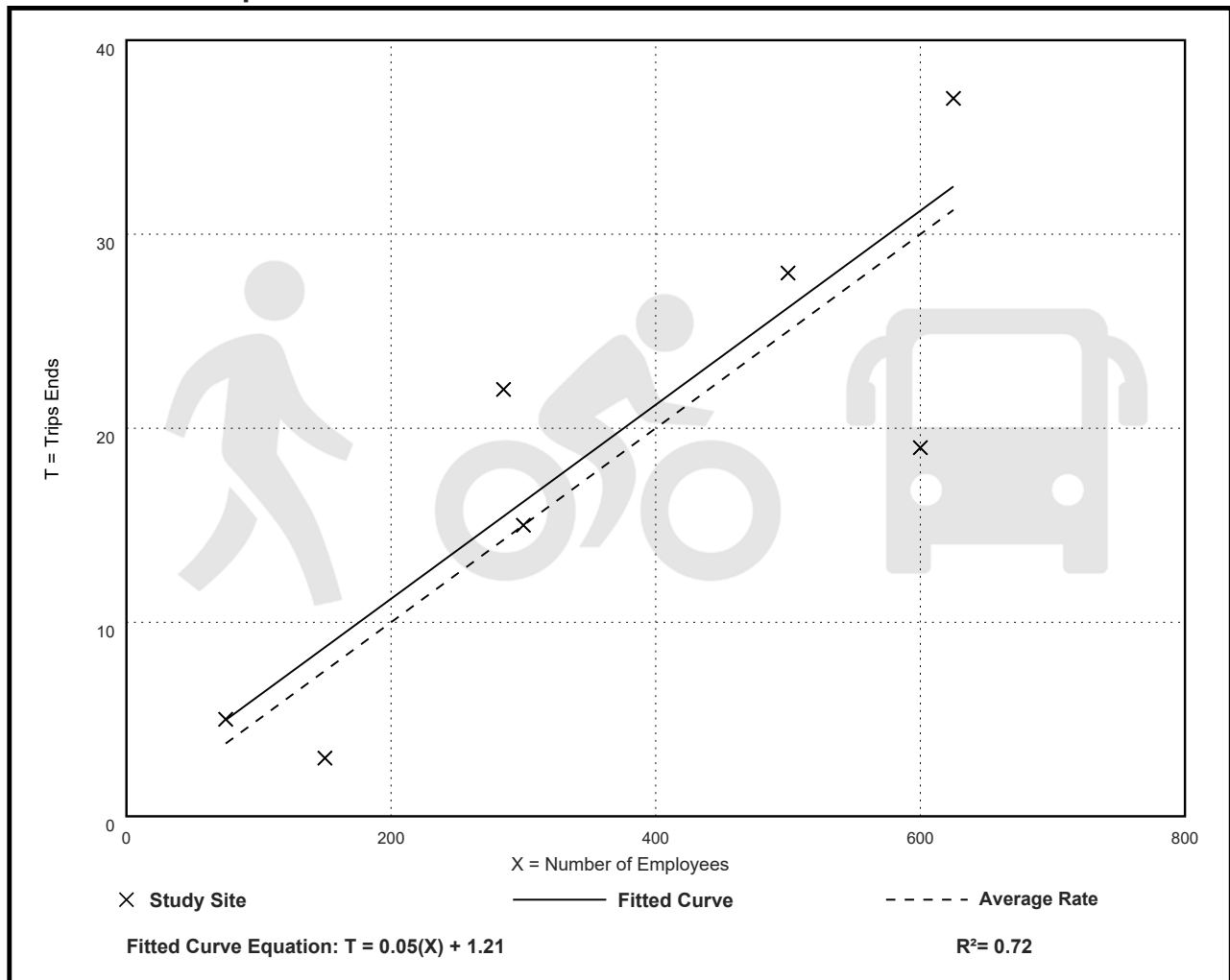
Avg. Num. of Employees: 362

Directional Distribution: Not Available

Walk+Bike+Transit Trip Generation per Employee

Average Rate	Range of Rates	Standard Deviation
0.05	0.02 - 0.08	0.02

Data Plot and Equation



Strip Retail Plaza (<40k) (822)

Vehicle Trip Ends vs: 1000 Sq. Ft. GLA

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban

Number of Studies: 5

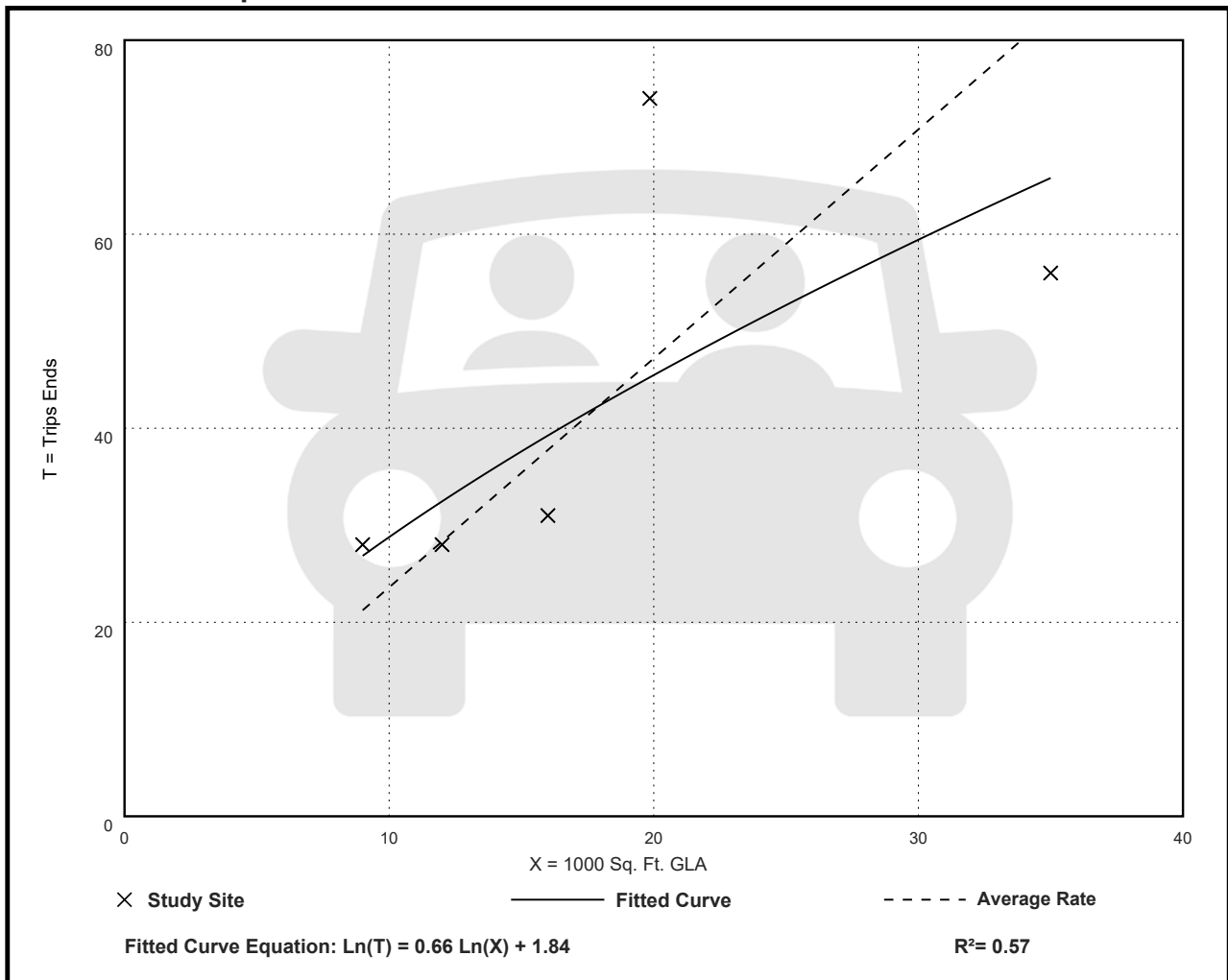
Avg. 1000 Sq. Ft. GLA: 18

Directional Distribution: 60% entering, 40% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GLA

Average Rate	Range of Rates	Standard Deviation
2.36	1.60 - 3.73	0.94

Data Plot and Equation



Strip Retail Plaza (<40k) (822)

Vehicle Trip Ends vs: 1000 Sq. Ft. GLA

On a: Saturday, Peak Hour of Generator

Setting/Location: General Urban/Suburban

Number of Studies: 12

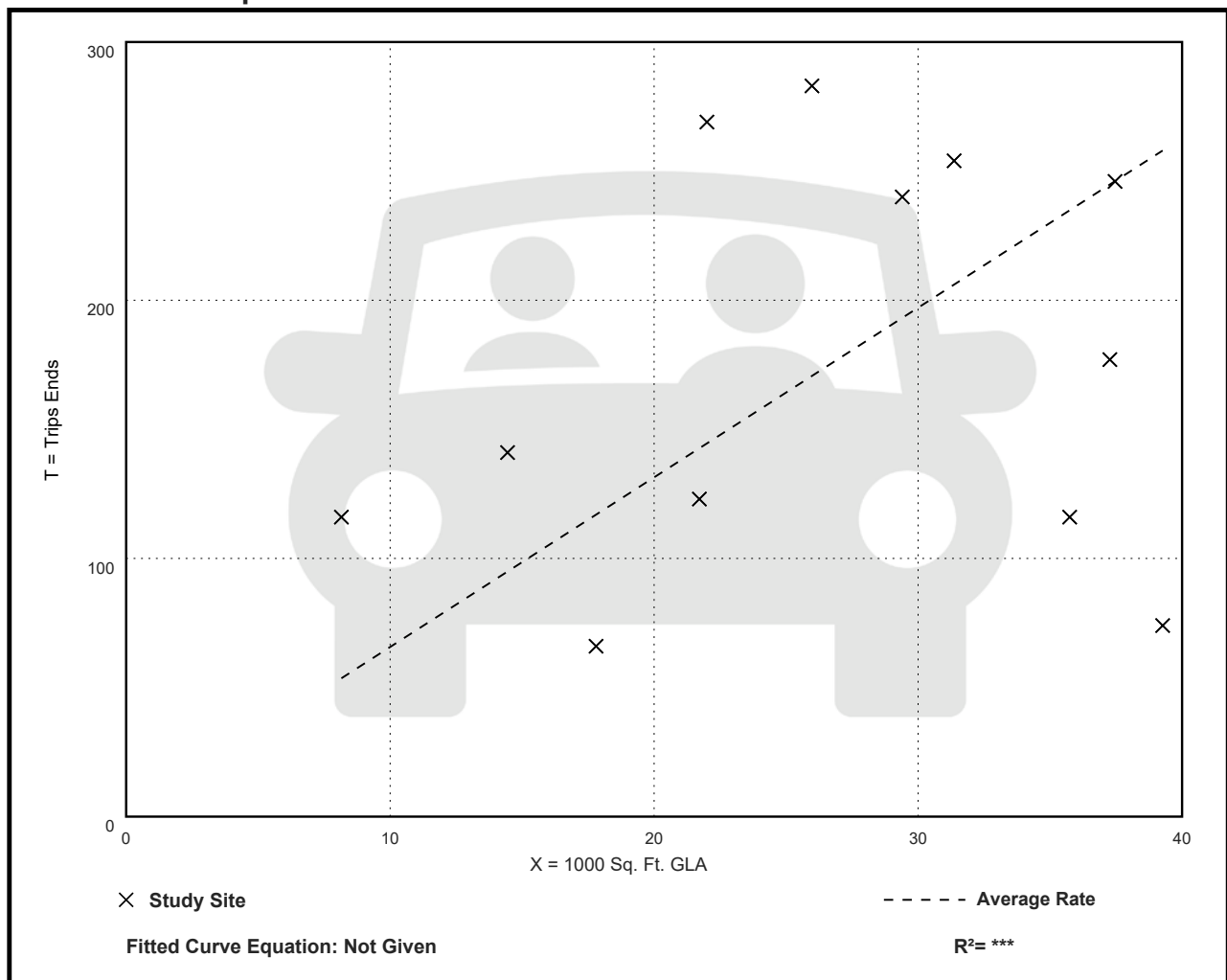
Avg. 1000 Sq. Ft. GLA: 27

Directional Distribution: 51% entering, 49% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GLA

Average Rate	Range of Rates	Standard Deviation
6.57	1.88 - 14.23	3.45

Data Plot and Equation



Hair Salon (918)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban

Number of Studies: 1

Avg. 1000 Sq. Ft. GFA: 4

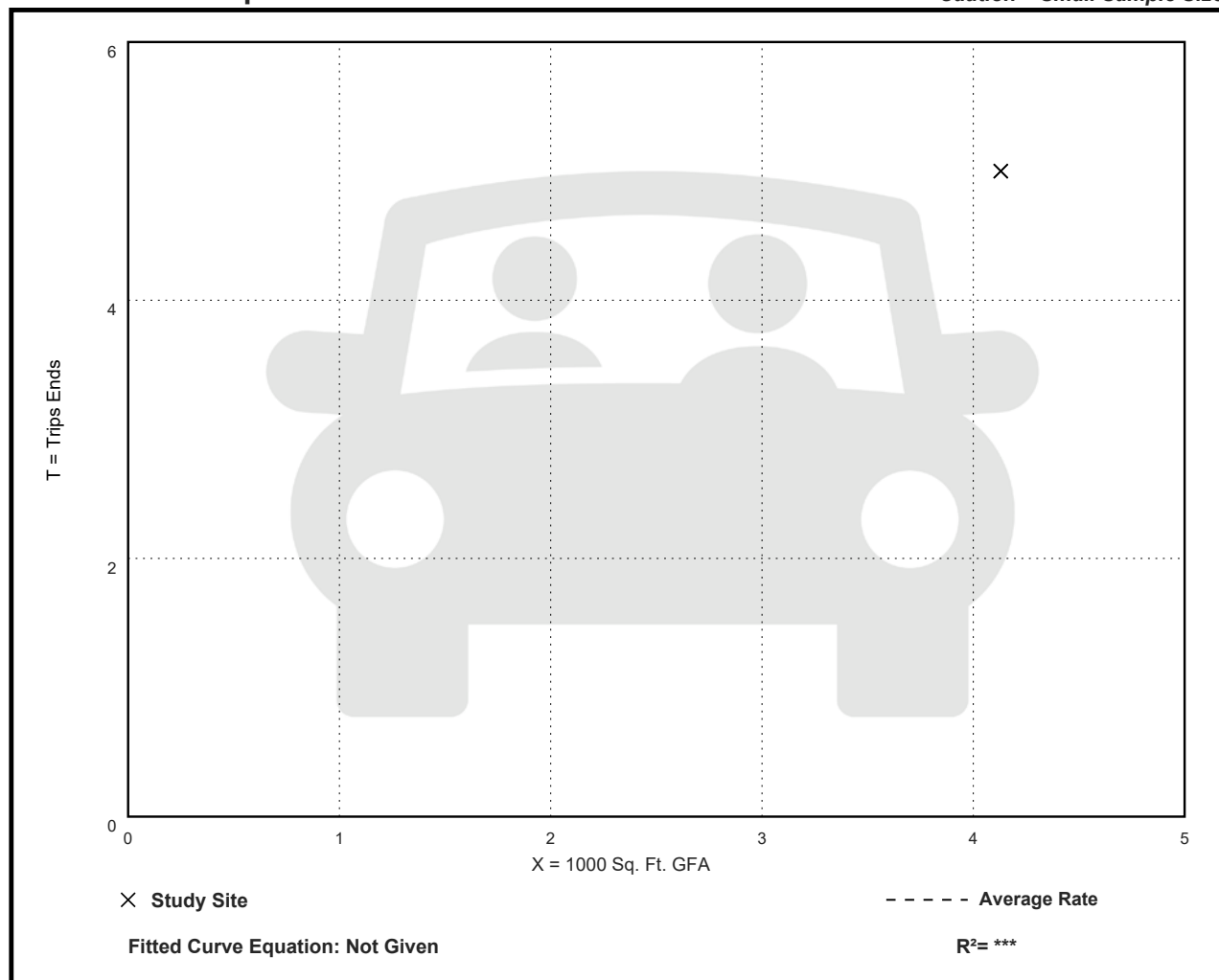
Directional Distribution: Not Available

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
1.21	1.21 - 1.21	***

Data Plot and Equation

Caution – Small Sample Size



Hair Salon (918)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 4 and 6 p.m.

Setting/Location: General Urban/Suburban

Number of Studies: 1

Avg. 1000 Sq. Ft. GFA: 4

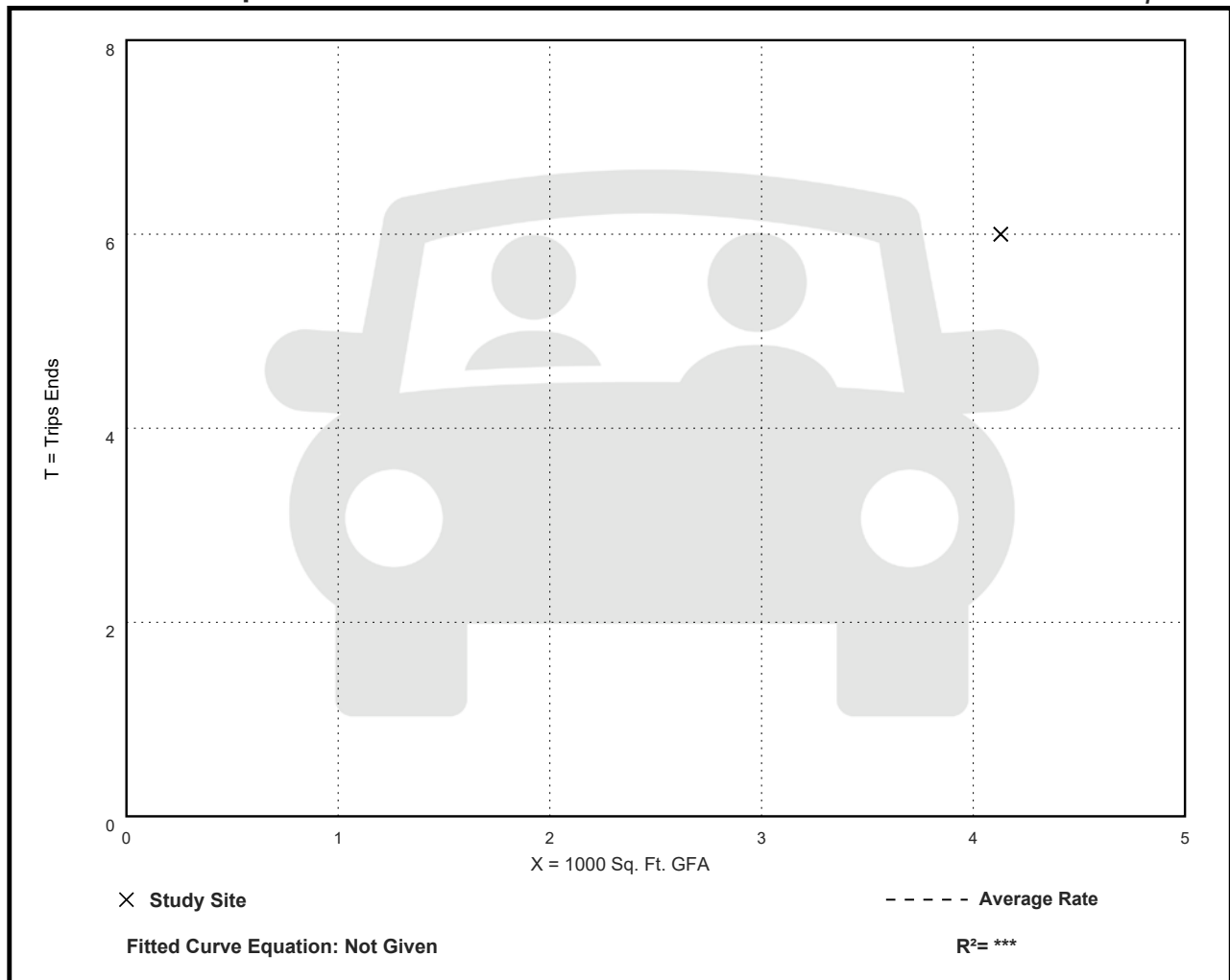
Directional Distribution: 17% entering, 83% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
1.45	1.45 - 1.45	***

Data Plot and Equation

Caution – Small Sample Size



Hair Salon (918)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Saturday, Peak Hour of Generator

Setting/Location: General Urban/Suburban

Number of Studies: 1

Avg. 1000 Sq. Ft. GFA: 4

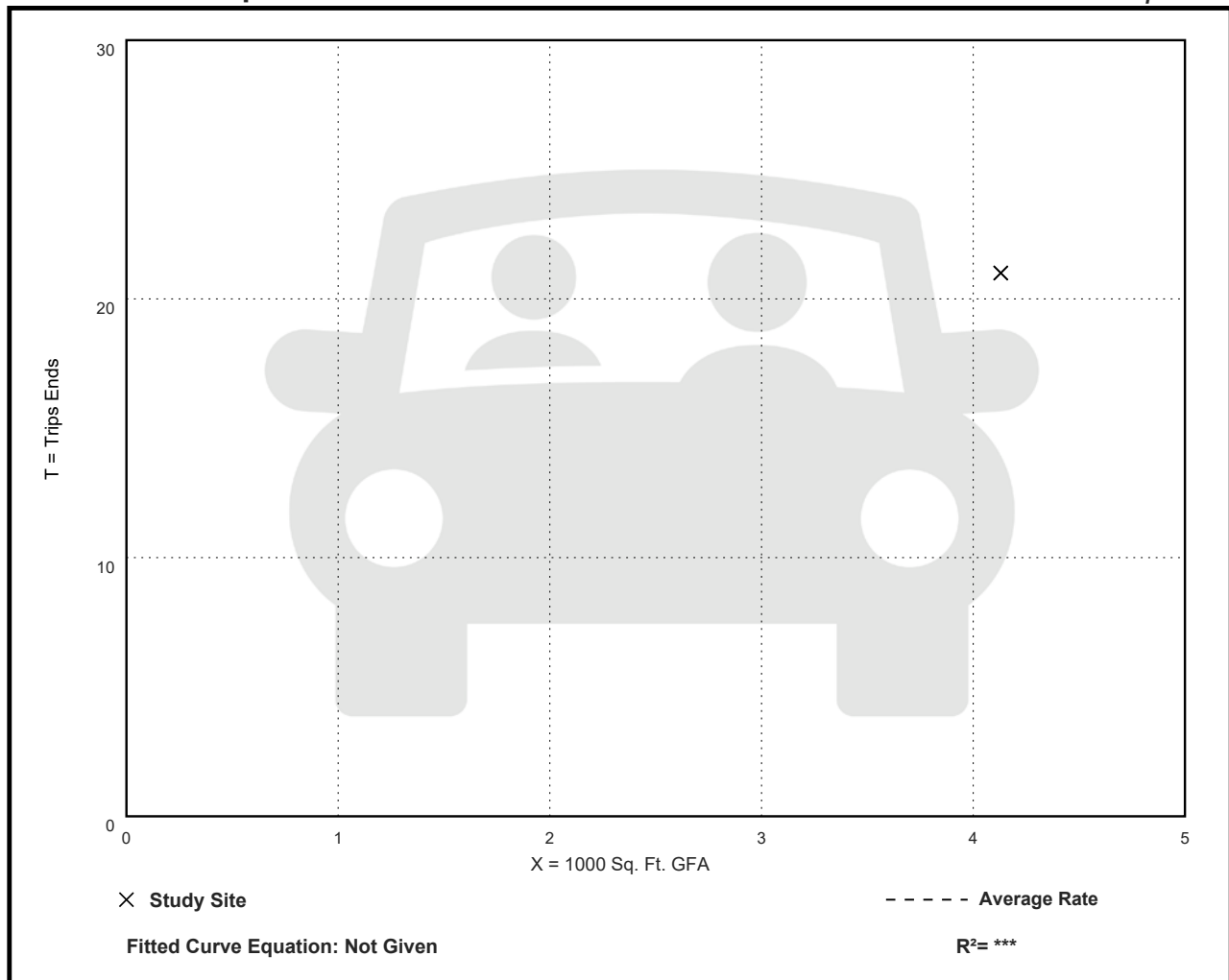
Directional Distribution: 36% entering, 64% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
5.08	5.08 - 5.08	***

Data Plot and Equation

Caution – Small Sample Size



Fast Casual Restaurant (930)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban

Number of Studies: 1

Avg. 1000 Sq. Ft. GFA: 1

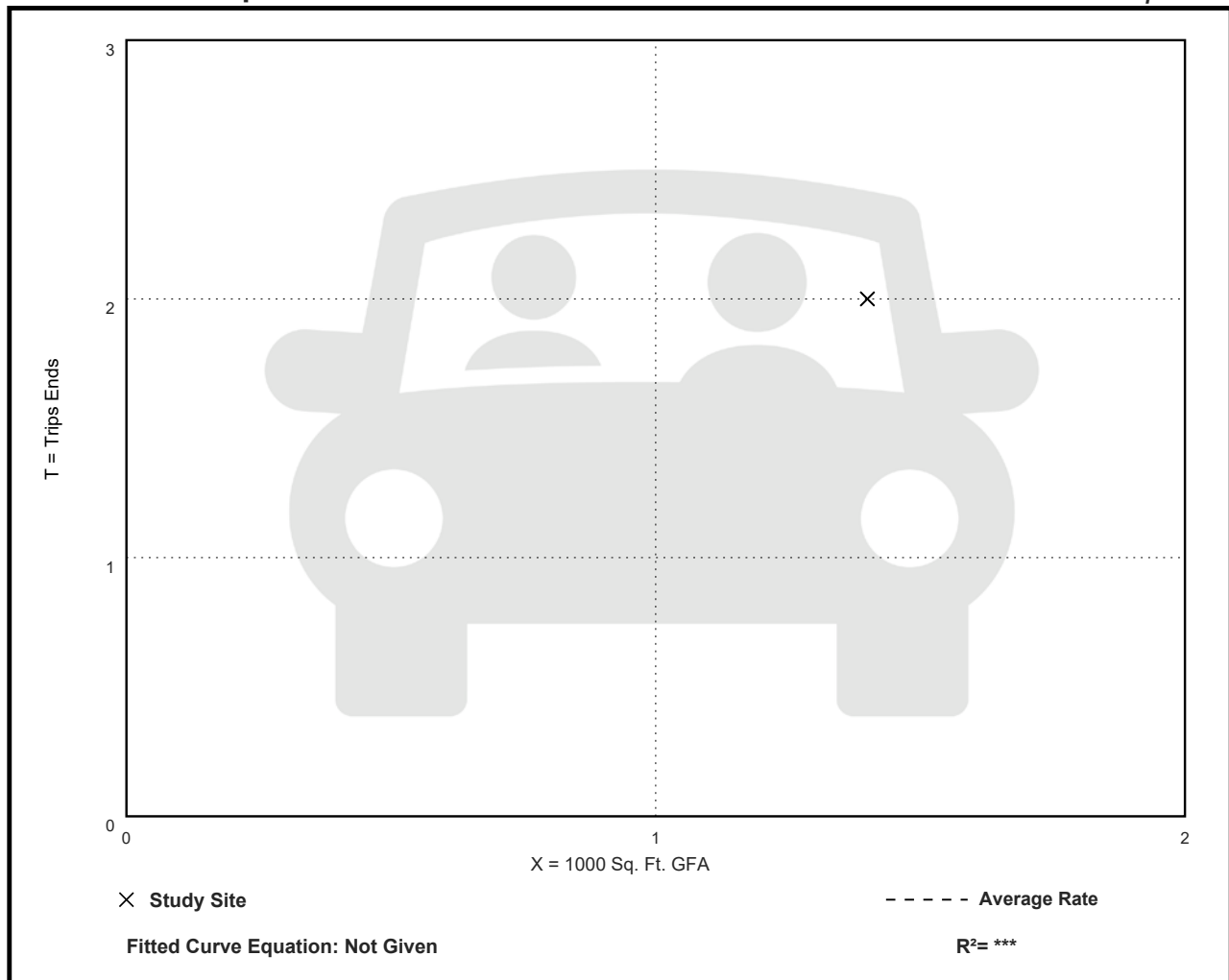
Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
1.43	1.43 - 1.43	***

Data Plot and Equation

Caution – Small Sample Size



Fast Casual Restaurant (930)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 4 and 6 p.m.

Setting/Location: General Urban/Suburban

Number of Studies: 15

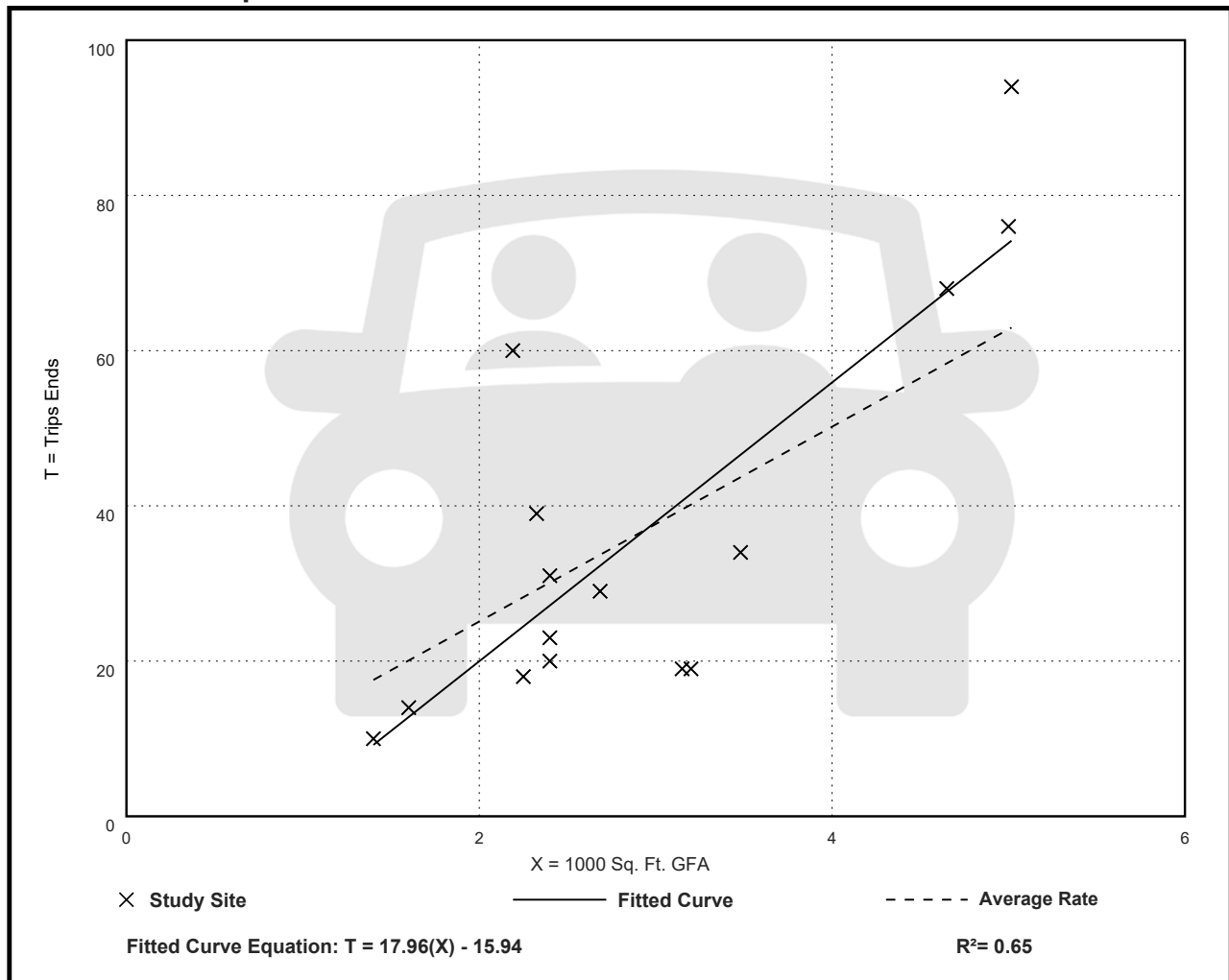
Avg. 1000 Sq. Ft. GFA: 3

Directional Distribution: 55% entering, 45% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
12.55	5.94 - 27.40	5.52

Data Plot and Equation



Fast Casual Restaurant (930)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Saturday, Peak Hour of Generator

Setting/Location: General Urban/Suburban

Number of Studies: 2

Avg. 1000 Sq. Ft. GFA: 5

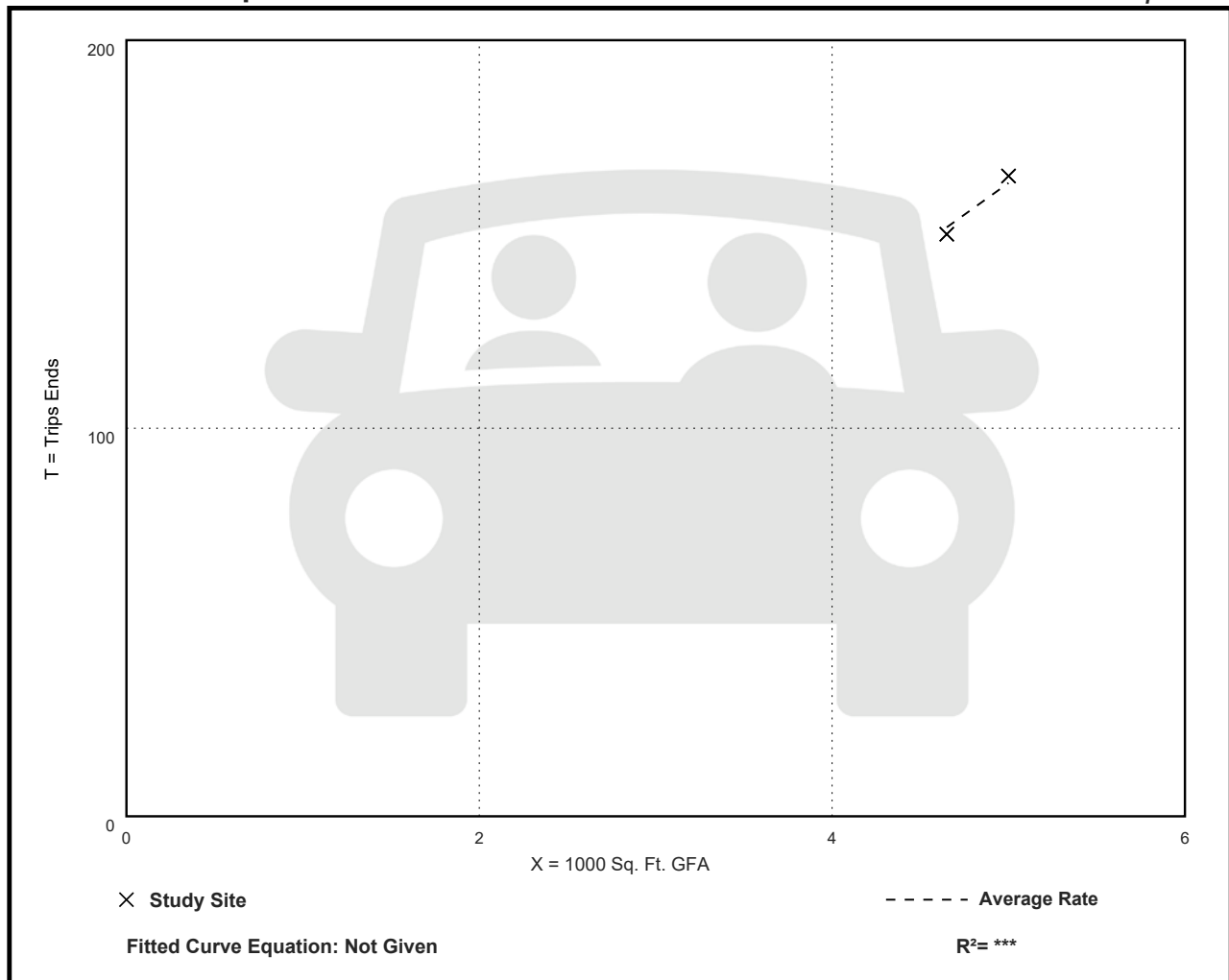
Directional Distribution: 55% entering, 45% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
32.64	32.26 - 33.00	***

Data Plot and Equation

Caution – Small Sample Size



Fine Dining Restaurant (931)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban

Number of Studies: 7

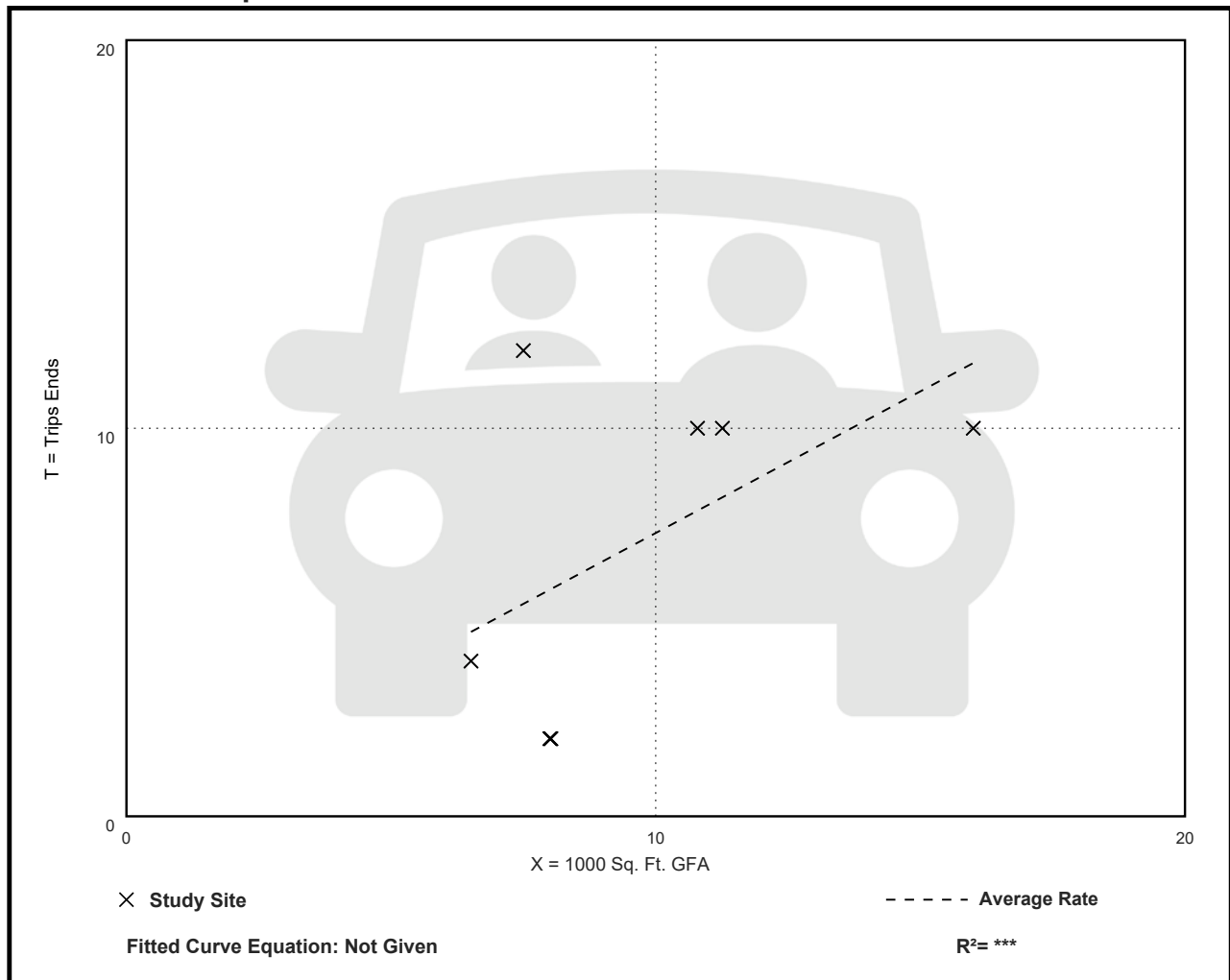
Avg. 1000 Sq. Ft. GFA: 10

Directional Distribution: Not Available

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
0.73	0.25 - 1.60	0.42

Data Plot and Equation



Fine Dining Restaurant (931)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 4 and 6 p.m.

Setting/Location: General Urban/Suburban

Number of Studies: 19

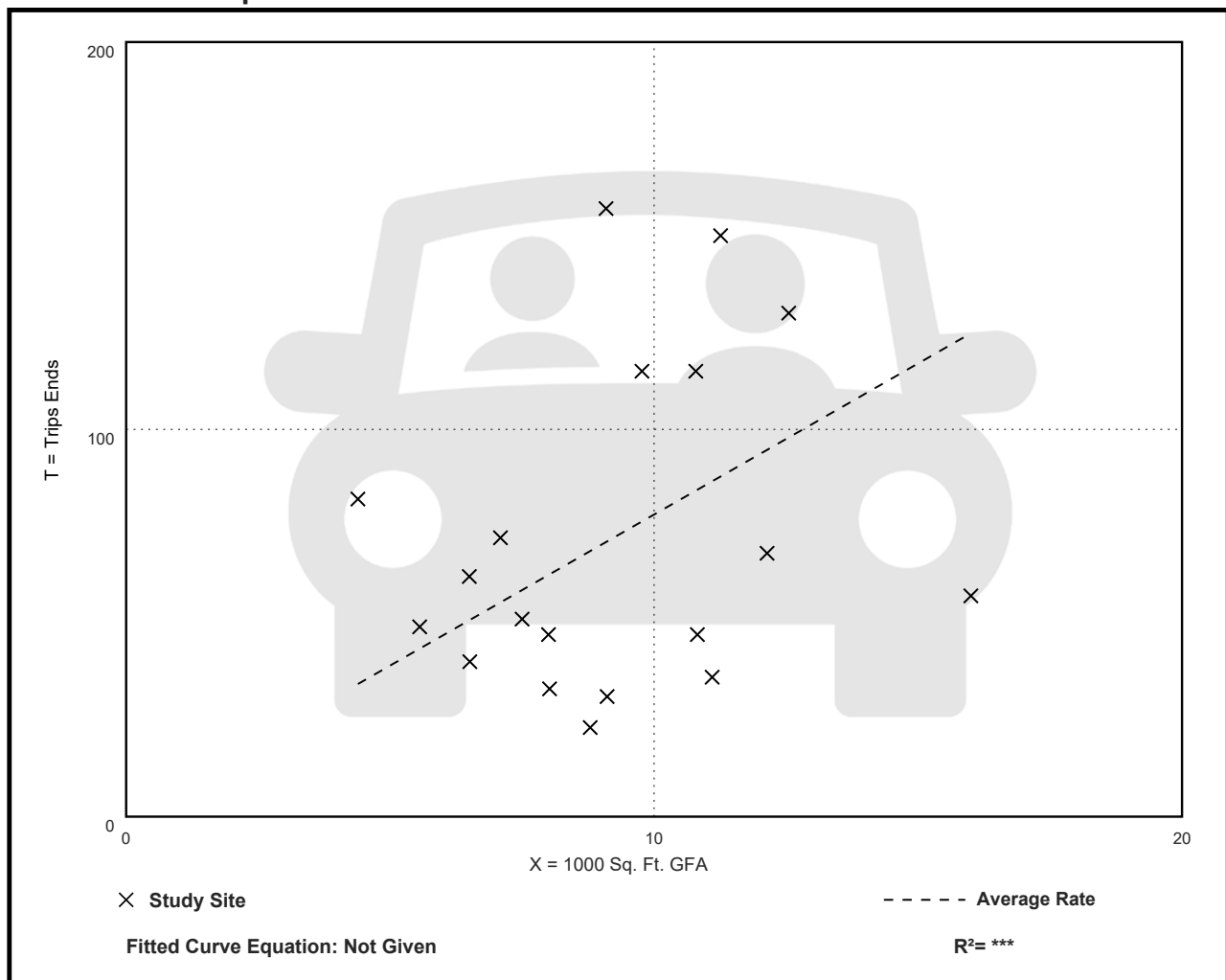
Avg. 1000 Sq. Ft. GFA: 9

Directional Distribution: 67% entering, 33% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
7.80	2.62 - 18.68	4.49

Data Plot and Equation



Fine Dining Restaurant (931)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Saturday, Peak Hour of Generator

Setting/Location: General Urban/Suburban

Number of Studies: 7

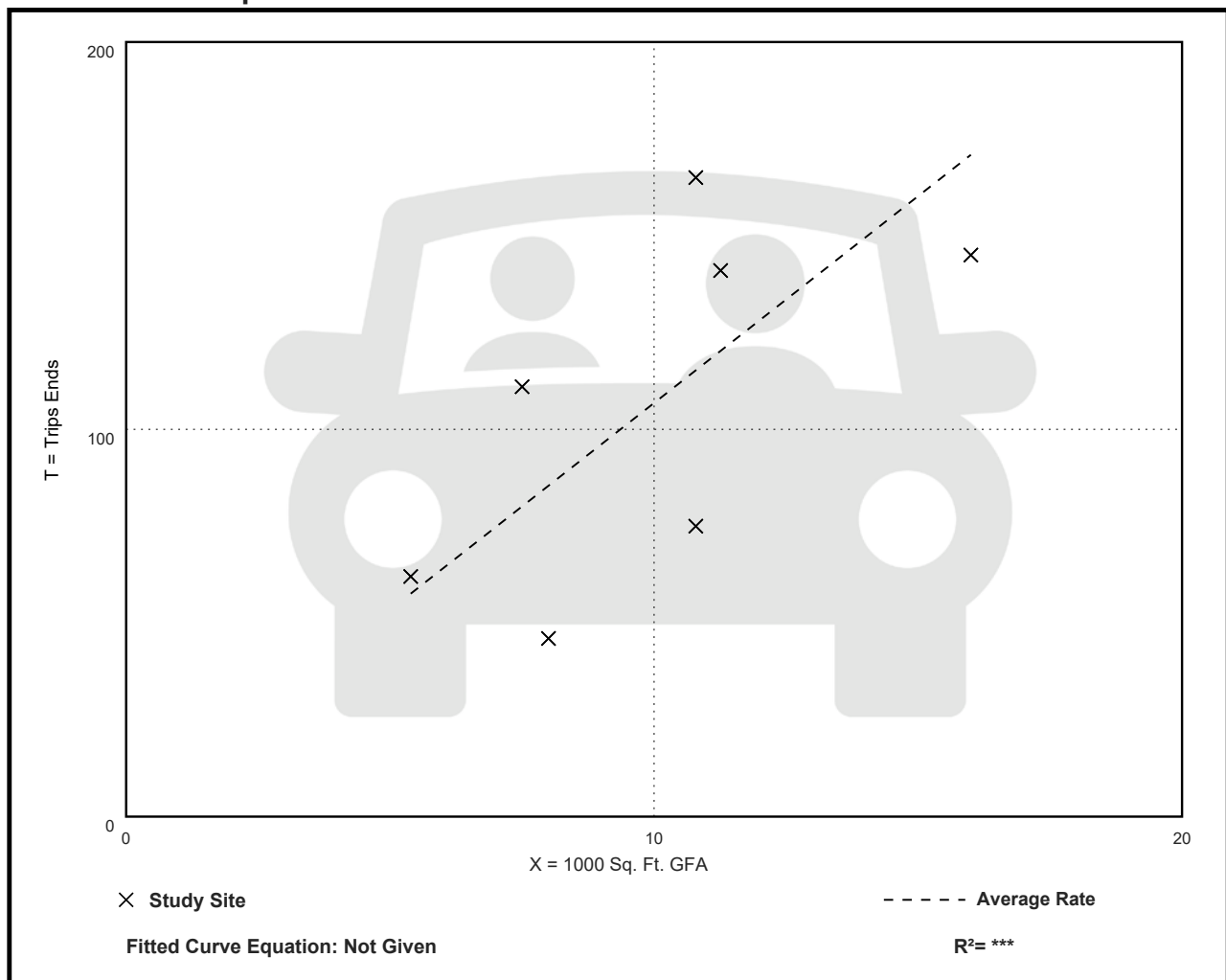
Avg. 1000 Sq. Ft. GFA: 10

Directional Distribution: 59% entering, 41% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
10.68	5.75 - 15.29	3.62

Data Plot and Equation



Fast-Food Restaurant without Drive-Through Window (933)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban

Number of Studies: 3

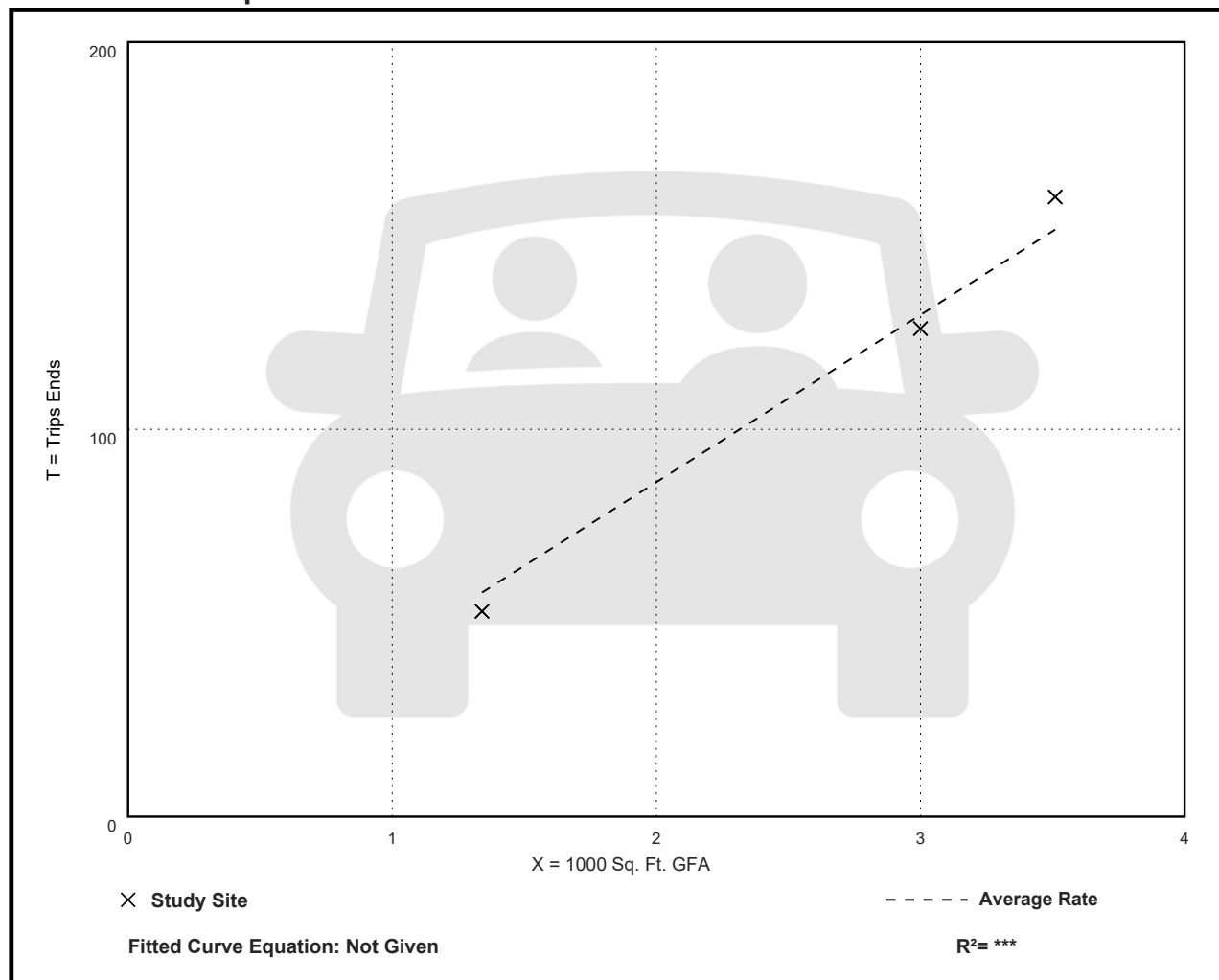
Avg. 1000 Sq. Ft. GFA: 3

Directional Distribution: 58% entering, 42% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
43.18	39.55 - 45.58	2.84

Data Plot and Equation



Fast-Food Restaurant without Drive-Through Window (933)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 4 and 6 p.m.

Setting/Location: General Urban/Suburban

Number of Studies: 8

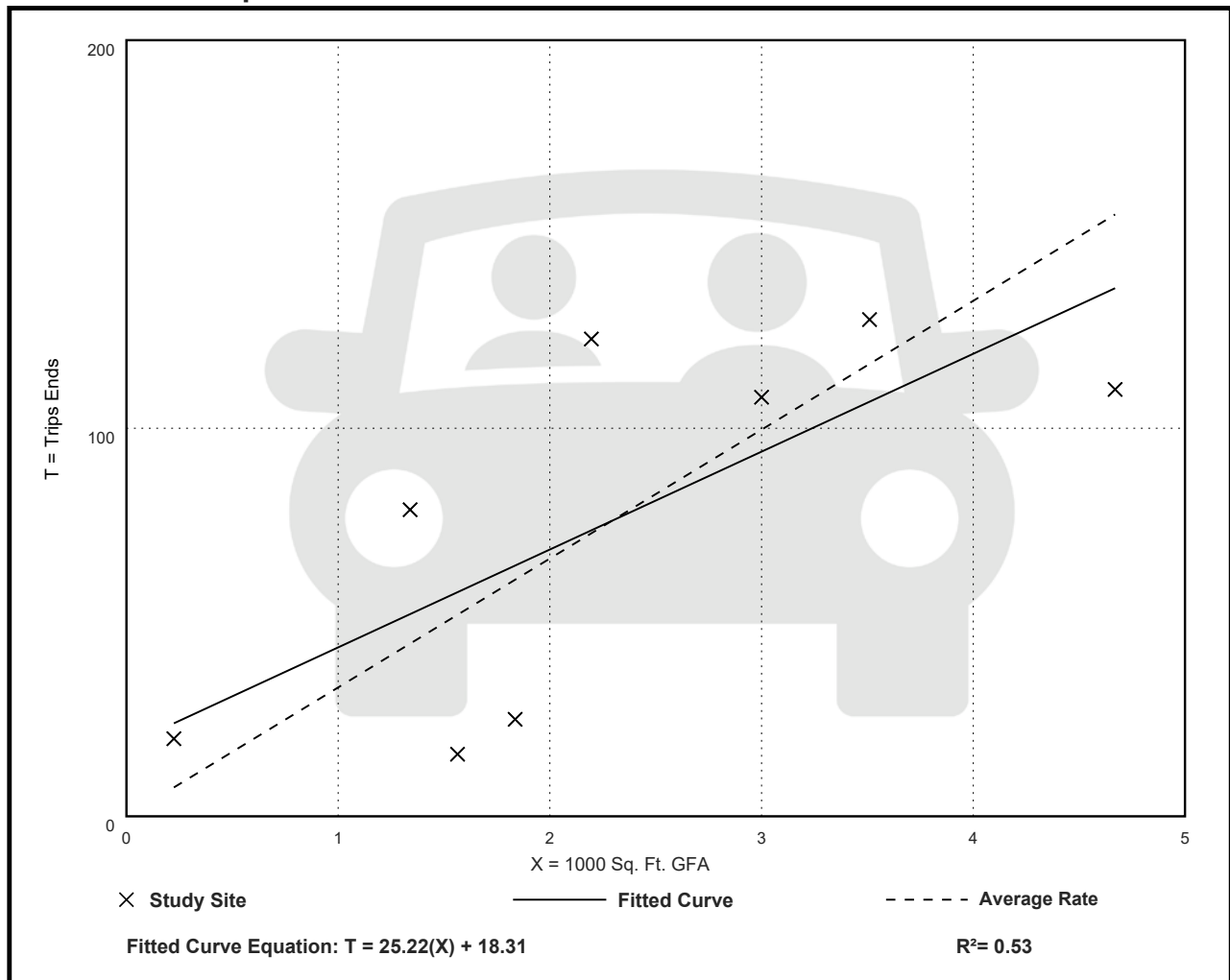
Avg. 1000 Sq. Ft. GFA: 2

Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
33.21	10.23 - 89.29	17.22

Data Plot and Equation



Fast-Food Restaurant without Drive-Through Window (933)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Saturday, Peak Hour of Generator

Setting/Location: General Urban/Suburban

Number of Studies: 1

Avg. 1000 Sq. Ft. GFA: 5

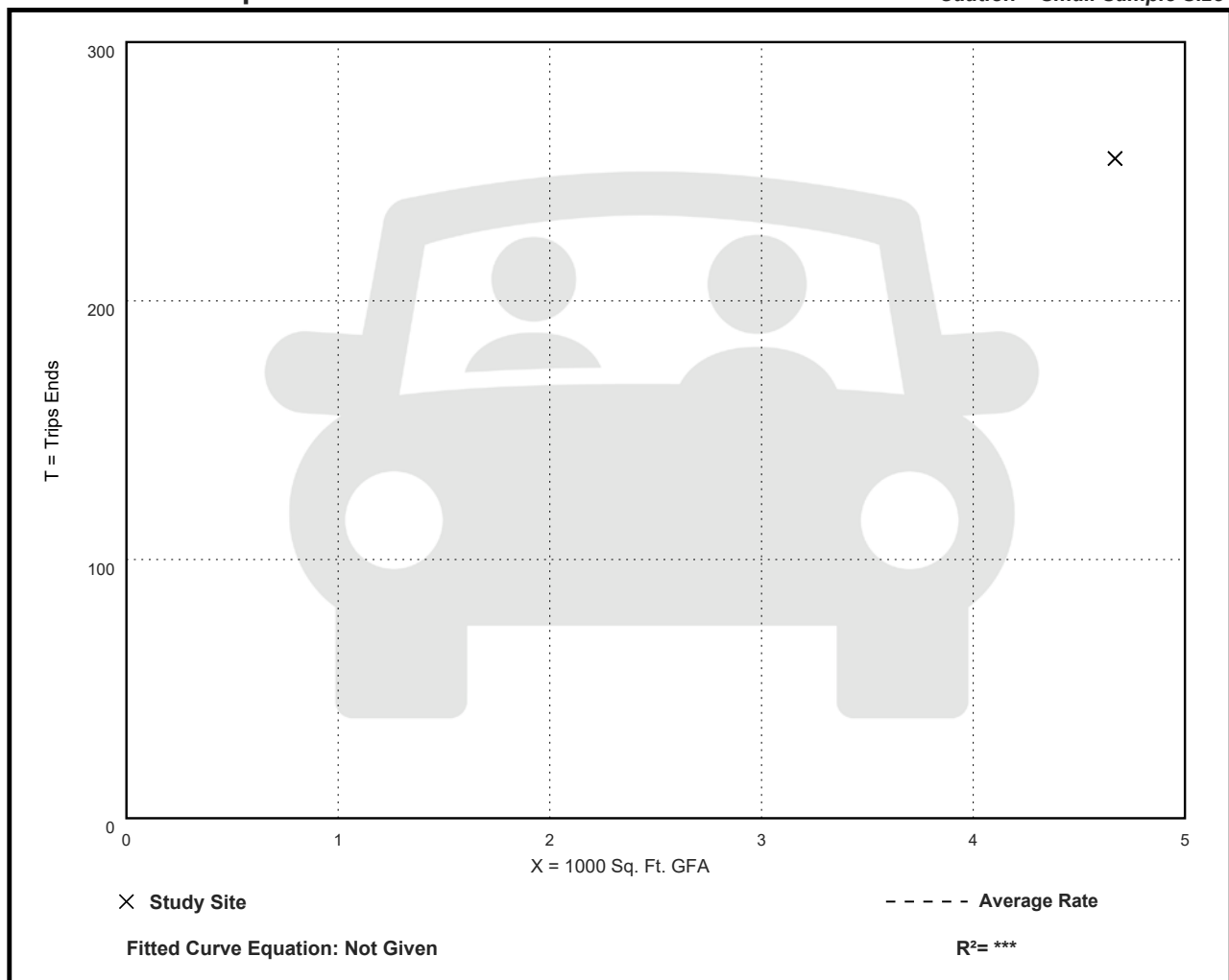
Directional Distribution: 49% entering, 51% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
54.60	54.60 - 54.60	***

Data Plot and Equation

Caution – Small Sample Size



Fast-Food Restaurant with Drive-Through Window (934)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban

Number of Studies: 96

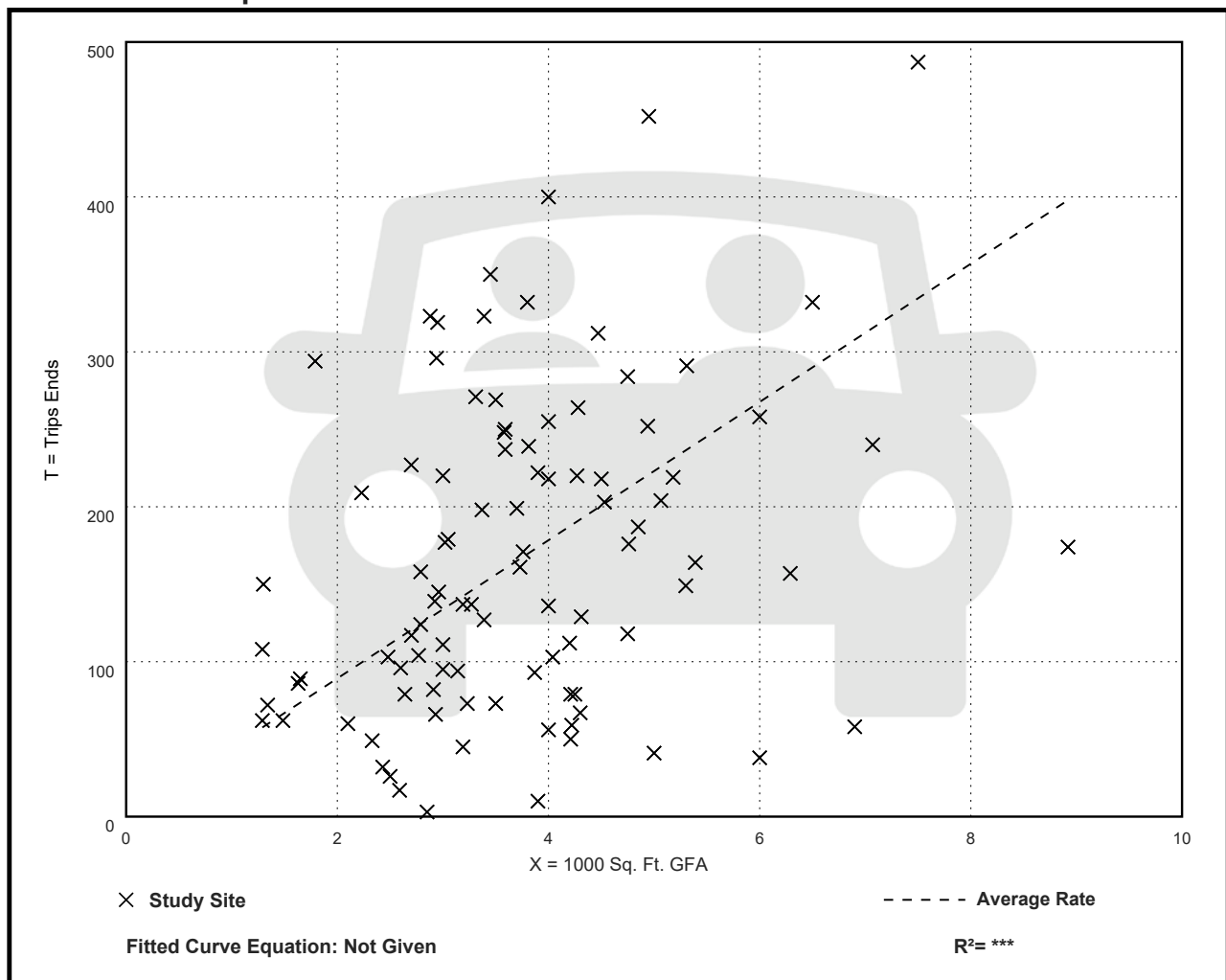
Avg. 1000 Sq. Ft. GFA: 4

Directional Distribution: 51% entering, 49% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
44.61	1.05 - 164.25	27.14

Data Plot and Equation



Fast-Food Restaurant with Drive-Through Window (934)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 4 and 6 p.m.

Setting/Location: General Urban/Suburban

Number of Studies: 190

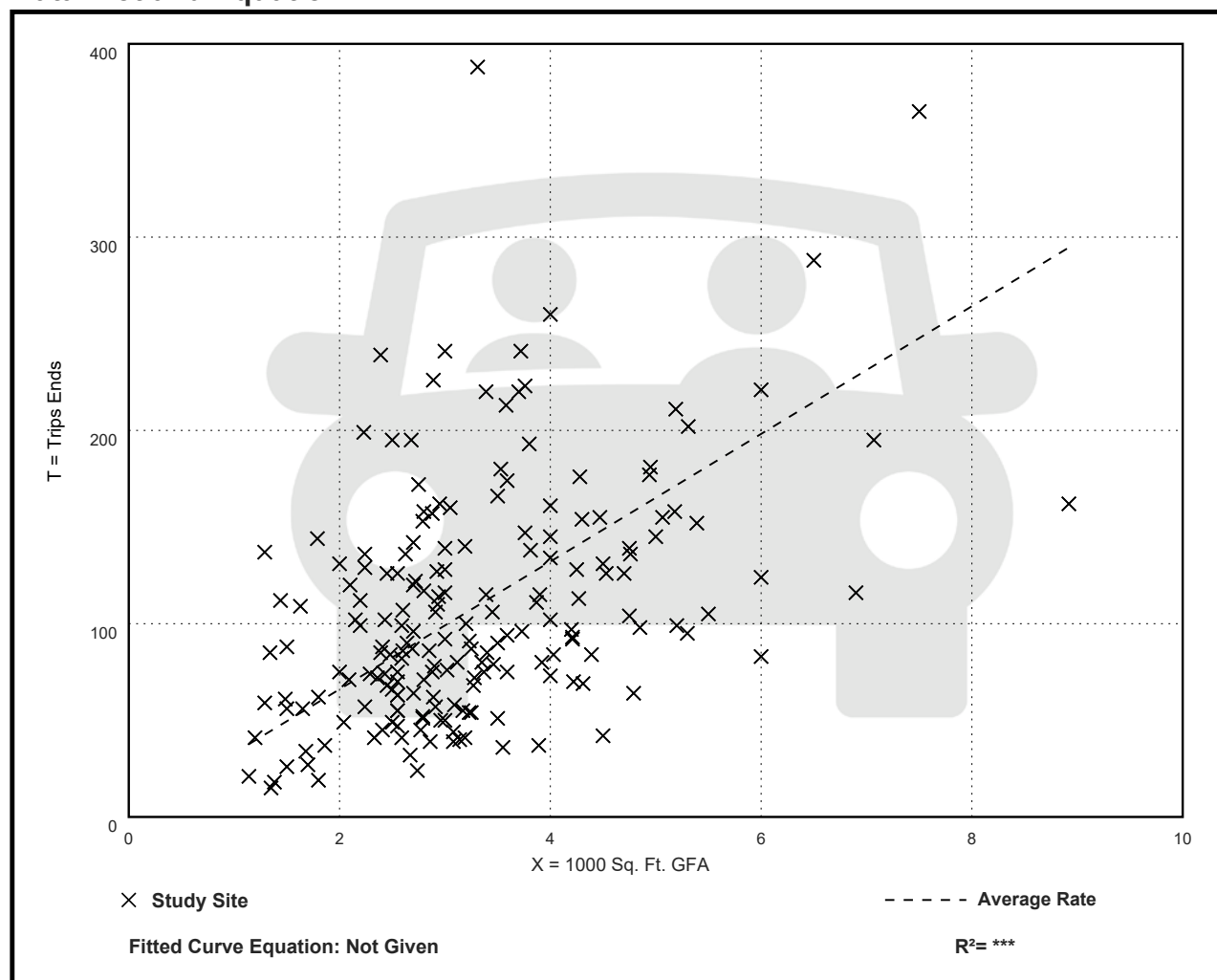
Avg. 1000 Sq. Ft. GFA: 3

Directional Distribution: 52% entering, 48% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
33.03	8.77 - 117.22	17.59

Data Plot and Equation



Fast-Food Restaurant with Drive-Through Window (934)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Saturday, Peak Hour of Generator

Setting/Location: General Urban/Suburban

Number of Studies: 53

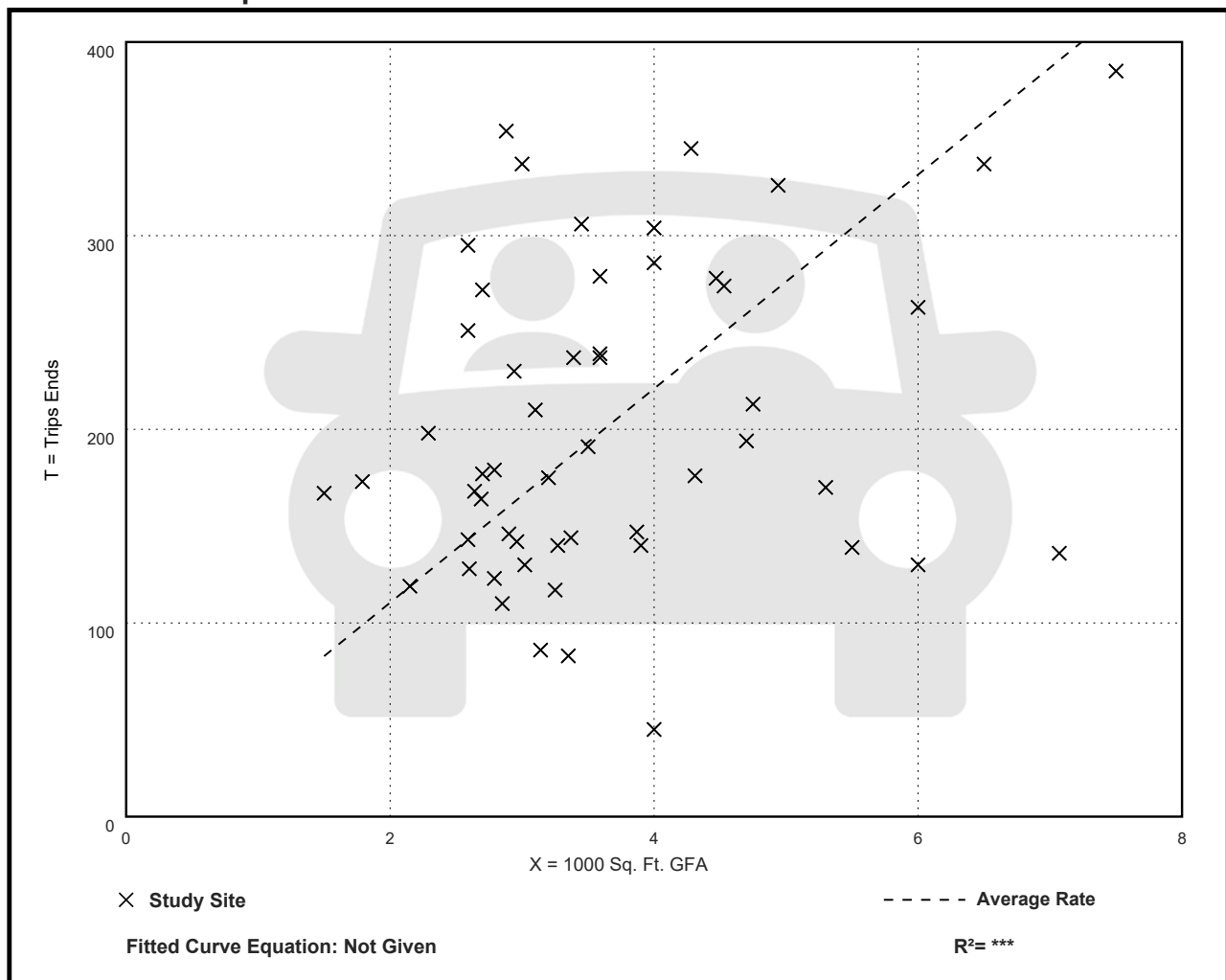
Avg. 1000 Sq. Ft. GFA: 4

Directional Distribution: 51% entering, 49% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
55.25	11.25 - 122.92	24.62

Data Plot and Equation



Brewery Tap Room (971)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban

Number of Studies: 2

Avg. 1000 Sq. Ft. GFA: 6

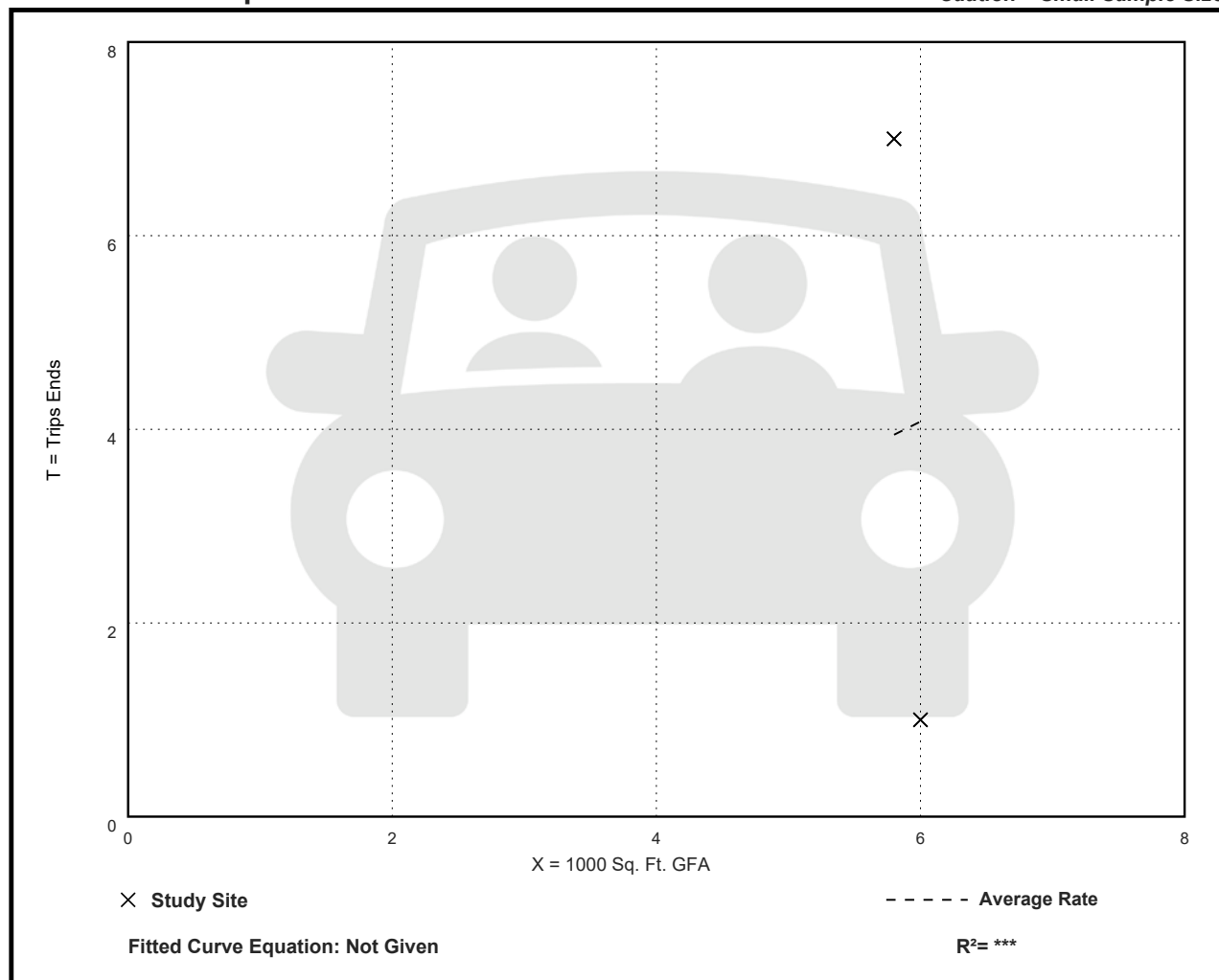
Directional Distribution: 88% entering, 12% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
0.68	0.17 - 1.21	***

Data Plot and Equation

Caution – Small Sample Size



Brewery Tap Room (971)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 4 and 6 p.m.

Setting/Location: General Urban/Suburban

Number of Studies: 2

Avg. 1000 Sq. Ft. GFA: 6

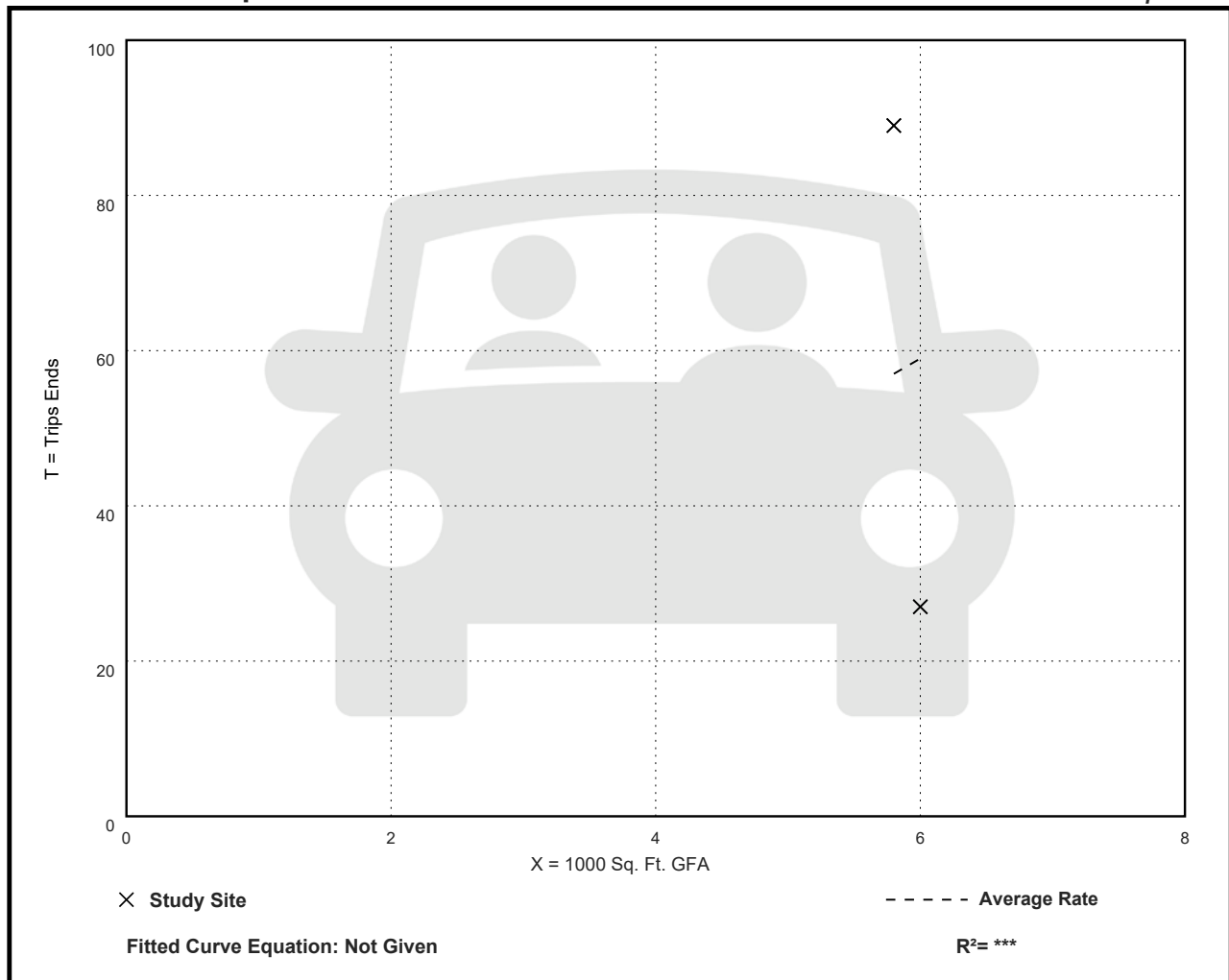
Directional Distribution: 59% entering, 41% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
9.83	4.50 - 15.34	***

Data Plot and Equation

Caution – Small Sample Size



Brewery Tap Room (971)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Saturday, Peak Hour of Generator

Setting/Location: General Urban/Suburban

Number of Studies: 2

Avg. 1000 Sq. Ft. GFA: 6

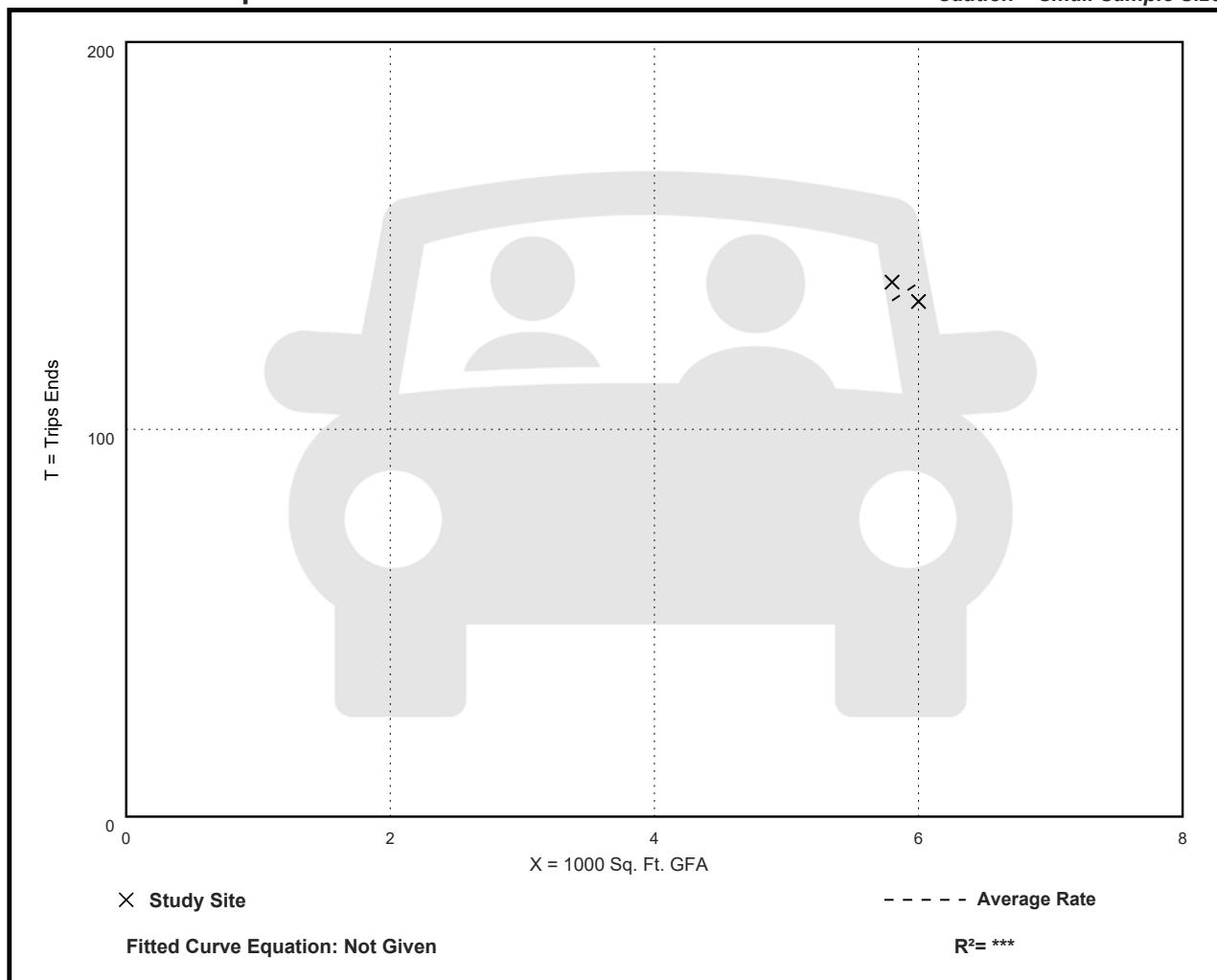
Directional Distribution: 56% entering, 44% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
22.97	22.17 - 23.79	***

Data Plot and Equation

Caution – Small Sample Size



CMAP 2050 Projections Letter



Chicago Metropolitan
Agency for Planning

433 West Van Buren Street, Suite 450
Chicago, IL 60607
cmap.illinois.gov | 312-454-0400

March 21, 2025

Ryan May
Project Coordinator
Kenig, Lindgren, O'Hara and Aboona, Inc.
9575 West Higgins Road
Suite 400
Rosemont, IL 60018

Subject: Roosevelt Road @ Finley Road
IDOT

Dear Ms. May:

In response to a request made on your behalf and dated March 20, 2025, we have developed year 2050 average daily traffic (ADT) projections for the subject location.

ROAD SEGMENT	Current ADT	Year 2050 ADT
Finley Rd north of Roosevelt Rd	6,800	7,750
Finley Rd south of Roosevelt Rd	18,700	21,300
Roosevelt Rd, at Finley Rd	34,300	37,900

Traffic projections are developed using existing ADT data provided in the request letter and the results from the December 2024 CMAP Travel Demand Analysis. The regional travel model uses CMAP 2050 socioeconomic projections and assumes the implementation of the ON TO 2050 Comprehensive Regional Plan for the Northeastern Illinois area. The provision of this data in support of your request does not constitute a CMAP endorsement of the proposed development or any subsequent developments.

If you have any questions, please call me at (312) 386-8806 or email me at jrodriguez@cmap.illinois.gov

Jose Rodriguez, PTP, AICP
Senior Planner, Research & Analysis

cc: Rios (IDOT)
\\2025_trafficForecasts\Lombard\du-18-25\du-18-25.docx

Level of Service Criteria

LEVEL OF SERVICE CRITERIA

Signalized Intersections		
Level of Service	Interpretation	Average Control Delay (seconds per vehicle)
A	Favorable progression. Most vehicles arrive during the green indication and travel through the intersection without stopping.	≤ 10
B	Good progression, with more vehicles stopping than for Level of Service A.	$> 10 - 20$
C	Individual cycle failures (i.e., one or more queued vehicles are not able to depart as a result of insufficient capacity during the cycle) may begin to appear. Number of vehicles stopping is significant, although many vehicles still pass through the intersection without stopping.	$> 20 - 35$
D	The volume-to-capacity ratio is high and either progression is ineffective or the cycle length is too long. Many vehicles stop and individual cycle failures are noticeable.	$> 35 - 55$
E	Progression is unfavorable. The volume-to-capacity ratio is high and the cycle length is long. Individual cycle failures are frequent.	$> 55 - 80$
F	The volume-to-capacity ratio is very high, progression is very poor, and the cycle length is long. Most cycles fail to clear the queue.	> 80
Unsignalized Intersections		
Level of Service		Average Total Delay (sec/veh)
A		0 - 10
B		$> 10 - 15$
C		$> 15 - 25$
D		$> 25 - 35$
E		$> 35 - 50$
F		> 50
Source: <i>Highway Capacity Manual</i> , 7 th Edition.		

Capacity Analysis Summary Sheets
Existing Weekday Morning Peak Hour

Lanes, Volumes, Timings

3: Finley Road & Roosevelt Road

05/20/2025

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	115	1073	313	143	1010	106	236	217	158	140	393	105
Future Volume (vph)	115	1073	313	143	1010	106	236	217	158	140	393	105
Ideal Flow (vphpl)	1900	2000	1900	1900	2000	1900	1900	2000	1900	1900	1900	1900
Storage Length (ft)	210		420	470		100	275		160	245		0
Storage Lanes	1		1	1		1	2		1	2		0
Taper Length (ft)	135			95			195			250		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	0.97	0.95	1.00	0.97	0.95	0.95
Frt			0.850			0.850			0.850		0.968	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1719	3654	1568	1787	3619	1568	3303	3725	1583	3335	3370	0
Flt Permitted	0.154			0.108			0.950			0.950		
Satd. Flow (perm)	279	3654	1568	203	3619	1568	3303	3725	1583	3335	3370	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			30			30	
Link Distance (ft)		623			970			675			547	
Travel Time (s)		12.1			18.9			15.3			12.4	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	5%	4%	3%	1%	5%	3%	6%	2%	2%	5%	2%	10%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	126	1179	344	157	1110	116	259	238	174	154	547	0
Turn Type	pm+pt	NA	pm+ov	pm+pt	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	
Protected Phases	5	2	3	1	6	7	3	8	1	7	4	
Permitted Phases	2		2	6		6			8			
Detector Phase	5	2	3	1	6	7	3	8	1	7	4	
Switch Phase												
Minimum Initial (s)	3.0	15.0	3.0	3.0	15.0	3.0	3.0	8.0	3.0	3.0	8.0	
Minimum Split (s)	9.5	44.0	9.5	9.5	24.0	9.5	9.5	32.0	9.5	9.5	24.0	
Total Split (s)	13.0	63.0	25.0	20.0	70.0	25.0	25.0	32.0	20.0	25.0	32.0	
Total Split (%)	9.3%	45.0%	17.9%	14.3%	50.0%	17.9%	17.9%	22.9%	14.3%	17.9%	22.9%	
Yellow Time (s)	3.5	4.0	3.5	3.5	4.0	3.5	3.5	4.0	3.5	3.5	4.0	
All-Red Time (s)	0.0	2.0	1.0	0.0	2.0	1.0	1.0	2.0	0.0	1.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	3.5	6.0	4.5	3.5	6.0	4.5	4.5	6.0	3.5	4.5	6.0	
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Recall Mode	None	C-Min	None	None	C-Min	None	None	None	None	None	None	
Act Effct Green (s)	75.6	64.2	86.6	81.1	67.5	85.3	16.4	31.9	50.0	11.8	27.3	
Actuated g/C Ratio	0.54	0.46	0.62	0.58	0.48	0.61	0.12	0.23	0.36	0.08	0.20	
v/c Ratio	0.52	0.70	0.36	0.62	0.64	0.12	0.67	0.28	0.31	0.55	0.83	
Control Delay (s/veh)	22.2	34.4	14.9	26.7	29.9	12.4	67.8	45.4	33.3	68.6	66.0	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay (s/veh)	22.2	34.4	14.9	26.7	29.9	12.4	67.8	45.4	33.3	68.6	66.0	
LOS	C	C	B	C	C	B	E	D	C	E	E	
Approach Delay (s/veh)		29.4			28.1			50.9			66.6	
Approach LOS		C			C			D			E	
Queue Length 50th (ft)	51	462	154	65	409	45	118	92	110	70	247	
Queue Length 95th (ft)	85	581	221	120	488	71	161	135	170	106	#353	

AMEX Existing Weekday Morning Peak Hour
sa

Synchro 12 Report

Lanes, Volumes, Timings

3: Finley Road & Roosevelt Road

05/20/2025

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)	543			890			595			467		
Turn Bay Length (ft)	210		420	470		100	275		160	245		
Base Capacity (vph)	249	1675	1015	305	1743	1052	483	848	614	488	666	
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.51	0.70	0.34	0.51	0.64	0.11	0.54	0.28	0.28	0.32	0.82	

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 133 (95%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 95

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.83

Intersection Signal Delay (s/veh): 38.2

Intersection LOS: D

Intersection Capacity Utilization 74.1%

ICU Level of Service D

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 3: Finley Road & Roosevelt Road

	Ø1		Ø2 (R)		Ø3		Ø4
20 s		63 s		25 s		32 s	
	Ø5		Ø6 (R)		Ø7		Ø8
13 s		70 s		25 s		32 s	

Intersection												
Int Delay, s/veh	0											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	7	1501	1	3	1342	6	0	0	0	0	0	0
Future Vol, veh/h	7	1501	1	3	1342	6	0	0	0	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	160	-	-	150	-	-	-	-	0	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	1	-	-	1	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	91	91	91	91	91	91	91	91	91	91	91	91
Heavy Vehicles, %	14	4	0	0	5	0	0	0	0	0	0	0
Mvmt Flow	8	1649	1	3	1475	7	0	0	0	0	0	0

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	1481	0	0	1651	0	0	2409	3153	825	2325	3151	741
Stage 1	-	-	-	-	-	-	1665	1665	-	1485	1485	-
Stage 2	-	-	-	-	-	-	744	1488	-	840	1666	-
Critical Hdwy	4.38	-	-	4.1	-	-	7.5	6.5	6.9	7.5	6.5	6.9
Critical Hdwy Stg 1	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Follow-up Hdwy	2.34	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	549	-	-	513	-	-	*82	1	*664	*140	1	*710
Stage 1	-	-	-	-	-	-	*210	241	-	*271	298	-
Stage 2	-	-	-	-	-	-	*669	296	-	*627	241	-
Platoon blocked, %	0	-	-	0	-	-	1	1	0	1	1	0
Mov Cap-1 Maneuver	549	-	-	513	-	-	*80	1	*664	*137	1	*710
Mov Cap-2 Maneuver	-	-	-	-	-	-	*168	119	-	*220	120	-
Stage 1	-	-	-	-	-	-	*207	238	-	*270	296	-
Stage 2	-	-	-	-	-	-	*665	295	-	*618	238	-

Approach	EB	WB	NB	SB
HCM Ctrl Dly, s/v	0.05	0.03	0	0
HCM LOS			A	A

Minor Lane/Major Mvmt	NBLn1	NBLn2	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	-	-	549	-	-	513	-	-	-
HCM Lane V/C Ratio	-	-	0.014	-	-	0.006	-	-	-
HCM Ctrl Dly (s/v)	0	0	11.7	-	-	12.1	-	-	0
HCM Lane LOS	A	A	B	-	-	B	-	-	A
HCM 95th %tile Q(veh)	-	-	0	-	-	0	-	-	-

Notes	
~: Volume exceeds capacity	\$. Delay exceeds 300s
+: Computation Not Defined	*: All major volume in platoon

Intersection

Int Delay, s/veh 0.8

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕		↕	↕	
Traffic Vol, veh/h	0	0	4	45	0	25	1	583	25	20	829	0
Future Vol, veh/h	0	0	4	45	0	25	1	583	25	20	829	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	120	-	-
Veh in Median Storage, #	-	0	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	97	97	97	97	97	97	97	97	97	97	97	97
Heavy Vehicles, %	0	0	50	3	0	4	0	2	0	5	2	0
Mvmt Flow	0	0	4	46	0	26	1	601	26	21	855	0

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	1198	1525	427	1085	1512	313	855	0	0	627	0	0
Stage 1	896	896	-	616	616	-	-	-	-	-	-	-
Stage 2	303	629	-	469	896	-	-	-	-	-	-	-
Critical Hdwy	7.5	6.5	7.9	7.56	6.5	6.98	4.1	-	-	4.2	-	-
Critical Hdwy Stg 1	6.5	5.5	-	6.56	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.5	5.5	-	6.56	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.8	3.53	4	3.34	2.2	-	-	2.25	-	-
Pot Cap-1 Maneuver	257	163	*744	*321	167	676	1027	-	-	931	-	-
Stage 1	527	515	-	*442	485	-	-	-	-	-	-	-
Stage 2	688	479	-	*801	515	-	-	-	-	-	-	-
Platoon blocked, %	0	0	0	0	0	0	-	-	-	-	-	-
Mov Cap-1 Maneuver	241	159	*744	*312	163	676	1027	-	-	931	-	-
Mov Cap-2 Maneuver	241	159	-	*377	300	-	-	-	-	-	-	-
Stage 1	516	504	-	*442	484	-	-	-	-	-	-	-
Stage 2	661	478	-	*779	504	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Ctrl Dly, s/v	9.87	14.57	0.02	0.21
HCM LOS	A	B		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	6	-	-	744	448	931	-	-
HCM Lane V/C Ratio	0.001	-	-	0.006	0.161	0.022	-	-
HCM Ctrl Dly (s/v)	8.5	0	-	9.9	14.6	9	-	-
HCM Lane LOS	A	A	-	A	B	A	-	-
HCM 95th %tile Q(veh)	0	-	-	0	0.6	0.1	-	-

Notes

~: Volume exceeds capacity \$: Delay exceeds 300s
 +: Computation Not Defined *: All major volume in platoon

Intersection							
Int Delay, s/veh	1.1						
Movement	EBL	EBR	NBL	NBT	SBU	SBT	SBR
Lane Configurations							
Traffic Vol, veh/h	35	40	10	562	12	853	12
Future Vol, veh/h	35	40	10	562	12	853	12
Conflicting Peds, #/hr	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	-	None
Storage Length	0	-	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	-	0	-
Grade, %	0	-	-	0	-	0	-
Peak Hour Factor	96	96	96	96	96	96	96
Heavy Vehicles, %	3	13	0	3	0	2	17
Mvmt Flow	36	42	10	585	13	889	13

Major/Minor	Minor2	Major1		Major2			
Conflicting Flow All	1233	451	901	0	585	-	0
Stage 1	920	-	-	-	-	-	-
Stage 2	314	-	-	-	-	-	-
Critical Hdwy	6.86	7.16	4.1	-	6.4	-	-
Critical Hdwy Stg 1	5.86	-	-	-	-	-	-
Critical Hdwy Stg 2	5.86	-	-	-	-	-	-
Follow-up Hdwy	3.53	3.43	2.2	-	2.5	-	-
Pot Cap-1 Maneuver	271	*824	977	-	619	-	-
Stage 1	535	-	-	-	-	-	-
Stage 2	711	-	-	-	-	-	-
Platoon blocked, %	0	0	0	-	-	-	-
Mov Cap-1 Maneuver	260	*824	977	-	619	-	-
Mov Cap-2 Maneuver	260	-	-	-	-	-	-
Stage 1	528	-	-	-	-	-	-
Stage 2	692	-	-	-	-	-	-

Approach	EB	NB	SB
HCM Ctrl Dly, s/v	15.84	0.26	0.43
HCM LOS	C		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	63	-	410	-	-
HCM Lane V/C Ratio	0.011	-	0.191	-	-
HCM Ctrl Dly (s/v)	8.7	0.1	15.8	0.3	-
HCM Lane LOS	A	A	C	A	-
HCM 95th %tile Q(veh)	0	-	0.7	-	-

Notes	
~: Volume exceeds capacity	\$. Delay exceeds 300s
+: Computation Not Defined	*: All major volume in platoon

Capacity Analysis Summary Sheets
Existing Weekday Evening Peak Hour

Lanes, Volumes, Timings

3: Finley Road & Roosevelt Road

06/02/2025

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	100	1117	303	208	1147	104	420	341	231	198	371	118
Future Volume (vph)	100	1117	303	208	1147	104	420	341	231	198	371	118
Ideal Flow (vphpl)	1900	2000	1900	1900	2000	1900	1900	2000	1900	1900	1900	1900
Storage Length (ft)	210		420	470		100	275		160	245		0
Storage Lanes	1		1	1		1	2		1	2		0
Taper Length (ft)	135			95			195			250		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	0.97	0.95	1.00	0.97	0.95	0.95
Frt			0.850			0.850			0.850		0.964	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1787	3762	1599	1805	3762	1583	3502	3762	1599	3502	3446	0
Flt Permitted	0.133			0.112			0.950			0.950		
Satd. Flow (perm)	250	3762	1599	213	3762	1583	3502	3762	1599	3502	3446	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			30			30	
Link Distance (ft)		623			970			675			547	
Travel Time (s)		12.1			18.9			15.3			12.4	
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Heavy Vehicles (%)	1%	1%	1%	0%	1%	2%	0%	1%	1%	0%	1%	1%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	102	1140	309	212	1170	106	429	348	236	202	499	0
Turn Type	pm+pt	NA	pm+ov	pm+pt	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	
Protected Phases	5	2	3	1	6	7	3	8	1	7	4	
Permitted Phases	2		2	6		6			8			
Detector Phase	5	2	3	1	6	7	3	8	1	7	4	
Switch Phase												
Minimum Initial (s)	3.0	15.0	3.0	3.0	15.0	3.0	3.0	8.0	3.0	3.0	8.0	
Minimum Split (s)	9.5	44.0	9.5	9.5	24.0	9.5	9.5	32.0	9.5	9.5	24.0	
Total Split (s)	17.0	68.0	34.0	20.0	71.0	22.0	34.0	40.0	20.0	22.0	28.0	
Total Split (%)	11.3%	45.3%	22.7%	13.3%	47.3%	14.7%	22.7%	26.7%	13.3%	14.7%	18.7%	
Yellow Time (s)	3.5	4.0	3.5	3.5	4.0	3.5	3.5	4.0	3.5	3.5	4.0	
All-Red Time (s)	0.0	2.0	1.0	0.0	2.0	1.0	1.0	2.0	0.0	1.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	-1.0	0.0	0.0	0.0	0.0	0.0	0.0	-1.5	
Total Lost Time (s)	3.5	6.0	4.5	2.5	6.0	4.5	4.5	6.0	3.5	4.5	4.5	
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Recall Mode	None	C-Min	None	None	C-Min	None	None	None	None	None	None	
Act Effct Green (s)	78.2	66.3	96.3	87.7	71.3	91.2	24.0	35.4	55.8	13.9	26.8	
Actuated g/C Ratio	0.52	0.44	0.64	0.58	0.48	0.61	0.16	0.24	0.37	0.09	0.18	
v/c Ratio	0.45	0.69	0.30	0.74	0.65	0.11	0.77	0.39	0.40	0.62	0.81	
Control Delay (s/veh)	21.9	37.1	13.1	36.9	32.9	13.2	69.5	49.7	36.6	73.9	70.1	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay (s/veh)	21.9	37.1	13.1	36.9	32.9	13.2	69.5	49.7	36.6	73.9	70.1	
LOS	C	D	B	D	C	B	E	D	D	E	E	
Approach Delay (s/veh)		31.3			32.1			55.1			71.2	
Approach LOS		C			C			E			E	
Queue Length 50th (ft)	45	487	136	98	465	43	209	148	162	100	246	
Queue Length 95th (ft)	75	575	176	191	564	72	259	204	247	140	#364	

Lanes, Volumes, Timings

3: Finley Road & Roosevelt Road

06/02/2025

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)	543			890			595			467		
Turn Bay Length (ft)	210		420	470		100	275		160	245		
Base Capacity (vph)	275	1662	1084	310	1787	999	688	898	617	408	616	
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.37	0.69	0.29	0.68	0.65	0.11	0.62	0.39	0.38	0.50	0.81	

Intersection Summary

Area Type: Other

Cycle Length: 150

Actuated Cycle Length: 150

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 95

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.81

Intersection Signal Delay (s/veh): 42.5

Intersection LOS: D

Intersection Capacity Utilization 82.7%

ICU Level of Service E

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 3: Finley Road & Roosevelt Road

 Ø1	 Ø2 (R)	 Ø3	 Ø4
20 s	68 s	34 s	28 s
 Ø5	 Ø6 (R)	 Ø7	 Ø8
17 s	71 s	22 s	40 s

Intersection

Int Delay, s/veh 0.1

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	9	1514	4	4	1678	3	0	0	5	1	0	1
Future Vol, veh/h	9	1514	4	4	1678	3	0	0	5	1	0	1
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	160	-	-	150	-	-	-	-	0	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	1	-	-	1	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	97	97	97	97	97	97	97	97	97	97	97	97
Heavy Vehicles, %	0	1	0	0	1	0	0	0	0	0	0	0
Mvmt Flow	9	1561	4	4	1730	3	0	0	5	1	0	1

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	1733	0	0	1565	0	0	2455	3323	782	2539	3323	866
Stage 1	-	-	-	-	-	-	1581	1581	-	1740	1740	-
Stage 2	-	-	-	-	-	-	873	1741	-	799	1584	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.5	6.5	6.9	7.5	6.5	6.9
Critical Hdwy Stg 1	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	493	-	-	592	-	-	*52	1	*651	*31	1	*608
Stage 1	-	-	-	-	-	-	*266	288	-	*211	238	-
Stage 2	-	-	-	-	-	-	*574	237	-	*613	287	-
Platoon blocked, %	0	-	-	0	-	-	1	1	0	1	1	0
Mov Cap-1 Maneuver	493	-	-	592	-	-	*51	1	*651	*30	1	*608
Mov Cap-2 Maneuver	-	-	-	-	-	-	*182	115	-	*152	117	-
Stage 1	-	-	-	-	-	-	*261	282	-	*210	236	-
Stage 2	-	-	-	-	-	-	*569	235	-	*597	281	-

Approach	EB	WB	NB	SB
HCM Ctrl Dly, s/v	0.07	0.03	10.58	19.93
HCM LOS			B	C

Minor Lane/Major Mvmt	NBLn1	NBLn2	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	-	651	493	-	-	592	-	-	243
HCM Lane V/C Ratio	-	0.008	0.019	-	-	0.007	-	-	0.008
HCM Ctrl Dly (s/v)	0	10.6	12.4	-	-	11.1	-	-	19.9
HCM Lane LOS	A	B	B	-	-	B	-	-	C
HCM 95th %tile Q(veh)	-	0	0.1	-	-	0	-	-	0

Notes

~: Volume exceeds capacity \$: Delay exceeds 300s
 +: Computation Not Defined *: All major volume in platoon

Intersection

Int Delay, s/veh 1.1

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕		↕	↕	
Traffic Vol, veh/h	0	0	1	48	2	24	2	974	63	26	856	0
Future Vol, veh/h	0	0	1	48	2	24	2	974	63	26	856	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	120	-	-
Veh in Median Storage, #	-	0	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	97	97	97	97	97	97	97	97	97	97	97	97
Heavy Vehicles, %	0	0	0	0	0	4	0	1	0	4	0	0
Mvmt Flow	0	0	1	49	2	25	2	1004	65	27	882	0

Major/Minor	Minor2		Minor1		Major1		Major2		Major2		Major2	
Conflicting Flow All	1443	2009	441	1536	1977	535	882	0	0	1069	0	0
Stage 1	936	936	-	1041	1041	-	-	-	-	-	-	-
Stage 2	507	1073	-	495	936	-	-	-	-	-	-	-
Critical Hdwy	7.5	6.5	6.9	7.5	6.5	6.98	4.1	-	-	4.18	-	-
Critical Hdwy Stg 1	6.5	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.5	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.34	2.2	-	-	2.24	-	-
Pot Cap-1 Maneuver	158	70	*843	*130	74	485	1017	-	-	636	-	-
Stage 1	512	502	-	*250	310	-	-	-	-	-	-	-
Stage 2	521	299	-	*795	502	-	-	-	-	-	-	-
Platoon blocked, %	0	0	0	0	0	0	-	-	-	-	-	-
Mov Cap-1 Maneuver	142	67	*843	*124	71	485	1017	-	-	636	-	-
Mov Cap-2 Maneuver	142	67	-	*208	202	-	-	-	-	-	-	-
Stage 1	490	481	-	*249	309	-	-	-	-	-	-	-
Stage 2	490	298	-	*761	481	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Ctrl Dly, s/v	9.27	24.98	0.04	0.32
HCM LOS	A	C		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	6	-	-	843	255	636	-	-
HCM Lane V/C Ratio	0.002	-	-	0.001	0.299	0.042	-	-
HCM Ctrl Dly (s/v)	8.5	0	-	9.3	25	10.9	-	-
HCM Lane LOS	A	A	-	A	C	B	-	-
HCM 95th %tile Q(veh)	0	-	-	0	1.2	0.1	-	-

Notes

~: Volume exceeds capacity \$: Delay exceeds 300s
 +: Computation Not Defined *: All major volume in platoon

Intersection							
Int Delay, s/veh	1.8						
Movement	EBL	EBR	NBL	NBT	SBU	SBT	SBR
Lane Configurations							
Traffic Vol, veh/h	29	24	50	998	12	848	35
Future Vol, veh/h	29	24	50	998	12	848	35
Conflicting Peds, #/hr	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	-	None
Storage Length	0	-	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	-	0	-
Grade, %	0	-	-	0	-	0	-
Peak Hour Factor	95	95	95	95	95	95	95
Heavy Vehicles, %	0	4	2	1	0	1	0
Mvmt Flow	31	25	53	1051	13	893	37

Major/Minor	Minor2	Major1		Major2			
Conflicting Flow All	1567	465	929	0	1051	-	0
Stage 1	936	-	-	-	-	-	-
Stage 2	631	-	-	-	-	-	-
Critical Hdwy	6.8	6.98	4.14	-	6.4	-	-
Critical Hdwy Stg 1	5.8	-	-	-	-	-	-
Critical Hdwy Stg 2	5.8	-	-	-	-	-	-
Follow-up Hdwy	3.5	3.34	2.22	-	2.5	-	-
Pot Cap-1 Maneuver	151	*833	955	-	313	-	-
Stage 1	546	-	-	-	-	-	-
Stage 2	498	-	-	-	-	-	-
Platoon blocked, %	0	0	0	-	-	-	-
Mov Cap-1 Maneuver	132	*833	955	-	313	-	-
Mov Cap-2 Maneuver	132	-	-	-	-	-	-
Stage 1	503	-	-	-	-	-	-
Stage 2	471	-	-	-	-	-	-

Approach	EB	NB	SB
HCM Ctrl Dly, s/v	27.77	1.03	1.08
HCM LOS	D		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	172	-	213	-	-
HCM Lane V/C Ratio	0.055	-	0.262	-	-
HCM Ctrl Dly (s/v)	9	0.6	27.8	0.9	-
HCM Lane LOS	A	A	D	A	-
HCM 95th %tile Q(veh)	0.2	-	1	-	-

Notes	
~: Volume exceeds capacity	\$: Delay exceeds 300s
+: Computation Not Defined	*: All major volume in platoon

Capacity Analysis Summary Sheets
Existing Saturday Midday Peak Hour

Lanes, Volumes, Timings

3: Finley Road & Roosevelt Road

05/20/2025

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	80	1026	259	199	966	116	294	260	199	123	250	83
Future Volume (vph)	80	1026	259	199	966	116	294	260	199	123	250	83
Ideal Flow (vphpl)	1900	2000	1900	1900	2000	1900	1900	2000	1900	1900	1900	1900
Storage Length (ft)	210		420	470		100	275		160	245		0
Storage Lanes	1		1	1		1	2		1	2		0
Taper Length (ft)	135			95			195			250		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	0.97	0.95	1.00	0.97	0.95	0.95
Frt			0.850			0.850			0.850		0.962	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1752	3762	1615	1787	3762	1599	3502	3762	1599	3502	3464	0
Flt Permitted	0.222			0.160			0.950			0.950		
Satd. Flow (perm)	410	3762	1615	301	3762	1599	3502	3762	1599	3502	3464	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			30			30	
Link Distance (ft)		623			970			675			547	
Travel Time (s)		12.1			18.9			15.3			12.4	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles (%)	3%	1%	0%	1%	1%	1%	0%	1%	1%	0%	0%	1%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	82	1058	267	205	996	120	303	268	205	127	344	0
Turn Type	pm+pt	NA	pm+ov	pm+pt	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	
Protected Phases	5	2	3	1	6	7	3	8	1	7	4	
Permitted Phases	2		2	6		6			8			
Detector Phase	5	2	3	1	6	7	3	8	1	7	4	
Switch Phase												
Minimum Initial (s)	3.0	15.0	3.0	3.0	15.0	3.0	3.0	8.0	3.0	3.0	8.0	
Minimum Split (s)	9.5	44.0	9.5	9.5	24.0	9.5	9.5	25.0	9.5	9.5	24.0	
Total Split (s)	13.0	60.0	22.0	23.0	70.0	22.0	22.0	25.0	23.0	22.0	25.0	
Total Split (%)	10.0%	46.2%	16.9%	17.7%	53.8%	16.9%	16.9%	19.2%	17.7%	16.9%	19.2%	
Yellow Time (s)	3.5	4.0	3.5	3.5	4.0	3.5	3.5	4.0	3.5	3.5	4.0	
All-Red Time (s)	0.0	2.0	1.0	0.0	2.0	1.0	1.0	2.0	0.0	1.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	3.5	6.0	4.5	3.5	6.0	4.5	4.5	6.0	3.5	4.5	6.0	
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Recall Mode	None	C-Min	None	None	C-Min	None	None	None	None	None	None	
Act Effct Green (s)	73.7	63.4	85.1	81.7	68.2	84.3	15.7	23.9	42.6	10.1	18.3	
Actuated g/C Ratio	0.57	0.49	0.65	0.63	0.52	0.65	0.12	0.18	0.33	0.08	0.14	
v/c Ratio	0.26	0.58	0.25	0.61	0.50	0.12	0.72	0.39	0.39	0.47	0.71	
Control Delay (s/veh)	12.6	26.5	10.8	18.9	21.7	9.5	65.0	48.3	35.2	62.8	61.8	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay (s/veh)	12.6	26.5	10.8	18.9	21.7	9.5	65.0	48.3	35.2	62.8	61.8	
LOS	B	C	B	B	C	A	E	D	D	E	E	
Approach Delay (s/veh)		22.7			20.2			51.4			62.1	
Approach LOS		C			C			D			E	
Queue Length 50th (ft)	27	335	89	71	288	38	127	103	130	53	145	
Queue Length 95th (ft)	49	450	151	110	359	63	176	150	191	85	201	

SATEX Existing Saturday Midday Peak Hour
sa

Synchro 12 Report

Lanes, Volumes, Timings

3: Finley Road & Roosevelt Road

05/20/2025

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)	543			890			595			467		
Turn Bay Length (ft)	210		420	470		100	275		160	245		
Base Capacity (vph)	335	1834	1079	412	1974	1128	471	691	607	471	511	
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.24	0.58	0.25	0.50	0.50	0.11	0.64	0.39	0.34	0.27	0.67	

Intersection Summary

Area Type: Other

Cycle Length: 130

Actuated Cycle Length: 130

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.72

Intersection Signal Delay (s/veh): 32.1

Intersection LOS: C

Intersection Capacity Utilization 73.0%

ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 3: Finley Road & Roosevelt Road

 Ø1	 Ø2 (R)	 Ø3	 Ø4
23 s	60 s	22 s	25 s
 Ø5	 Ø6 (R)	 Ø7	 Ø8
13 s	70 s	22 s	25 s

Intersection												
Int Delay, s/veh	0.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	29	1359	3	8	1333	2	0	0	5	1	0	0
Future Vol, veh/h	29	1359	3	8	1333	2	0	0	5	1	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	160	-	-	150	-	-	-	-	0	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	1	-	-	1	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	96	96	96	96	96	96	96	96	96	96	96	96
Heavy Vehicles, %	0	1	0	0	1	0	0	0	0	0	0	0
Mvmt Flow	30	1416	3	8	1389	2	0	0	5	1	0	0

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	1391	0	0	1419	0	0	2189	2885	709	2174	2885	695
Stage 1	-	-	-	-	-	-	1478	1478	-	1406	1406	-
Stage 2	-	-	-	-	-	-	711	1407	-	768	1479	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.5	6.5	6.9	7.5	6.5	6.9
Critical Hdwy Stg 1	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	680	-	-	659	-	-	*26	16	*710	*27	16	*713
Stage 1	-	-	-	-	-	-	*275	301	-	*317	335	-
Stage 2	-	-	-	-	-	-	*672	335	-	*669	301	-
Platoon blocked, %	0	-	-	0	-	-			0			0
Mov Cap-1 Maneuver	680	-	-	659	-	-	*25	16	*710	*25	16	*713
Mov Cap-2 Maneuver	-	-	-	-	-	-	*179	141	-	*200	145	-
Stage 1	-	-	-	-	-	-	*263	288	-	*313	331	-
Stage 2	-	-	-	-	-	-	*664	331	-	*635	287	-

Approach	EB	WB	NB	SB
HCM Ctrl Dly, s/v	0.22	0.06	10.11	23.06
HCM LOS			B	C

Minor Lane/Major Mvmt	NBLn1	NBLn2	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	-	710	680	-	-	659	-	-	200
HCM Lane V/C Ratio	-	0.007	0.044	-	-	0.013	-	-	0.005
HCM Ctrl Dly (s/v)	0	10.1	10.5	-	-	10.5	-	-	23.1
HCM Lane LOS	A	B	B	-	-	B	-	-	C
HCM 95th %tile Q(veh)	-	0	0.1	-	-	0	-	-	0

Notes	
~: Volume exceeds capacity	\$. Delay exceeds 300s
+: Computation Not Defined	*: All major volume in platoon

Intersection												
Int Delay, s/veh	1.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕		↕	↕	
Traffic Vol, veh/h	0	0	2	49	1	31	2	733	36	29	679	0
Future Vol, veh/h	0	0	2	49	1	31	2	733	36	29	679	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	120	-	-
Veh in Median Storage, #	-	0	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	0	0	0	0	0	3	0	1	0	0	0	0
Mvmt Flow	0	0	2	54	1	34	2	814	40	32	754	0

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	1231	1678	377	1281	1658	427	754	0	0	854	0	0
Stage 1	819	819	-	839	839	-	-	-	-	-	-	-
Stage 2	412	859	-	442	819	-	-	-	-	-	-	-
Critical Hdwy	7.5	6.5	6.9	7.5	6.5	6.96	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.5	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.5	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.33	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	208	117	*904	*188	121	573	1063	-	-	794	-	-
Stage 1	521	517	-	*331	384	-	-	-	-	-	-	-
Stage 2	593	376	-	*853	517	-	-	-	-	-	-	-
Platoon blocked, %	0	0	0	0	0	0	0	-	-	-	-	-
Mov Cap-1 Maneuver	186	112	*904	*179	115	573	1063	-	-	794	-	-
Mov Cap-2 Maneuver	186	112	-	*275	248	-	-	-	-	-	-	-
Stage 1	500	496	-	*330	383	-	-	-	-	-	-	-
Stage 2	554	375	-	*816	496	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Ctrl Dly, s/v	8.99	19.21	0.04	0.4
HCM LOS	A	C		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	9	-	-	904	343	794	-	-
HCM Lane V/C Ratio	0.002	-	-	0.002	0.263	0.041	-	-
HCM Ctrl Dly (s/v)	8.4	0	-	9	19.2	9.7	-	-
HCM Lane LOS	A	A	-	A	C	A	-	-
HCM 95th %tile Q(veh)	0	-	-	0	1	0.1	-	-

Notes	
~: Volume exceeds capacity	\$. Delay exceeds 300s
+: Computation Not Defined	*: All major volume in platoon

Intersection							
Int Delay, s/veh	1.1						
Movement	EBL	EBR	NBL	NBT	SBU	SBT	SBR
Lane Configurations							
Traffic Vol, veh/h	32	22	25	730	9	703	18
Future Vol, veh/h	32	22	25	730	9	703	18
Conflicting Peds, #/hr	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	-	None
Storage Length	0	-	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	-	0	-
Grade, %	0	-	-	0	-	0	-
Peak Hour Factor	91	91	91	91	91	92	91
Heavy Vehicles, %	0	0	0	1	0	0	0
Mvmt Flow	35	24	27	802	10	764	20

Major/Minor	Minor2	Major1		Major2			
Conflicting Flow All	1250	392	784	0	802	-	0
Stage 1	794	-	-	-	-	-	-
Stage 2	456	-	-	-	-	-	-
Critical Hdwy	6.8	6.9	4.1	-	6.4	-	-
Critical Hdwy Stg 1	5.8	-	-	-	-	-	-
Critical Hdwy Stg 2	5.8	-	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	2.5	-	-
Pot Cap-1 Maneuver	247	*888	1056	-	451	-	-
Stage 1	602	-	-	-	-	-	-
Stage 2	611	-	-	-	-	-	-
Platoon blocked, %	0	0	0	-	-	-	-
Mov Cap-1 Maneuver	232	*888	1056	-	451	-	-
Mov Cap-2 Maneuver	232	-	-	-	-	-	-
Stage 1	582	-	-	-	-	-	-
Stage 2	594	-	-	-	-	-	-

Approach	EB	NB	SB
HCM Ctrl Dly, s/v	18.19	0.54	0.51
HCM LOS	C		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	119	-	332	-	-
HCM Lane V/C Ratio	0.026	-	0.179	-	-
HCM Ctrl Dly (s/v)	8.5	0.3	18.2	0.4	-
HCM Lane LOS	A	A	C	A	-
HCM 95th %tile Q(veh)	0.1	-	0.6	-	-

Notes	
~: Volume exceeds capacity	\$. Delay exceeds 300s
+: Computation Not Defined	*: All major volume in platoon

Capacity Analysis Summary Sheets
Year 2031 No-Build Weekday Morning Peak Hour

Lanes, Volumes, Timings

3: Finley Road & Roosevelt Road

05/20/2025

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		 			 		 	 		 	 	
Traffic Volume (vph)	118	1106	322	147	1041	109	243	224	163	144	405	108
Future Volume (vph)	118	1106	322	147	1041	109	243	224	163	144	405	108
Ideal Flow (vphpl)	1900	2000	1900	1900	2000	1900	1900	2000	1900	1900	1900	1900
Storage Length (ft)	210		420	470		100	275		160	245		0
Storage Lanes	1		1	1		1	2		1	2		0
Taper Length (ft)	135			95			195			250		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	0.97	0.95	1.00	0.97	0.95	0.95
Frt			0.850			0.850			0.850		0.968	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1719	3654	1568	1787	3619	1568	3303	3725	1583	3335	3370	0
Flt Permitted	0.142			0.092			0.950			0.950		
Satd. Flow (perm)	257	3654	1568	173	3619	1568	3303	3725	1583	3335	3370	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			30			30	
Link Distance (ft)		623			970			675			547	
Travel Time (s)		12.1			18.9			15.3			12.4	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	5%	4%	3%	1%	5%	3%	6%	2%	2%	5%	2%	10%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	130	1215	354	162	1144	120	267	246	179	158	564	0
Turn Type	pm+pt	NA	pm+ov	pm+pt	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	
Protected Phases	5	2	3	1	6	7	3	8	1	7	4	
Permitted Phases	2		2	6		6			8			
Detector Phase	5	2	3	1	6	7	3	8	1	7	4	
Switch Phase												
Minimum Initial (s)	3.0	15.0	3.0	3.0	15.0	3.0	3.0	8.0	3.0	3.0	8.0	
Minimum Split (s)	9.5	44.0	9.5	9.5	24.0	9.5	9.5	32.0	9.5	9.5	24.0	
Total Split (s)	13.0	63.0	25.0	20.0	70.0	25.0	25.0	32.0	20.0	25.0	32.0	
Total Split (%)	9.3%	45.0%	17.9%	14.3%	50.0%	17.9%	17.9%	22.9%	14.3%	17.9%	22.9%	
Yellow Time (s)	3.5	4.0	3.5	3.5	4.0	3.5	3.5	4.0	3.5	3.5	4.0	
All-Red Time (s)	0.0	2.0	1.0	0.0	2.0	1.0	1.0	2.0	0.0	1.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	3.5	6.0	4.5	3.5	6.0	4.5	4.5	6.0	3.5	4.5	6.0	
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Recall Mode	None	C-Min	None	None	C-Min	None	None	None	None	None	None	
Act Effct Green (s)	74.4	62.9	85.6	80.8	66.8	84.8	16.7	32.3	51.1	12.0	27.6	
Actuated g/C Ratio	0.53	0.45	0.61	0.58	0.48	0.61	0.12	0.23	0.37	0.09	0.20	
v/c Ratio	0.57	0.74	0.37	0.66	0.66	0.13	0.68	0.29	0.31	0.56	0.85	
Control Delay (s/veh)	24.6	36.4	15.5	32.2	30.9	12.5	67.9	45.3	32.8	68.7	67.2	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay (s/veh)	24.6	36.4	15.5	32.2	30.9	12.5	67.9	45.3	32.8	68.7	67.2	
LOS	C	D	B	C	C	B	E	D	C	E	E	
Approach Delay (s/veh)		31.1			29.5			50.8			67.5	
Approach LOS		C			C			D			E	
Queue Length 50th (ft)	53	491	163	67	427	47	122	95	112	72	257	
Queue Length 95th (ft)	87	606	228	140	508	73	165	140	175	108	#371	

AMNB Year 2031 NB Weekday Morning Peak Hour
sa

Synchro 12 Report

Lanes, Volumes, Timings

3: Finley Road & Roosevelt Road

05/20/2025

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)	543			890			595			467		
Turn Bay Length (ft)	210		420	470		100	275		160	245		
Base Capacity (vph)	236	1642	1001	291	1726	1044	483	858	619	488	667	
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.55	0.74	0.35	0.56	0.66	0.11	0.55	0.29	0.29	0.32	0.85	

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 133 (95%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 95

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.85

Intersection Signal Delay (s/veh): 39.4

Intersection LOS: D

Intersection Capacity Utilization 75.8%

ICU Level of Service D

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 3: Finley Road & Roosevelt Road

	Ø1		Ø2 (R)		Ø3		Ø4
20 s		63 s		25 s		32 s	
	Ø5		Ø6 (R)		Ø7		Ø8
13 s		70 s		25 s		32 s	

Intersection												
Int Delay, s/veh	0											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	7	1546	1	3	1383	6	0	0	0	0	0	0
Future Vol, veh/h	7	1546	1	3	1383	6	0	0	0	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	160	-	-	150	-	-	-	-	0	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	1	-	-	1	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	91	91	91	91	91	91	91	91	91	91	91	91
Heavy Vehicles, %	14	4	0	0	5	0	0	0	0	0	0	0
Mvmt Flow	8	1699	1	3	1520	7	0	0	0	0	0	0

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	1526	0	0	1700	0	0	2481	3248	850	2395	3245	763
Stage 1	-	-	-	-	-	-	1715	1715	-	1530	1530	-
Stage 2	-	-	-	-	-	-	766	1533	-	865	1715	-
Critical Hdwy	4.38	-	-	4.1	-	-	7.5	6.5	6.9	7.5	6.5	6.9
Critical Hdwy Stg 1	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Follow-up Hdwy	2.34	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	527	-	-	488	-	-	*69	1	*649	*127	1	*695
Stage 1	-	-	-	-	-	-	*196	228	-	*258	286	-
Stage 2	-	-	-	-	-	-	*655	284	-	*612	228	-
Platoon blocked, %	0	-	-	0	-	-	1	1	0	1	1	0
Mov Cap-1 Maneuver	527	-	-	488	-	-	*67	1	*649	*124	1	*695
Mov Cap-2 Maneuver	-	-	-	-	-	-	*156	113	-	*208	114	-
Stage 1	-	-	-	-	-	-	*193	225	-	*257	284	-
Stage 2	-	-	-	-	-	-	*651	282	-	*603	225	-

Approach	EB	WB	NB	SB
HCM Ctrl Dly, s/v	0.05	0.03	0	0
HCM LOS			A	A

Minor Lane/Major Mvmt	NBLn1	NBLn2	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	-	-	527	-	-	488	-	-	-
HCM Lane V/C Ratio	-	-	0.015	-	-	0.007	-	-	-
HCM Ctrl Dly (s/v)	0	0	11.9	-	-	12.4	-	-	0
HCM Lane LOS	A	A	B	-	-	B	-	-	A
HCM 95th %tile Q(veh)	-	-	0	-	-	0	-	-	-

Notes	
~: Volume exceeds capacity	\$. Delay exceeds 300s
+: Computation Not Defined	*: All major volume in platoon

Intersection

Int Delay, s/veh 0.8

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕		↕	↕	
Traffic Vol, veh/h	0	0	4	45	0	25	1	602	25	20	854	0
Future Vol, veh/h	0	0	4	45	0	25	1	602	25	20	854	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	120	-	-
Veh in Median Storage, #	-	0	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	97	97	97	97	97	97	97	97	97	97	97	97
Heavy Vehicles, %	0	0	50	3	0	4	0	2	0	5	2	0
Mvmt Flow	0	0	4	46	0	26	1	621	26	21	880	0

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	1234	1570	440	1117	1557	323	880	0	0	646	0	0
Stage 1	922	922	-	636	636	-	-	-	-	-	-	-
Stage 2	312	648	-	481	922	-	-	-	-	-	-	-
Critical Hdwy	7.5	6.5	7.9	7.56	6.5	6.98	4.1	-	-	4.2	-	-
Critical Hdwy Stg 1	6.5	5.5	-	6.56	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.5	5.5	-	6.56	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.8	3.53	4	3.34	2.2	-	-	2.25	-	-
Pot Cap-1 Maneuver	238	151	*744	*300	154	667	999	-	-	915	-	-
Stage 1	505	498	-	*430	475	-	-	-	-	-	-	-
Stage 2	679	469	-	*801	498	-	-	-	-	-	-	-
Platoon blocked, %	0	0	0	0	0	0	0	-	-	-	-	-
Mov Cap-1 Maneuver	224	147	*744	*291	151	667	999	-	-	915	-	-
Mov Cap-2 Maneuver	224	147	-	*364	288	-	-	-	-	-	-	-
Stage 1	493	487	-	*430	475	-	-	-	-	-	-	-
Stage 2	651	468	-	*778	487	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Ctrl Dly, s/v	9.87	14.93	0.02	0.21
HCM LOS	A	B		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	5	-	-	744	435	915	-	-
HCM Lane V/C Ratio	0.001	-	-	0.006	0.166	0.023	-	-
HCM Ctrl Dly (s/v)	8.6	0	-	9.9	14.9	9	-	-
HCM Lane LOS	A	A	-	A	B	A	-	-
HCM 95th %tile Q(veh)	0	-	-	0	0.6	0.1	-	-

Notes

~: Volume exceeds capacity \$: Delay exceeds 300s
 +: Computation Not Defined *: All major volume in platoon

HCM 7th TWSC
12: Finley Road & International Drive

06/12/2025

Intersection							
Int Delay, s/veh	1.1						
Movement	EBL	EBR	NBL	NBT	SBU	SBT	SBR
Lane Configurations	Y			↑↑		↑↑	
Traffic Vol, veh/h	35	40	10	581	12	878	12
Future Vol, veh/h	35	40	10	581	12	878	12
Conflicting Peds, #/hr	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	-	None
Storage Length	0	-	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	-	0	-
Grade, %	0	-	-	0	-	0	-
Peak Hour Factor	96	96	96	96	96	96	96
Heavy Vehicles, %	3	13	0	3	0	2	17
Mvmt Flow	36	42	10	605	13	915	13

Major/Minor	Minor2	Major1		Major2			
Conflicting Flow All	1269	464	927	0	605	-	0
Stage 1	946	-	-	-	-	-	-
Stage 2	323	-	-	-	-	-	-
Critical Hdwy	6.86	7.16	4.1	-	6.4	-	-
Critical Hdwy Stg 1	5.86	-	-	-	-	-	-
Critical Hdwy Stg 2	5.86	-	-	-	-	-	-
Follow-up Hdwy	3.53	3.43	2.2	-	2.5	-	-
Pot Cap-1 Maneuver	264	*809	972	-	601	-	-
Stage 1	534	-	-	-	-	-	-
Stage 2	703	-	-	-	-	-	-
Platoon blocked, %	0	0	0	-	-	-	-
Mov Cap-1 Maneuver	253	*809	972	-	601	-	-
Mov Cap-2 Maneuver	253	-	-	-	-	-	-
Stage 1	527	-	-	-	-	-	-
Stage 2	683	-	-	-	-	-	-

Approach	EB	NB	SB
HCM Ctrl Dly, s/v	16.18	0.26	0.44
HCM LOS	C		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	61	-	400	-	-
HCM Lane V/C Ratio	0.011	-	0.195	-	-
HCM Ctrl Dly (s/v)	8.7	0.1	16.2	0.3	-
HCM Lane LOS	A	A	C	A	-
HCM 95th %tile Q(veh)	0	-	0.7	-	-

Notes	
~: Volume exceeds capacity	\$. Delay exceeds 300s
+: Computation Not Defined	*: All major volume in platoon

Capacity Analysis Summary Sheets
Year 2031 No-Build Weekday Evening Peak Hour

Lanes, Volumes, Timings

3: Finley Road & Roosevelt Road

06/11/2025

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	103	1151	312	214	1181	107	433	351	238	204	382	122
Future Volume (vph)	103	1151	312	214	1181	107	433	351	238	204	382	122
Ideal Flow (vphpl)	1900	2000	1900	1900	2000	1900	1900	2000	1900	1900	1900	1900
Storage Length (ft)	210		420	470		100	275		160	245		0
Storage Lanes	1		1	1		1	2		1	2		0
Taper Length (ft)	135			95			195			250		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	0.97	0.95	1.00	0.97	0.95	0.95
Frt			0.850			0.850			0.850		0.964	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1787	3762	1599	1805	3762	1583	3433	3762	1599	3502	3446	0
Flt Permitted	0.114			0.093			0.950			0.950		
Satd. Flow (perm)	214	3762	1599	177	3762	1583	3433	3762	1599	3502	3446	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			30			30	
Link Distance (ft)		623			970			675			547	
Travel Time (s)		12.1			18.9			15.3			12.4	
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Heavy Vehicles (%)	1%	1%	1%	0%	1%	2%	2%	1%	1%	0%	1%	1%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	105	1174	318	218	1205	109	442	358	243	208	514	0
Turn Type	pm+pt	NA	pm+ov	pm+pt	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	
Protected Phases	5	2	3	1	6	7	3	8	1	7	4	
Permitted Phases	2		2	6		6			8			
Detector Phase	5	2	3	1	6	7	3	8	1	7	4	
Switch Phase												
Minimum Initial (s)	3.0	15.0	3.0	3.0	15.0	3.0	3.0	8.0	3.0	3.0	8.0	
Minimum Split (s)	9.5	44.0	9.5	9.5	24.0	9.5	9.5	32.0	9.5	9.5	24.0	
Total Split (s)	17.0	68.0	34.0	20.0	71.0	22.0	34.0	40.0	20.0	22.0	28.0	
Total Split (%)	11.3%	45.3%	22.7%	13.3%	47.3%	14.7%	22.7%	26.7%	13.3%	14.7%	18.7%	
Yellow Time (s)	3.5	4.0	3.5	3.5	4.0	3.5	3.5	4.0	3.5	3.5	4.0	
All-Red Time (s)	0.0	2.0	1.0	0.0	2.0	1.0	1.0	2.0	0.0	1.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	-1.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	3.5	6.0	4.5	2.5	6.0	4.5	4.5	6.0	3.5	4.5	6.0	
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Recall Mode	None	C-Min	None	None	C-Min	None	None	None	None	None	None	
Act Effct Green (s)	76.1	63.9	94.8	85.9	69.2	89.3	24.9	37.0	58.0	14.1	26.2	
Actuated g/C Ratio	0.51	0.43	0.63	0.57	0.46	0.60	0.17	0.25	0.39	0.09	0.17	
v/c Ratio	0.50	0.73	0.31	0.80	0.69	0.12	0.78	0.39	0.39	0.63	0.85	
Control Delay (s/veh)	24.3	39.7	13.6	48.4	35.1	13.6	69.5	48.9	35.7	74.1	74.2	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay (s/veh)	24.3	39.7	13.6	48.4	35.1	13.6	69.5	48.9	35.7	74.1	74.2	
LOS	C	D	B	D	D	B	E	D	D	E	E	
Approach Delay (s/veh)		33.4			35.5			54.6			74.2	
Approach LOS		C			D			D			E	
Queue Length 50th (ft)	46	511	140	123	485	45	215	154	168	102	260	
Queue Length 95th (ft)	77	599	182	#236	594	75	268	210	255	144	#399	

Lanes, Volumes, Timings

3: Finley Road & Roosevelt Road

06/11/2025

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)	543			890			595			467		
Turn Bay Length (ft)	210		420	470		100	275		160	245		
Base Capacity (vph)	255	1603	1060	291	1735	978	675	928	634	408	602	
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.41	0.73	0.30	0.75	0.69	0.11	0.65	0.39	0.38	0.51	0.85	

Intersection Summary

Area Type: Other

Cycle Length: 150

Actuated Cycle Length: 150

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 95

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.85

Intersection Signal Delay (s/veh): 44.6

Intersection LOS: D

Intersection Capacity Utilization 86.0%

ICU Level of Service E

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 3: Finley Road & Roosevelt Road

	Ø1		Ø2 (R)		Ø3		Ø4
20 s		68 s		34 s		28 s	
	Ø5		Ø6 (R)		Ø7		Ø8
17 s		71 s		22 s		40 s	

Intersection												
Int Delay, s/veh	0.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↰	↰↱		↰	↰↱			↰	↰		↰↱	
Traffic Vol, veh/h	9	1560	4	4	1729	3	0	0	5	1	0	1
Future Vol, veh/h	9	1560	4	4	1729	3	0	0	5	1	0	1
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	160	-	-	150	-	-	-	-	0	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	1	-	-	1	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	97	97	97	97	97	97	97	97	97	97	97	97
Heavy Vehicles, %	0	1	0	0	1	0	0	0	0	0	0	0
Mvmt Flow	9	1608	4	4	1782	3	0	0	5	1	0	1

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	1786	0	0	1612	0	0	2528	3423	806	2615	3423	893
Stage 1	-	-	-	-	-	-	1629	1629	-	1792	1792	-
Stage 2	-	-	-	-	-	-	899	1794	-	823	1631	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.5	6.5	6.9	7.5	6.5	6.9
Critical Hdwy Stg 1	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	465	-	-	565	-	-	*37	0	*637	*21	0	*594
Stage 1	-	-	-	-	-	-	*250	273	-	*195	222	-
Stage 2	-	-	-	-	-	-	*560	221	-	*600	272	-
Platoon blocked, %	0	-	-	0	-	-	1	1	0	1	1	0
Mov Cap-1 Maneuver	465	-	-	565	-	-	*36	0	*637	*20	0	*594
Mov Cap-2 Maneuver	-	-	-	-	-	-	*168	108	-	*139	109	-
Stage 1	-	-	-	-	-	-	*245	268	-	*193	220	-
Stage 2	-	-	-	-	-	-	*555	220	-	*583	267	-

Approach	EB	WB	NB	SB
HCM Ctrl Dly, s/v	0.07	0.03	10.7	21.12
HCM LOS			B	C

Minor Lane/Major Mvmt	NBLn1	NBLn2	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	-	637	465	-	-	565	-	-	225
HCM Lane V/C Ratio	-	0.008	0.02	-	-	0.007	-	-	0.009
HCM Ctrl Dly (s/v)	0	10.7	12.9	-	-	11.4	-	-	21.1
HCM Lane LOS	A	B	B	-	-	B	-	-	C
HCM 95th %tile Q(veh)	-	0	0.1	-	-	0	-	-	0

Notes	
~: Volume exceeds capacity	\$. Delay exceeds 300s
+: Computation Not Defined	*: All major volume in platoon

Intersection

Int Delay, s/veh 1.1

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕		↕	↕	
Traffic Vol, veh/h	0	0	1	48	2	24	2	1004	63	26	882	0
Future Vol, veh/h	0	0	1	48	2	24	2	1004	63	26	882	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	120	-	-
Veh in Median Storage, #	-	0	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	97	97	97	97	97	97	97	97	97	97	97	97
Heavy Vehicles, %	0	0	0	0	0	4	0	1	0	4	0	0
Mvmt Flow	0	0	1	49	2	25	2	1035	65	27	909	0

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	1486	2067	455	1580	2035	550	909	0	0	1100	0	0
Stage 1	963	963	-	1072	1072	-	-	-	-	-	-	-
Stage 2	523	1104	-	508	963	-	-	-	-	-	-	-
Critical Hdwy	7.5	6.5	6.9	7.5	6.5	6.98	4.1	-	-	4.18	-	-
Critical Hdwy Stg 1	6.5	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.5	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.34	2.2	-	-	2.24	-	-
Pot Cap-1 Maneuver	144	63	*843	*118	67	474	987	-	-	619	-	-
Stage 1	489	485	-	*239	300	-	-	-	-	-	-	-
Stage 2	511	289	-	*795	485	-	-	-	-	-	-	-
Platoon blocked, %	0	0	0	0	0	0	-	-	-	-	-	-
Mov Cap-1 Maneuver	129	60	*843	*112	64	474	987	-	-	619	-	-
Mov Cap-2 Maneuver	129	60	-	*199	193	-	-	-	-	-	-	-
Stage 1	467	464	-	*238	299	-	-	-	-	-	-	-
Stage 2	479	288	-	*760	464	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Ctrl Dly, s/v	9.27	26.25	0.04	0.32
HCM LOS	A	D		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	6	-	-	843	245	619	-	-
HCM Lane V/C Ratio	0.002	-	-	0.001	0.312	0.043	-	-
HCM Ctrl Dly (s/v)	8.7	0	-	9.3	26.2	11.1	-	-
HCM Lane LOS	A	A	-	A	D	B	-	-
HCM 95th %tile Q(veh)	0	-	-	0	1.3	0.1	-	-

Notes

~: Volume exceeds capacity \$: Delay exceeds 300s
 +: Computation Not Defined *: All major volume in platoon

06/02/2025

Capacity Analysis Summary Sheets
Year 2031 No-Build Saturday Midday Peak Hour

Lanes, Volumes, Timings

3: Finley Road & Roosevelt Road

06/02/2025

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	82	1057	267	205	995	119	303	267	205	127	258	85
Future Volume (vph)	82	1057	267	205	995	119	303	267	205	127	258	85
Ideal Flow (vphpl)	1900	2000	1900	1900	2000	1900	1900	2000	1900	1900	1900	1900
Storage Length (ft)	210		420	470		100	275		160	245		0
Storage Lanes	1		1	1		1	2		1	2		0
Taper Length (ft)	135			95			195			250		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	0.97	0.95	1.00	0.97	0.95	0.95
Frt			0.850			0.850			0.850		0.963	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1752	3762	1615	1787	3762	1599	3502	3762	1599	3502	3468	0
Flt Permitted	0.211			0.147			0.950			0.950		
Satd. Flow (perm)	389	3762	1615	277	3762	1599	3502	3762	1599	3502	3468	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			30			30	
Link Distance (ft)		623			970			675			547	
Travel Time (s)		12.1			18.9			15.3			12.4	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles (%)	3%	1%	0%	1%	1%	1%	0%	1%	1%	0%	0%	1%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	85	1090	275	211	1026	123	312	275	211	131	354	0
Turn Type	pm+pt	NA	pm+ov	pm+pt	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	
Protected Phases	5	2	3	1	6	7	3	8	1	7	4	
Permitted Phases	2		2	6		6			8			
Detector Phase	5	2	3	1	6	7	3	8	1	7	4	
Switch Phase												
Minimum Initial (s)	3.0	15.0	3.0	3.0	15.0	3.0	3.0	8.0	3.0	3.0	8.0	
Minimum Split (s)	9.5	44.0	9.5	9.5	24.0	9.5	9.5	32.0	9.5	9.5	24.0	
Total Split (s)	13.0	60.0	22.0	23.0	70.0	22.0	22.0	25.0	23.0	22.0	25.0	
Total Split (%)	10.0%	46.2%	16.9%	17.7%	53.8%	16.9%	16.9%	19.2%	17.7%	16.9%	19.2%	
Yellow Time (s)	3.5	4.0	3.5	3.5	4.0	3.5	3.5	4.0	3.5	3.5	4.0	
All-Red Time (s)	0.0	2.0	1.0	0.0	2.0	1.0	1.0	2.0	0.0	1.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	3.5	6.0	4.5	3.5	6.0	4.5	4.5	6.0	3.5	4.5	6.0	
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Recall Mode	None	C-Min	None	None	C-Min	None	None	None	None	None	None	
Act Effct Green (s)	72.9	62.5	84.4	81.5	67.9	84.1	15.9	24.0	43.3	10.2	18.3	
Actuated g/C Ratio	0.56	0.48	0.65	0.63	0.52	0.65	0.12	0.18	0.33	0.08	0.14	
v/c Ratio	0.28	0.60	0.26	0.65	0.52	0.12	0.73	0.40	0.40	0.48	0.72	
Control Delay (s/veh)	13.1	27.6	11.3	21.4	22.2	9.5	65.5	48.4	34.9	62.8	62.6	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay (s/veh)	13.1	27.6	11.3	21.4	22.2	9.5	65.5	48.4	34.9	62.8	62.6	
LOS	B	C	B	C	C	A	E	D	C	E	E	
Approach Delay (s/veh)		23.7			20.9			51.5			62.6	
Approach LOS		C			C			D			E	
Queue Length 50th (ft)	28	354	93	74	301	39	131	105	133	55	150	
Queue Length 95th (ft)	50	472	159	126	374	64	181	153	195	87	206	

Lanes, Volumes, Timings

3: Finley Road & Roosevelt Road

06/02/2025

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)	543			890			595			467		
Turn Bay Length (ft)	210		420	470		100	275		160	245		
Base Capacity (vph)	322	1808	1068	400	1965	1124	471	694	608	471	510	
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.26	0.60	0.26	0.53	0.52	0.11	0.66	0.40	0.35	0.28	0.69	

Intersection Summary

Area Type: Other

Cycle Length: 130

Actuated Cycle Length: 130

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 95

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.73

Intersection Signal Delay (s/veh): 32.8

Intersection LOS: C

Intersection Capacity Utilization 74.7%

ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 3: Finley Road & Roosevelt Road

 Ø1	 Ø2 (R)	 Ø3	 Ø4
23 s	60 s	22 s	25 s
 Ø5	 Ø6 (R)	 Ø7	 Ø8
13 s	70 s	22 s	25 s

Intersection

Int Delay, s/veh 0.2

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	29	1399	3	8	1373	2	0	0	6	1	0	0
Future Vol, veh/h	29	1399	3	8	1373	2	0	0	6	1	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	160	-	-	150	-	-	-	-	0	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	1	-	-	1	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	96	96	96	96	96	96	96	96	96	96	96	96
Heavy Vehicles, %	0	1	0	0	1	0	0	0	0	0	0	0
Mvmt Flow	30	1457	3	8	1430	2	0	0	6	1	0	0

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	1432	0	0	1460	0	0	2251	2968	730	2237	2969	716
Stage 1	-	-	-	-	-	-	1519	1519	-	1448	1448	-
Stage 2	-	-	-	-	-	-	732	1449	-	789	1521	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.5	6.5	6.9	7.5	6.5	6.9
Critical Hdwy Stg 1	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	660	-	-	638	-	-	*23	14	*695	*24	14	*697
Stage 1	-	-	-	-	-	-	*264	291	-	*306	325	-
Stage 2	-	-	-	-	-	-	*657	324	-	*655	290	-
Platoon blocked, %	0	-	-	0	-	-			0			0
Mov Cap-1 Maneuver	660	-	-	638	-	-	*22	14	*695	*22	14	*697
Mov Cap-2 Maneuver	-	-	-	-	-	-	*172	136	-	*193	139	-
Stage 1	-	-	-	-	-	-	*252	277	-	*302	321	-
Stage 2	-	-	-	-	-	-	*648	320	-	*619	277	-

Approach	EB	WB	NB	SB
HCM Ctrl Dly, s/v	0.22	0.06	10.23	23.72
HCM LOS			B	C

Minor Lane/Major Mvmt	NBLn1	NBLn2	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	-	695	660	-	-	638	-	-	193
HCM Lane V/C Ratio	-	0.009	0.046	-	-	0.013	-	-	0.005
HCM Ctrl Dly (s/v)	0	10.2	10.7	-	-	10.7	-	-	23.7
HCM Lane LOS	A	B	B	-	-	B	-	-	C
HCM 95th %tile Q(veh)	-	0	0.1	-	-	0	-	-	0

Notes

~: Volume exceeds capacity \$: Delay exceeds 300s
 +: Computation Not Defined *: All major volume in platoon

Intersection

Int Delay, s/veh 1.2

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕		↕	↕	
Traffic Vol, veh/h	0	0	2	49	1	31	2	755	36	29	701	0
Future Vol, veh/h	0	0	2	49	1	31	2	755	36	29	701	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	120	-	-
Veh in Median Storage, #	-	0	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	0	0	0	0	0	3	0	1	0	0	0	0
Mvmt Flow	0	0	2	54	1	34	2	839	40	32	779	0

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	1268	1727	389	1317	1707	439	779	0	0	879	0	0
Stage 1	843	843	-	863	863	-	-	-	-	-	-	-
Stage 2	424	883	-	454	843	-	-	-	-	-	-	-
Critical Hdwy	7.5	6.5	6.9	7.5	6.5	6.96	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.5	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.5	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.33	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	202	110	*888	*182	114	563	1061	-	-	777	-	-
Stage 1	525	518	-	*320	374	-	-	-	-	-	-	-
Stage 2	583	366	-	*837	518	-	-	-	-	-	-	-
Platoon blocked, %	0	0	0	0	0	0	-	-	-	-	-	-
Mov Cap-1 Maneuver	180	105	*888	*174	109	563	1061	-	-	777	-	-
Mov Cap-2 Maneuver	180	105	-	*266	242	-	-	-	-	-	-	-
Stage 1	503	496	-	*319	373	-	-	-	-	-	-	-
Stage 2	544	365	-	*800	496	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Ctrl Dly, s/v	9.06	19.77	0.04	0.39
HCM LOS	A	C		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	9	-	-	888	333	777	-	-
HCM Lane V/C Ratio	0.002	-	-	0.003	0.27	0.041	-	-
HCM Ctrl Dly (s/v)	8.4	0	-	9.1	19.8	9.8	-	-
HCM Lane LOS	A	A	-	A	C	A	-	-
HCM 95th %tile Q(veh)	0	-	-	0	1.1	0.1	-	-

Notes

~: Volume exceeds capacity \$: Delay exceeds 300s
 +: Computation Not Defined *: All major volume in platoon

Intersection							
Int Delay, s/veh	1.2						
Movement	EBL	EBR	NBL	NBT	SBU	SBT	SBR
Lane Configurations							
Traffic Vol, veh/h	32	22	25	752	9	725	18
Future Vol, veh/h	32	22	25	752	9	725	18
Conflicting Peds, #/hr	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	-	None
Storage Length	0	-	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	-	0	-
Grade, %	0	-	-	0	-	0	-
Peak Hour Factor	91	91	91	91	91	92	91
Heavy Vehicles, %	0	0	0	1	0	0	0
Mvmt Flow	35	24	27	826	10	788	20

Major/Minor	Minor2	Major1		Major2			
Conflicting Flow All	1286	404	808	0	826	-	0
Stage 1	818	-	-	-	-	-	-
Stage 2	468	-	-	-	-	-	-
Critical Hdwy	6.8	6.9	4.1	-	6.4	-	-
Critical Hdwy Stg 1	5.8	-	-	-	-	-	-
Critical Hdwy Stg 2	5.8	-	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	2.5	-	-
Pot Cap-1 Maneuver	232	*888	1030	-	436	-	-
Stage 1	582	-	-	-	-	-	-
Stage 2	602	-	-	-	-	-	-
Platoon blocked, %	0	0	0	-	-	-	-
Mov Cap-1 Maneuver	217	*888	1030	-	436	-	-
Mov Cap-2 Maneuver	217	-	-	-	-	-	-
Stage 1	562	-	-	-	-	-	-
Stage 2	585	-	-	-	-	-	-

Approach	EB	NB	SB
HCM Ctrl Dly, s/v	19.15	0.55	0.53
HCM LOS	C		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	116	-	313	-	-
HCM Lane V/C Ratio	0.027	-	0.189	-	-
HCM Ctrl Dly (s/v)	8.6	0.3	19.1	0.4	-
HCM Lane LOS	A	A	C	A	-
HCM 95th %tile Q(veh)	0.1	-	0.7	-	-

Notes	
~: Volume exceeds capacity	\$. Delay exceeds 300s
+: Computation Not Defined	*: All major volume in platoon

Capacity Analysis Summary Sheets
Year 2031 Total Projected Weekday Morning Peak
Hour

Lanes, Volumes, Timings
3: Finley Road & Roosevelt Road

06/11/2025

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	127	1124	322	162	1060	109	248	234	175	144	422	108
Future Volume (vph)	127	1124	322	162	1060	109	248	234	175	144	422	108
Ideal Flow (vphpl)	1900	2000	1900	1900	2000	1900	1900	2000	1900	1900	1900	1900
Storage Length (ft)	210		420	470		100	275		160	245		0
Storage Lanes	1		1	1		1	2		1	2		0
Taper Length (ft)	135			95			195			250		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	0.97	0.95	1.00	0.97	0.95	0.95
Frt			0.850			0.850			0.850		0.969	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1719	3654	1568	1787	3619	1568	3303	3725	1583	3335	3375	0
Flt Permitted	0.133			0.081			0.950			0.950		
Satd. Flow (perm)	241	3654	1568	152	3619	1568	3303	3725	1583	3335	3375	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			30			30	
Link Distance (ft)		623			970			177			547	
Travel Time (s)		12.1			18.9			4.0			12.4	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	5%	4%	3%	1%	5%	3%	6%	2%	2%	5%	2%	10%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	140	1235	354	178	1165	120	273	257	192	158	583	0
Turn Type	pm+pt	NA	pm+ov	pm+pt	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	
Protected Phases	5	2	3	1	6	7	3	8	1	7	4	
Permitted Phases	2		2	6		6			8			
Detector Phase	5	2	3	1	6	7	3	8	1	7	4	
Switch Phase												
Minimum Initial (s)	3.0	15.0	3.0	3.0	15.0	3.0	3.0	8.0	3.0	3.0	8.0	
Minimum Split (s)	9.5	44.0	9.5	9.5	24.0	9.5	9.5	32.0	9.5	9.5	24.0	
Total Split (s)	13.0	63.0	25.0	20.0	70.0	25.0	25.0	32.0	20.0	25.0	32.0	
Total Split (%)	9.3%	45.0%	17.9%	14.3%	50.0%	17.9%	17.9%	22.9%	14.3%	17.9%	22.9%	
Yellow Time (s)	3.5	4.0	3.5	3.5	4.0	3.5	3.5	4.0	3.5	3.5	4.0	
All-Red Time (s)	0.0	2.0	1.0	0.0	2.0	1.0	1.0	2.0	0.0	1.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	3.5	6.0	4.5	3.5	6.0	4.5	4.5	6.0	3.5	4.5	6.0	
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Recall Mode	None	C-Min	None	None	C-Min	None	None	None	None	None	None	
Act Effct Green (s)	73.3	61.7	84.5	80.4	66.1	84.0	16.8	32.9	52.4	12.0	28.0	
Actuated g/C Ratio	0.52	0.44	0.60	0.57	0.47	0.60	0.12	0.24	0.37	0.09	0.20	
v/c Ratio	0.63	0.77	0.37	0.73	0.68	0.13	0.69	0.29	0.32	0.56	0.86	
Control Delay (s/veh)	29.1	38.1	16.0	42.1	31.8	12.7	68.0	45.1	32.4	68.7	68.2	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay (s/veh)	29.1	38.1	16.0	42.1	31.8	12.7	68.0	45.1	32.4	68.7	68.2	
LOS	C	D	B	D	C	B	E	D	C	E	E	
Approach Delay (s/veh)		32.8			31.5			50.4			68.3	
Approach LOS		C			C			D			E	
Queue Length 50th (ft)	58	511	166	88	439	47	124	100	120	72	268	
Queue Length 95th (ft)	98	620	228	171	522	73	169	146	187	108	#390	

Lanes, Volumes, Timings

3: Finley Road & Roosevelt Road

06/11/2025

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)	543			890			97			467		
Turn Bay Length (ft)	210		420	470		100	275		160	245		
Base Capacity (vph)	227	1609	987	281	1708	1036	483	874	625	488	674	
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.62	0.77	0.36	0.63	0.68	0.12	0.57	0.29	0.31	0.32	0.86	

Intersection Summary

Area Type: Other

Cycle Length: 140

Actuated Cycle Length: 140

Offset: 133 (95%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 95

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.86

Intersection Signal Delay (s/veh): 40.8

Intersection LOS: D

Intersection Capacity Utilization 77.8%

ICU Level of Service D

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 3: Finley Road & Roosevelt Road

	Ø1		Ø2 (R)		Ø3		Ø4
20 s		63 s		25 s		32 s	
	Ø5		Ø6 (R)		Ø7		Ø8
13 s		70 s		25 s		32 s	

Intersection												
Int Delay, s/veh	0.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↰	↰↱		↰	↰↱			↱	↱		↱↰	
Traffic Vol, veh/h	7	1531	49	34	1376	6	33	0	42	0	0	0
Future Vol, veh/h	7	1531	49	34	1376	6	33	0	42	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	160	-	-	150	-	-	-	-	0	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	1	-	-	1	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	91	91	91	91	91	91	91	91	91	91	91	91
Heavy Vehicles, %	14	4	0	0	5	0	0	0	0	0	0	0
Mvmt Flow	8	1682	54	37	1512	7	36	0	46	0	0	0

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	1519	0	0	1736	0	0	2555	3318	868	2447	3342	759
Stage 1	-	-	-	-	-	-	1725	1725	-	1590	1590	-
Stage 2	-	-	-	-	-	-	831	1593	-	857	1752	-
Critical Hdwy	4.38	-	-	4.1	-	-	7.5	6.5	6.9	7.5	6.5	6.9
Critical Hdwy Stg 1	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Follow-up Hdwy	2.34	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	520	-	-	455	-	-	*~ 32	1	*664	*65	0	*710
Stage 1	-	-	-	-	-	-	*183	218	-	*217	251	-
Stage 2	-	-	-	-	-	-	*669	250	-	*627	208	-
Platoon blocked, %	0	-	-	0	-	-	1	1	0	1	1	0
Mov Cap-1 Maneuver	520	-	-	455	-	-	*~ 29	0	*664	*54	0	*710
Mov Cap-2 Maneuver	-	-	-	-	-	-	*136	100	-	*151	89	-
Stage 1	-	-	-	-	-	-	*181	215	-	*199	231	-
Stage 2	-	-	-	-	-	-	*614	230	-	*574	205	-

Approach	EB	WB	NB	SB
HCM Ctrl Dly, s/v	0.05	0.33	24	0
HCM LOS			C	A

Minor Lane/Major Mvmt	NBLn1	NBLn2	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	136	664	520	-	-	455	-	-	-
HCM Lane V/C Ratio	0.266	0.069	0.015	-	-	0.082	-	-	-
HCM Ctrl Dly (s/v)	40.8	10.8	12	-	-	13.6	-	-	0
HCM Lane LOS	E	B	B	-	-	B	-	-	A
HCM 95th %tile Q(veh)	1	0.2	0	-	-	0.3	-	-	-

Notes	
~: Volume exceeds capacity	\$. Delay exceeds 300s
+: Computation Not Defined	*: All major volume in platoon

Intersection

Int Delay, s/veh 1.2

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	27	0	13	45	0	25	23	602	25	20	862	8
Future Vol, veh/h	27	0	13	45	0	25	23	602	25	20	862	8
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	0	-	-	-	-	-	120	-	-	120	-	-
Veh in Median Storage, #	-	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	97	97	97	97	97	97	97	97	97	97	97	97
Heavy Vehicles, %	0	0	50	3	0	4	0	2	0	5	2	0
Mvmt Flow	28	0	13	46	0	26	24	621	26	21	889	8

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	1292	1628	448	1166	1619	323	897	0	0	646	0	0
Stage 1	934	934	-	681	681	-	-	-	-	-	-	-
Stage 2	358	694	-	486	938	-	-	-	-	-	-	-
Critical Hdwy	7.5	6.5	7.9	7.56	6.5	6.98	4.1	-	-	4.2	-	-
Critical Hdwy Stg 1	6.5	5.5	-	6.56	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.5	5.5	-	6.56	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.8	3.53	4	3.34	2.2	-	-	2.25	-	-
Pot Cap-1 Maneuver	221	140	*730	*285	142	667	1004	-	-	915	-	-
Stage 1	518	506	-	*404	453	-	-	-	-	-	-	-
Stage 2	638	447	-	*786	503	-	-	-	-	-	-	-
Platoon blocked, %	0	0	0	0	0	0	-	-	-	-	-	-
Mov Cap-1 Maneuver	203	134	*730	*267	136	667	1004	-	-	915	-	-
Mov Cap-2 Maneuver	338	267	-	*335	269	-	-	-	-	-	-	-
Stage 1	506	494	-	*395	443	-	-	-	-	-	-	-
Stage 2	599	437	-	*754	491	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Ctrl Dly, s/v	14.46		15.73		0.31		0.2	
HCM LOS	B		C					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1004	-	-	338	730	407	915	-	-
HCM Lane V/C Ratio	0.024	-	-	0.082	0.018	0.177	0.023	-	-
HCM Ctrl Dly (s/v)	8.7	-	-	16.6	10	15.7	9	-	-
HCM Lane LOS	A	-	-	C	B	C	A	-	-
HCM 95th %tile Q(veh)	0.1	-	-	0.3	0.1	0.6	0.1	-	-

Notes

~: Volume exceeds capacity \$: Delay exceeds 300s
 +: Computation Not Defined *: All major volume in platoon

Intersection							
Int Delay, s/veh	1.1						
Movement	EBL	EBR	NBL	NBT	SBU	SBT	SBR
Lane Configurations							
Traffic Vol, veh/h	35	40	10	603	12	895	12
Future Vol, veh/h	35	40	10	603	12	895	12
Conflicting Peds, #/hr	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	-	None
Storage Length	0	-	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	-	0	-
Grade, %	0	-	-	0	-	0	-
Peak Hour Factor	96	96	96	96	96	96	96
Heavy Vehicles, %	3	13	0	3	0	2	17
Mvmt Flow	36	42	10	628	13	932	13

Major/Minor	Minor2	Major1		Major2			
Conflicting Flow All	1298	472	945	0	628	-	0
Stage 1	964	-	-	-	-	-	-
Stage 2	335	-	-	-	-	-	-
Critical Hdwy	6.86	7.16	4.1	-	6.4	-	-
Critical Hdwy Stg 1	5.86	-	-	-	-	-	-
Critical Hdwy Stg 2	5.86	-	-	-	-	-	-
Follow-up Hdwy	3.53	3.43	2.2	-	2.5	-	-
Pot Cap-1 Maneuver	250	*809	953	-	582	-	-
Stage 1	520	-	-	-	-	-	-
Stage 2	694	-	-	-	-	-	-
Platoon blocked, %	0	0	0	-	-	-	-
Mov Cap-1 Maneuver	239	*809	953	-	582	-	-
Mov Cap-2 Maneuver	239	-	-	-	-	-	-
Stage 1	513	-	-	-	-	-	-
Stage 2	673	-	-	-	-	-	-

Approach	EB	NB	SB
HCM Ctrl Dly, s/v	16.79	0.26	0.46
HCM LOS	C		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	59	-	383	-	-
HCM Lane V/C Ratio	0.011	-	0.204	-	-
HCM Ctrl Dly (s/v)	8.8	0.1	16.8	0.3	-
HCM Lane LOS	A	A	C	A	-
HCM 95th %tile Q(veh)	0	-	0.8	-	-

Notes	
~: Volume exceeds capacity	\$. Delay exceeds 300s
+: Computation Not Defined	*: All major volume in platoon

Intersection						
Int Delay, s/veh	0.1					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		↗		↑↑	↑↑	
Traffic Vol, veh/h	0	18	0	654	877	29
Future Vol, veh/h	0	18	0	654	877	29
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	0	0	0	2	2	0
Mvmt Flow	0	19	0	688	923	31

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	-	477	-	0	-	0
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.9	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.3	-	-	-	-
Pot Cap-1 Maneuver	0	540	0	-	-	-
Stage 1	0	-	0	-	-	-
Stage 2	0	-	0	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	-	540	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	EB	NB	SB
HCM Ctrl Dly, s/v	11.91	0	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	-	540	-	-
HCM Lane V/C Ratio	-	0.035	-	-
HCM Ctrl Dly (s/v)	-	11.9	-	-
HCM Lane LOS	-	B	-	-
HCM 95th %tile Q(veh)	-	0.1	-	-

Intersection						
Int Delay, s/veh	0					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		↗		↑↑	↑↑	
Traffic Vol, veh/h	0	4	0	654	886	9
Future Vol, veh/h	0	4	0	654	886	9
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	0	0	0	2	2	0
Mvmt Flow	0	4	0	688	933	9

Major/Minor	Minor2	Major1	Major2
Conflicting Flow All	- 471	- 0	- 0
Stage 1	- -	- -	- -
Stage 2	- -	- -	- -
Critical Hdwy	- 6.9	- -	- -
Critical Hdwy Stg 1	- -	- -	- -
Critical Hdwy Stg 2	- -	- -	- -
Follow-up Hdwy	- 3.3	- -	- -
Pot Cap-1 Maneuver	0 545	0 -	- -
Stage 1	0 -	0 -	- -
Stage 2	0 -	0 -	- -
Platoon blocked, %		- -	- -
Mov Cap-1 Maneuver	- 545	- -	- -
Mov Cap-2 Maneuver	- -	- -	- -
Stage 1	- -	- -	- -
Stage 2	- -	- -	- -

Approach	EB	NB	SB
HCM Ctrl Dly, s/v	11.66	0	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBT EBLn1	SBT	SBR
Capacity (veh/h)	- 545	- -	- -
HCM Lane V/C Ratio	- 0.008	- -	- -
HCM Ctrl Dly (s/v)	- 11.7	- -	- -
HCM Lane LOS	- B	- -	- -
HCM 95th %tile Q(veh)	- 0	- -	- -

Capacity Analysis Summary Sheets
Year 2031 Total Projected Weekday Evening Peak Hour

Lanes, Volumes, Timings

3: Finley Road & Roosevelt Road

06/12/2025

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	122	1207	312	296	1288	107	449	373	277	204	463	122
Future Volume (vph)	122	1207	312	296	1288	107	449	373	277	204	463	122
Ideal Flow (vphpl)	1900	2000	1900	1900	2000	1900	1900	2000	1900	1900	1900	1900
Storage Length (ft)	210		420	470		100	275		160	245		0
Storage Lanes	1		1	1		1	2		1	2		0
Taper Length (ft)	135			95			195			250		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	0.97	0.95	1.00	0.97	0.95	0.95
Frt			0.850			0.850			0.850		0.969	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1787	3762	1599	1805	3762	1583	3467	3762	1599	3502	3463	0
Flt Permitted	0.074			0.071			0.950			0.950		
Satd. Flow (perm)	139	3762	1599	135	3762	1583	3467	3762	1599	3502	3463	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			30			30	
Link Distance (ft)		623			2310			177			547	
Travel Time (s)		12.1			45.0			4.0			12.4	
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Heavy Vehicles (%)	1%	1%	1%	0%	1%	2%	1%	1%	1%	0%	1%	1%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	124	1232	318	302	1314	109	458	381	283	208	596	0
Turn Type	pm+pt	NA	pm+ov	pm+pt	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	
Protected Phases	5	2	3	1	6	7	3	8	1	7	4	
Permitted Phases	2		2	6		6			8			
Detector Phase	5	2	3	1	6	7	3	8	1	7	4	
Switch Phase												
Minimum Initial (s)	3.0	15.0	3.0	3.0	15.0	3.0	3.0	8.0	3.0	3.0	8.0	
Minimum Split (s)	9.5	44.0	9.5	9.5	24.0	9.5	9.5	32.0	9.5	9.5	24.0	
Total Split (s)	17.0	68.0	34.0	20.0	71.0	22.0	34.0	40.0	20.0	22.0	28.0	
Total Split (%)	11.3%	45.3%	22.7%	13.3%	47.3%	14.7%	22.7%	26.7%	13.3%	14.7%	18.7%	
Yellow Time (s)	3.5	4.0	3.5	3.5	4.0	3.5	3.5	4.0	3.5	3.5	4.0	
All-Red Time (s)	0.0	2.0	1.0	0.0	2.0	1.0	1.0	2.0	0.0	1.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	-2.0	0.0	0.0	0.0	0.0	0.0	0.0	-1.8	
Total Lost Time (s)	3.5	6.0	4.5	1.5	6.0	4.5	4.5	6.0	3.5	4.5	4.2	
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Recall Mode	None	C-Min	None	None	C-Min	None	None	None	None	None	None	
Act Effct Green (s)	75.5	62.0	93.2	86.5	67.5	87.6	25.2	37.4	59.9	14.1	28.1	
Actuated g/C Ratio	0.50	0.41	0.62	0.58	0.45	0.58	0.17	0.25	0.40	0.09	0.19	
v/c Ratio	0.65	0.79	0.32	1.07	0.78	0.12	0.79	0.41	0.44	0.63	0.92	
Control Delay (s/veh)	41.0	43.0	14.0	112.6	39.2	14.3	69.7	49.1	36.2	74.1	79.8	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay (s/veh)	41.0	43.0	14.0	112.6	39.2	14.3	69.7	49.1	36.2	74.1	79.8	
LOS	D	D	B	F	D	B	E	D	D	E	E	
Approach Delay (s/veh)		37.4			50.5			54.3			78.4	
Approach LOS		D			D			D			E	
Queue Length 50th (ft)	56	548	138	~262	566	46	223	165	201	102	306	
Queue Length 95th (ft)	126	641	182	#459	677	75	277	223	300	144	#468	

Lanes, Volumes, Timings

3: Finley Road & Roosevelt Road

06/12/2025

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)	543			2230			97			467		
Turn Bay Length (ft)	210		420	470		100	275		160	245		
Base Capacity (vph)	220	1554	1039	283	1693	960	681	938	638	408	647	
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.56	0.79	0.31	1.07	0.78	0.11	0.67	0.41	0.44	0.51	0.92	

Intersection Summary

Area Type: Other

Cycle Length: 150

Actuated Cycle Length: 150

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 95

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.07

Intersection Signal Delay (s/veh): 51.4

Intersection LOS: D

Intersection Capacity Utilization 93.2%

ICU Level of Service F

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 3: Finley Road & Roosevelt Road

 Ø1 20 s	 Ø2 (R) 68 s	 Ø3 34 s	 Ø4 28 s
 Ø5 17 s	 Ø6 (R) 71 s	 Ø7 22 s	 Ø8 40 s

Lanes, Volumes, Timings
3: Finley Road & Roosevelt Road

06/11/2025

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	122	1207	312	296	1288	107	449	373	277	204	463	122
Future Volume (vph)	122	1207	312	296	1288	107	449	373	277	204	463	122
Ideal Flow (vphpl)	1900	2000	1900	1900	2000	1900	1900	2000	1900	1900	1900	1900
Storage Length (ft)	210		420	470		100	275		160	245		0
Storage Lanes	1		1	1		1	2		1	2		0
Taper Length (ft)	135			95			195			250		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	0.97	0.95	1.00	0.97	0.95	0.95
Frt			0.850			0.850			0.850		0.969	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1787	3762	1599	1805	3762	1583	3467	3762	1599	3502	3463	0
Flt Permitted	0.074			0.067			0.950			0.950		
Satd. Flow (perm)	139	3762	1599	127	3762	1583	3467	3762	1599	3502	3463	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			30			30	
Link Distance (ft)		623			2310			177			547	
Travel Time (s)		12.1			45.0			4.0			12.4	
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Heavy Vehicles (%)	1%	1%	1%	0%	1%	2%	1%	1%	1%	0%	1%	1%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	124	1232	318	302	1314	109	458	381	283	208	596	0
Turn Type	pm+pt	NA	pm+ov	pm+pt	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	
Protected Phases	5	2	3	1	6	7	3	8	1	7	4	
Permitted Phases	2		2	6		6			8			
Detector Phase	5	2	3	1	6	7	3	8	1	7	4	
Switch Phase												
Minimum Initial (s)	3.0	15.0	3.0	3.0	15.0	3.0	3.0	8.0	3.0	3.0	8.0	
Minimum Split (s)	9.5	44.0	9.5	9.5	24.0	9.5	9.5	32.0	9.5	9.5	24.0	
Total Split (s)	17.0	68.0	34.0	20.0	71.0	22.0	34.0	40.0	20.0	22.0	28.0	
Total Split (%)	11.3%	45.3%	22.7%	13.3%	47.3%	14.7%	22.7%	26.7%	13.3%	14.7%	18.7%	
Yellow Time (s)	3.5	4.0	3.5	3.5	4.0	3.5	3.5	4.0	3.5	3.5	4.0	
All-Red Time (s)	0.0	2.0	1.0	0.0	2.0	1.0	1.0	2.0	0.0	1.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	-1.0	0.0	0.0	0.0	0.0	0.0	0.0	-1.8	
Total Lost Time (s)	3.5	6.0	4.5	2.5	6.0	4.5	4.5	6.0	3.5	4.5	4.2	
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Recall Mode	None	C-Min	None	None	C-Min	None	None	None	None	None	None	
Act Effct Green (s)	75.5	62.0	93.2	85.5	67.5	87.6	25.2	37.4	59.9	14.1	28.1	
Actuated g/C Ratio	0.50	0.41	0.62	0.57	0.45	0.58	0.17	0.25	0.40	0.09	0.19	
v/c Ratio	0.65	0.79	0.32	1.13	0.78	0.12	0.79	0.41	0.44	0.63	0.92	
Control Delay (s/veh)	41.0	43.0	14.0	132.9	39.2	14.3	69.7	49.1	36.2	74.1	79.8	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay (s/veh)	41.0	43.0	14.0	132.9	39.2	14.3	69.7	49.1	36.2	74.1	79.8	
LOS	D	D	B	F	D	B	E	D	D	E	E	
Approach Delay (s/veh)		37.4			54.0			54.3			78.4	
Approach LOS		D			D			D			E	
Queue Length 50th (ft)	56	548	138	~282	566	46	223	165	201	102	306	
Queue Length 95th (ft)	126	641	182	#479	677	75	277	223	300	144	#468	

Lanes, Volumes, Timings

3: Finley Road & Roosevelt Road

06/11/2025

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)		543			2230			97			467	
Turn Bay Length (ft)	210		420	470		100	275		160	245		
Base Capacity (vph)	220	1554	1039	268	1693	960	681	938	638	408	647	
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.56	0.79	0.31	1.13	0.78	0.11	0.67	0.41	0.44	0.51	0.92	

Intersection Summary

Area Type: Other

Cycle Length: 150

Actuated Cycle Length: 150

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 95

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.13

Intersection Signal Delay (s/veh): 52.5

Intersection LOS: D

Intersection Capacity Utilization 93.2%

ICU Level of Service F

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 3: Finley Road & Roosevelt Road

 Ø1	 Ø2 (R)	 Ø3	 Ø4
20 s	68 s	34 s	28 s
 Ø5	 Ø6 (R)	 Ø7	 Ø8
17 s	71 s	22 s	40 s

Intersection

Int Delay, s/veh 5.1

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	9	1522	204	155	1701	3	109	0	118	1	0	1
Future Vol, veh/h	9	1522	204	155	1701	3	109	0	118	1	0	1
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	160	-	-	150	-	-	-	-	0	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	1	-	-	1	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	97	97	97	97	97	97	97	97	97	97	97	97
Heavy Vehicles, %	0	1	0	0	1	0	0	0	0	0	0	0
Mvmt Flow	9	1569	210	160	1754	3	112	0	122	1	0	1

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	1757	0	0	1779	0	0	2889	3769	890	2878	3873	878
Stage 1	-	-	-	-	-	-	1693	1693	-	2075	2075	-
Stage 2	-	-	-	-	-	-	1196	2076	-	803	1798	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.5	6.5	6.9	7.5	6.5	6.9
Critical Hdwy Stg 1	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.5	5.5	-	6.5	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	476	-	-	435	-	-	*~ 3	0	*651	*4	0	*608
Stage 1	-	-	-	-	-	-	*206	237	-	*91	124	-
Stage 2	-	-	-	-	-	-	*574	124	-	*613	196	-
Platoon blocked, %	0	-	-	0	-	-	1	1	0	1	1	0
Mov Cap-1 Maneuver	476	-	-	435	-	-	*~ 2	0	*651	*2	0	*608
Mov Cap-2 Maneuver	-	-	-	-	-	-	*118	49	-	*45	21	-
Stage 1	-	-	-	-	-	-	*202	232	-	*58	79	-
Stage 2	-	-	-	-	-	-	*362	78	-	*489	192	-

Approach	EB	WB	NB	SB
HCM Ctrl Dly, s/v	0.07	1.5	73.03	48.83
HCM LOS			F	E

Minor Lane/Major Mvmt	NBLn1	NBLn2	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	118	651	476	-	-	435	-	-	84
HCM Lane V/C Ratio	0.951	0.187	0.02	-	-	0.368	-	-	0.024
HCM Ctrl Dly (s/v)	139.3	11.8	12.7	-	-	18	-	-	48.8
HCM Lane LOS	F	B	B	-	-	C	-	-	E
HCM 95th %tile Q(veh)	6.1	0.7	0.1	-	-	1.7	-	-	0.1

Notes

~: Volume exceeds capacity \$: Delay exceeds 300s
 +: Computation Not Defined *: All major volume in platoon

Intersection

Int Delay, s/veh 4.6

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	103	0	39	48	2	24	135	978	63	26	900	55
Future Vol, veh/h	103	0	39	48	2	24	135	978	63	26	900	55
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	0	-	-	-	-	-	145	-	-	120	-	-
Veh in Median Storage, #	-	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	97	97	97	97	97	97	97	97	97	97	97	97
Heavy Vehicles, %	0	0	0	0	0	4	0	1	0	4	0	0
Mvmt Flow	106	0	40	49	2	25	139	1008	65	27	928	57

Major/Minor	Minor2		Minor1		Major1		Major2		Major2		Major2	
Conflicting Flow All	1793	2361	492	1837	2357	537	985	0	0	1073	0	0
Stage 1	1010	1010	-	1319	1319	-	-	-	-	-	-	-
Stage 2	784	1352	-	518	1038	-	-	-	-	-	-	-
Critical Hdwy	7.5	6.5	6.9	7.5	6.5	6.98	4.1	-	-	4.18	-	-
Critical Hdwy Stg 1	6.5	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.5	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.34	2.2	-	-	2.24	-	-
Pot Cap-1 Maneuver	~ 76	37	*829	*69	37	483	929	-	-	634	-	-
Stage 1	470	469	-	*169	229	-	-	-	-	-	-	-
Stage 2	357	221	-	*782	451	-	-	-	-	-	-	-
Platoon blocked, %	0	0	0	0	0	0	-	-	-	-	-	-
Mov Cap-1 Maneuver	~ 58	30	*829	*53	30	483	929	-	-	634	-	-
Mov Cap-2 Maneuver	174	118	-	*119	120	-	-	-	-	-	-	-
Stage 1	450	449	-	*143	194	-	-	-	-	-	-	-
Stage 2	285	188	-	*712	432	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Ctrl Dly, s/v	41.61	47.77	1.1	0.29
HCM LOS	E	E		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	929	-	-	174	829	157	634	-	-
HCM Lane V/C Ratio	0.15	-	-	0.611	0.048	0.485	0.042	-	-
HCM Ctrl Dly (s/v)	9.6	-	-	53.7	9.6	47.8	10.9	-	-
HCM Lane LOS	A	-	-	F	A	E	B	-	-
HCM 95th %tile Q(veh)	0.5	-	-	3.4	0.2	2.3	0.1	-	-

Notes

~: Volume exceeds capacity \$: Delay exceeds 300s
 +: Computation Not Defined *: All major volume in platoon

Intersection							
Int Delay, s/veh	2.2						
Movement	EBL	EBR	NBL	NBT	SBU	SBT	SBR
Lane Configurations							
Traffic Vol, veh/h	29	24	50	1135	12	930	35
Future Vol, veh/h	29	24	50	1135	12	930	35
Conflicting Peds, #/hr	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	-	None
Storage Length	0	-	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	-	0	-
Grade, %	0	-	-	0	-	0	-
Peak Hour Factor	95	95	95	95	95	95	95
Heavy Vehicles, %	0	4	2	1	0	1	0
Mvmt Flow	31	25	53	1195	13	979	37

Major/Minor	Minor2	Major1		Major2			
Conflicting Flow All	1725	508	1016	0	1195	-	0
Stage 1	1023	-	-	-	-	-	-
Stage 2	703	-	-	-	-	-	-
Critical Hdwy	6.8	6.98	4.14	-	6.4	-	-
Critical Hdwy Stg 1	5.8	-	-	-	-	-	-
Critical Hdwy Stg 2	5.8	-	-	-	-	-	-
Follow-up Hdwy	3.5	3.34	2.22	-	2.5	-	-
Pot Cap-1 Maneuver	114	*819	886	-	253	-	-
Stage 1	496	-	-	-	-	-	-
Stage 2	458	-	-	-	-	-	-
Platoon blocked, %	0	0	0	-	-	-	-
Mov Cap-1 Maneuver	97	*819	886	-	253	-	-
Mov Cap-2 Maneuver	97	-	-	-	-	-	-
Stage 1	452	-	-	-	-	-	-
Stage 2	426	-	-	-	-	-	-

Approach	EB	NB	SB
HCM Ctrl Dly, s/v	38.73	1.12	1.53
HCM LOS	E		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	152	-	161	-	-
HCM Lane V/C Ratio	0.059	-	0.346	-	-
HCM Ctrl Dly (s/v)	9.3	0.8	38.7	1.4	-
HCM Lane LOS	A	A	E	A	-
HCM 95th %tile Q(veh)	0.2	-	1.4	-	-

Notes	
~: Volume exceeds capacity	\$. Delay exceeds 300s
+: Computation Not Defined	*: All major volume in platoon

Intersection						
Int Delay, s/veh	0.1					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		↗		↑↑	↑↑	
Traffic Vol, veh/h	0	21	0	1105	998	73
Future Vol, veh/h	0	21	0	1105	998	73
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	0	0	0	2	2	0
Mvmt Flow	0	22	0	1163	1051	77

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	-	564	-	0	-	0
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.9	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.3	-	-	-	-
Pot Cap-1 Maneuver	0	474	0	-	-	-
Stage 1	0	-	0	-	-	-
Stage 2	0	-	0	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	-	474	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	EB	NB	SB
HCM Ctrl Dly, s/v	12.96	0	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	-	474	-	-
HCM Lane V/C Ratio	-	0.047	-	-
HCM Ctrl Dly (s/v)	-	13	-	-
HCM Lane LOS	-	B	-	-
HCM 95th %tile Q(veh)	-	0.1	-	-

Intersection

Int Delay, s/veh 0.1

Movement EBL EBR NBL NBT SBT SBRLane Configurations 

Traffic Vol, veh/h 0 21 0 1105 960 59

Future Vol, veh/h 0 21 0 1105 960 59

Conflicting Peds, #/hr 0 0 0 0 0 0

Sign Control Stop Stop Free Free Free Free

RT Channelized - None - None - None

Storage Length - 0 - - - -

Veh in Median Storage, # 0 - - 0 0 -

Grade, % 0 - - 0 0 -

Peak Hour Factor 95 95 95 95 95 95

Heavy Vehicles, % 0 0 0 2 2 0

Mvmt Flow 0 22 0 1163 1011 62

Major/Minor Minor2 Major1 Major2

Conflicting Flow All - 536 - 0 - 0

Stage 1 - - - - - -

Stage 2 - - - - - -

Critical Hdwy - 6.9 - - - -

Critical Hdwy Stg 1 - - - - - -

Critical Hdwy Stg 2 - - - - - -

Follow-up Hdwy - 3.3 - - - -

Pot Cap-1 Maneuver 0 494 0 - - -

Stage 1 0 - 0 - - -

Stage 2 0 - 0 - - -

Platoon blocked, % - - - - - -

Mov Cap-1 Maneuver - 494 - - - -

Mov Cap-2 Maneuver - - - - - -

Stage 1 - - - - - -

Stage 2 - - - - - -

Approach EB NB SB

HCM Ctrl Dly, s/v 12.63 0 0

HCM LOS B

Minor Lane/Major Mvmt NBT EBLn1 SBT SBR

Capacity (veh/h) - 494 - -

HCM Lane V/C Ratio - 0.045 - -

HCM Ctrl Dly (s/v) - 12.6 - -

HCM Lane LOS - B - -

HCM 95th %tile Q(veh) - 0.1 - -

Capacity Analysis Summary Sheets
Year 2031 Total Projected Saturday Midday Peak Hour

Lanes, Volumes, Timings
3: Finley Road & Roosevelt Road

06/11/2025

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		 			 		 	 		 	 	
Traffic Volume (vph)	110	1142	267	279	1092	119	331	301	264	127	331	85
Future Volume (vph)	110	1142	267	279	1092	119	331	301	264	127	331	85
Ideal Flow (vphpl)	1900	2000	1900	1900	2000	1900	1900	2000	1900	1900	1900	1900
Storage Length (ft)	210		420	470		100	275		160	245		0
Storage Lanes	1		1	1		1	2		1	2		0
Taper Length (ft)	135			95			195			250		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	0.97	0.95	1.00	0.97	0.95	0.95
Frt			0.850			0.850			0.850		0.969	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1752	3762	1615	1787	3762	1599	3502	3762	1599	3502	3491	0
Flt Permitted	0.181			0.095			0.950			0.950		
Satd. Flow (perm)	334	3762	1615	179	3762	1599	3502	3762	1599	3502	3491	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		35			35			30			30	
Link Distance (ft)		623			970			178			547	
Travel Time (s)		12.1			18.9			4.0			12.4	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles (%)	3%	1%	0%	1%	1%	1%	0%	1%	1%	0%	0%	1%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	113	1177	275	288	1126	123	341	310	272	131	429	0
Turn Type	pm+pt	NA	pm+ov	pm+pt	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	
Protected Phases	5	2	3	1	6	7	3	8	1	7	4	
Permitted Phases	2		2	6		6			8			
Detector Phase	5	2	3	1	6	7	3	8	1	7	4	
Switch Phase												
Minimum Initial (s)	3.0	15.0	3.0	3.0	15.0	3.0	3.0	8.0	3.0	3.0	8.0	
Minimum Split (s)	9.5	44.0	9.5	9.5	24.0	9.5	9.5	25.0	9.5	9.5	24.0	
Total Split (s)	13.0	60.0	22.0	23.0	70.0	22.0	22.0	25.0	23.0	22.0	25.0	
Total Split (%)	10.0%	46.2%	16.9%	17.7%	53.8%	16.9%	16.9%	19.2%	17.7%	16.9%	19.2%	
Yellow Time (s)	3.5	4.0	3.5	3.5	4.0	3.5	3.5	4.0	3.5	3.5	4.0	
All-Red Time (s)	0.0	2.0	1.0	0.0	2.0	1.0	1.0	2.0	0.0	1.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	3.5	6.0	4.5	3.5	6.0	4.5	4.5	6.0	3.5	4.5	6.0	
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Recall Mode	None	C-Min	None	None	C-Min	None	None	None	None	None	None	
Act Effct Green (s)	67.4	56.4	78.8	80.5	66.0	82.2	16.4	25.3	49.4	10.2	19.2	
Actuated g/C Ratio	0.52	0.43	0.61	0.62	0.51	0.63	0.13	0.19	0.38	0.08	0.15	
v/c Ratio	0.43	0.72	0.28	0.86	0.59	0.12	0.78	0.42	0.45	0.48	0.83	
Control Delay (s/veh)	16.7	34.0	13.4	52.9	24.5	10.0	67.5	48.1	32.8	62.8	68.7	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay (s/veh)	16.7	34.0	13.4	52.9	24.5	10.0	67.5	48.1	32.8	62.8	68.7	
LOS	B	C	B	D	C	B	E	D	C	E	E	
Approach Delay (s/veh)		29.2			28.6			50.8			67.3	
Approach LOS		C			C			D			E	
Queue Length 50th (ft)	37	438	107	160	349	40	144	120	164	55	187	
Queue Length 95th (ft)	64	525	159	#306	422	64	197	172	254	87	#268	

Lanes, Volumes, Timings

3: Finley Road & Roosevelt Road

06/11/2025

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)	543			890			98			467		
Turn Bay Length (ft)	210		420	470		100	275		160	245		
Base Capacity (vph)	279	1632	992	351	1909	1100	471	733	625	471	518	
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.41	0.72	0.28	0.82	0.59	0.11	0.72	0.42	0.44	0.28	0.83	

Intersection Summary

Area Type: Other

Cycle Length: 130

Actuated Cycle Length: 130

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.86

Intersection Signal Delay (s/veh): 38.0

Intersection LOS: D

Intersection Capacity Utilization 83.8%

ICU Level of Service E

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 3: Finley Road & Roosevelt Road

	Ø1		Ø2 (R)		Ø3		Ø4
23 s		60 s		22 s		25 s	
	Ø5		Ø6 (R)		Ø7		Ø8
13 s		70 s		22 s		25 s	

Intersection

Int Delay, s/veh 11.2

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	29	1319	230	171	1335	2	162	0	198	1	0	0
Future Vol, veh/h	29	1319	230	171	1335	2	162	0	198	1	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	160	-	-	150	-	-	-	-	0	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	1	-	-	1	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	96	96	96	96	96	96	96	96	96	96	96	96
Heavy Vehicles, %	0	1	0	0	1	0	0	0	0	0	0	0
Mvmt Flow	30	1374	240	178	1391	2	169	0	206	1	0	0

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	1393	0	0	1614	0	0	2606	3303	807	2495	3422	696
Stage 1	-	-	-	-	-	-	1554	1554	-	1748	1748	-
Stage 2	-	-	-	-	-	-	1052	1749	-	747	1674	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.5	6.5	6.9	7.5	6.5	6.9
Critical Hdwy Stg 1	-	-	-	-	-	-	6	5.5	-	6.5	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6	5.5	-	6.5	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	678	-	-	501	-	-	~ 12	9	*725	*15	7	*713
Stage 1	-	-	-	-	-	-	252	259	-	*154	193	-
Stage 2	-	-	-	-	-	-	658	193	-	*683	214	-
Platoon blocked, %	0	-	-	0	-	-			0			0
Mov Cap-1 Maneuver	678	-	-	501	-	-	~ 8	5	*725	*7	4	*713
Mov Cap-2 Maneuver	-	-	-	-	-	-	~ 140	64	-	*70	21	-
Stage 1	-	-	-	-	-	-	240	248	-	*99	124	-
Stage 2	-	-	-	-	-	-	424	124	-	*467	204	-

Approach	EB	WB	NB	SB
HCM Ctrl Dly, s/v	0.19	1.82	99.03	57.5
HCM LOS			F	F

Minor Lane/Major Mvmt	NBLn1	NBLn2	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	140	725	678	-	-	501	-	-	70
HCM Lane V/C Ratio	1.21	0.285	0.045	-	-	0.355	-	-	0.015
HCM Ctrl Dly (s/v)	205.5	11.9	10.6	-	-	16.1	-	-	57.5
HCM Lane LOS	F	B	B	-	-	C	-	-	F
HCM 95th %tile Q(veh)	10	1.2	0.1	-	-	1.6	-	-	0

Notes

~: Volume exceeds capacity \$: Delay exceeds 300s
 +: Computation Not Defined *: All major volume in platoon

Intersection												
Int Delay, s/veh	12.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	167	0	61	49	1	31	147	709	36	29	723	52
Future Vol, veh/h	167	0	61	49	1	31	147	709	36	29	723	52
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	0	-	-	-	-	-	145	-	-	120	-	-
Veh in Median Storage, #	-	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	0	0	0	0	0	3	0	1	0	0	0	0
Mvmt Flow	186	0	68	54	1	34	163	788	40	32	803	58

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	1618	2051	431	1601	2060	414	861	0	0	828	0	0
Stage 1	897	897	-	1134	1134	-	-	-	-	-	-	-
Stage 2	721	1154	-	466	926	-	-	-	-	-	-	-
Critical Hdwy	7.5	6.5	6.9	7.5	6.5	6.96	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.5	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.5	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.33	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	~ 98	63	*888	*102	62	585	975	-	-	812	-	-
Stage 1	480	485	-	*219	280	-	-	-	-	-	-	-
Stage 2	389	274	-	*837	467	-	-	-	-	-	-	-
Platoon blocked, %	0	0	0	0	0	0	-	-	-	-	-	-
Mov Cap-1 Maneuver	~ 73	50	*888	*75	49	585	975	-	-	812	-	-
Mov Cap-2 Maneuver	187	144	-	*149	139	-	-	-	-	-	-	-
Stage 1	461	465	-	*182	233	-	-	-	-	-	-	-
Stage 2	303	228	-	*743	449	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Ctrl Dly, s/v	87.56	34.79	1.56	0.35
HCM LOS	F	D		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	975	-	-	187	888	208	812	-	-
HCM Lane V/C Ratio	0.168	-	-	0.995	0.076	0.432	0.04	-	-
HCM Ctrl Dly (s/v)	9.4	-	-	116.1	9.4	34.8	9.6	-	-
HCM Lane LOS	A	-	-	F	A	D	A	-	-
HCM 95th %tile Q(veh)	0.6	-	-	8.3	0.2	2	0.1	-	-

Notes	
~: Volume exceeds capacity	\$. Delay exceeds 300s
+: Computation Not Defined	*: All major volume in platoon

Intersection							
Int Delay, s/veh	1.3						
Movement	EBL	EBR	NBL	NBT	SBU	SBT	SBR
Lane Configurations	Y			↑↑		↑↑	
Traffic Vol, veh/h	32	22	25	851	9	806	18
Future Vol, veh/h	32	22	25	851	9	806	18
Conflicting Peds, #/hr	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	-	None
Storage Length	0	-	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	-	0	-
Grade, %	0	-	-	0	-	0	-
Peak Hour Factor	91	91	91	91	91	92	91
Heavy Vehicles, %	0	0	0	1	0	0	0
Mvmt Flow	35	24	27	935	10	876	20

Major/Minor	Minor2	Major1		Major2			
Conflicting Flow All	1428	448	896	0	935	-	0
Stage 1	906	-	-	-	-	-	-
Stage 2	523	-	-	-	-	-	-
Critical Hdwy	6.8	6.9	4.1	-	6.4	-	-
Critical Hdwy Stg 1	5.8	-	-	-	-	-	-
Critical Hdwy Stg 2	5.8	-	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	2.5	-	-
Pot Cap-1 Maneuver	185	*871	962	-	371	-	-
Stage 1	534	-	-	-	-	-	-
Stage 2	565	-	-	-	-	-	-
Platoon blocked, %	0	0	0	-	-	-	-
Mov Cap-1 Maneuver	172	*871	962	-	371	-	-
Mov Cap-2 Maneuver	172	-	-	-	-	-	-
Stage 1	513	-	-	-	-	-	-
Stage 2	545	-	-	-	-	-	-

Approach	EB	NB	SB
HCM Ctrl Dly, s/v	23.33	0.57	0.67
HCM LOS	C		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	103	-	255	-	-
HCM Lane V/C Ratio	0.029	-	0.233	-	-
HCM Ctrl Dly (s/v)	8.9	0.3	23.3	0.5	-
HCM Lane LOS	A	A	C	A	-
HCM 95th %tile Q(veh)	0.1	-	0.9	-	-

Notes	
~: Volume exceeds capacity	\$. Delay exceeds 300s
+: Computation Not Defined	*: All major volume in platoon

Intersection						
Int Delay, s/veh	0.2					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		↗		↑↑	↑↑	
Traffic Vol, veh/h	0	33	0	907	802	75
Future Vol, veh/h	0	33	0	907	802	75
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	0	0	0	2	2	0
Mvmt Flow	0	35	0	955	844	79

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	-	462	-	0	-	0
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.9	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.3	-	-	-	-
Pot Cap-1 Maneuver	0	552	0	-	-	-
Stage 1	0	-	0	-	-	-
Stage 2	0	-	0	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	-	552	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	EB	NB	SB
HCM Ctrl Dly, s/v	11.95	0	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	-	552	-	-
HCM Lane V/C Ratio	-	0.063	-	-
HCM Ctrl Dly (s/v)	-	12	-	-
HCM Lane LOS	-	B	-	-
HCM 95th %tile Q(veh)	-	0.2	-	-

Intersection						
Int Delay, s/veh	0.2					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		↗		↑↑	↑↑	
Traffic Vol, veh/h	0	33	0	907	771	64
Future Vol, veh/h	0	33	0	907	771	64
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	0	0	0	2	2	0
Mvmt Flow	0	35	0	955	812	67
Major/Minor	Minor2	Major1		Major2		
Conflicting Flow All	-	439	-	0	-	0
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.9	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.3	-	-	-	-
Pot Cap-1 Maneuver	0	571	0	-	-	-
Stage 1	0	-	0	-	-	-
Stage 2	0	-	0	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	-	571	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	EB	NB		SB		
HCM Ctrl Dly, s/v	11.71	0		0		
HCM LOS	B					
Minor Lane/Major Mvmt	NBT EBLn1		SBT	SBR		
Capacity (veh/h)	- 571		-	-		
HCM Lane V/C Ratio	- 0.061		-	-		
HCM Ctrl Dly (s/v)	- 11.7		-	-		
HCM Lane LOS	- B		-	-		
HCM 95th %tile Q(veh)	- 0.2		-	-		