Existing Conditions Traffic Evaluation Glenbard East High School

Lombard, Illinois



Prepared For:



1. Introduction

This report summarizes the preliminary findings of a traffic evaluation being conducted by Kenig, Lindgren, O'Hara Aboona, Inc. (KLOA, Inc.) for the Glenbard East High School located at 1014 S. Main Street in Lombard, Illinois.

The campus is generally bounded by Main Street on the east, Wilson Avenue on the north, and residential homes to the west and south. The purpose of this study is to document the current drop-off and pick-up activities at the school and prepare geometric and operational improvements to improve the drop-off and pick-up activities and limit the impact of the school loading activities on Main Street.

Figure 1 shows an aerial view of Glenbard East High School in relation to the area roadway network and adjacent land uses.

The sections of this report present the following:

- Existing roadway conditions
- Existing campus operations
- Evaluation of the existing access system and drop-off/pick-up operations
- Traffic analyses for the weekday morning and weekday afternoon peak hours
- Prepared campus improvement alternatives to address concerns related to:
 - Improving the circulation of traffic through the consolidation of curb cuts.
 - Separating passenger vehicle and bus loading activities.
 - Creating designated loading and staging areas for buses.
 - Enhancing the available on campus parking for staff and students.
 - Reducing the need for student parking on Wilson Avenue.
 - Limiting the opportunity for drop-off/pick-up queues to extend onto Main Street.
- Evaluation of the campus improvement alternatives to determine their viability





Aerial View of Site

Figure 1

Glenbard East High School Lombard, Illinois



2. Existing Conditions

Existing traffic and roadway conditions were documented based on field visits, traffic counts, and observations conducted by KLOA, Inc. The following provides a detailed description of the physical characteristics of the roadways including geometry and traffic control, adjacent land uses and peak hour traffic flows along area roadways.

Campus Location

The campus is located in the southwest corner of the intersection of Main Street with Wilson Avenue and is bounded by residential homes on the south and west. Land uses in the vicinity of the site are primarily residential in all directions with various commercial uses located along Main Street, such as the Lombard Pines Shopping Center located 1,500 feet southeast of the campus.

Existing Roadway System Characteristics

The characteristics of the existing roadways that surround the proposed development are illustrated in **Figure 2** and described below. All of the roadway are under the jurisdiction of the Village of Lombard.

Main Street is a north-south roadway that provides two travel lanes in each direction and is classified as a minor arterial roadway in the 2014 Comprehensive Plan. At its signalized intersection with Wilson Avenue, Main Street provides an exclusive left-turn lane, a through lane, and a shared through/right-turn lane on the northbound and southbound approaches. At its signalized intersection with the Glenbard East High School signalized access drive, Main Street provides an exclusive left-turn lane and two through lanes on the northbound approach and a through lane and a shared through/right-turn lane on the southbound approach. No exclusive turn lanes are provided on Main Street at the four unsignalized access drives serving Glenbard East High School. High visibility crosswalks are provided on the north and south legs of the signalized intersection of Main Street with Wilson Avenue and on the north leg of the signalized intersection of Main Street with the Glenbard East High School signalized access drive. Main Street and a shared through I and a street with Wilson Avenue and on the north leg of the signalized intersection of Main Street with Wilson Avenue and on the north leg of the signalized intersection of Main Street with the Glenbard East High School signalized access drive. Main Street carries an annual average daily traffic (AADT) volume of 13,100 vehicles (IDOT 2020), and has a posted speed limit of 35 miles per hour.

Wilson Avenue is an east-west roadway that provides one travel lane in each direction and is classified as a collector street in the 2014 Comprehensive Plan. At its signalized intersection with Main Street, Wilson Avenue provides an exclusive left-turn lane and a shared through/right-turn lane on the eastbound and westbound approaches. Wilson Avenue operates under free flow conditions at its unsignalized intersections with Lincoln Avenue, Elizabeth Street, and at the Glenbard East High School access drives and exclusive turn lanes are not provided along the roadway. High visibility crosswalks are provided on the east and west legs of the signalized intersection of Main Street with Wilson Avenue, and on the east leg of the unsignalized intersection of Wilson Avenue with Elizabeth Street. West of Lincoln Avenue, on-street parking is available for students along the south side of the roadway. Wilson Avenue carries an AADT volume of 2,900 vehicles (IDOT 2020) and has a posted speed limit of 25 miles per hour.





Elizabeth Street is a north-south roadway that extends north from Wilson Avenue, provides one travel lane in each direction, and is classified as a collector roadway in the 2014 Comprehensive Plan. Elizabeth Street provides a single-lane approach and is under stop sign control at its intersection with Wilson Avenue.

Lincoln Avenue is a north-south roadway that extends north from Wilson Avenue, provides one travel lane in each direction, and is classified as a local roadway in the 2014 Comprehensive Plan. Lincoln Avenue provides a single-lane approach and is under stop sign control at its intersection with Wilson Avenue.

Main Drive is a north-south frontage roadway that runs parallel to Main Street north of Central Avenue. Approximately 800 feet north of Central Avenue, Main Drive curves west and intersects Main Street where it provides a single-lane approach under stop sign control.

Glenbard East High School Existing Characteristics

Glenbard East High School has an enrollment of approximately 2,250 students ranging from Grades 9 to 12 and 180 faculty members. The school day starts at 7:35 A.M. with dismissal occurring at 2:35 P.M. Under existing conditions, the campus primarily consists of the school building, a football field, two tennis courts, approximately six various athletic fields, and three main surface parking locations as follows:

- South of the school building there is an approximately 162-space parking lot that is utilized for staff parking. Three of the parking spaces are ADA accessible spaces and 24 spaces are reserved spaces including spaces for small buses, maintenance personnel, and driver's education vehicles.
- North of the school building there is an approximately 137-space parking lot that is utilized for staff and visitor parking. Three of the parking spaces are ADA accessible spaces, two parking spaces are reserved for the Lombard Police Department, and six spaces are reserved for visitors.
- Northwest of the school building there is an approximately 113-space parking lot that is utilized for student permit parking between 6:00 A.M. and 2:30 P.M. Two of the parking spaces are ADA accessible spaces.

As previously indicated, additional student parking is permitted along the south side of Wilson Avenue west of Lincoln Avenue. This on-street parking can accommodate approximately 51 vehicles.

Access to the campus parking locations is provided via the following:

• A full movement, signalized access drive off Main Street located 375 feet north of Central Avenue and 800 feet south of Wilson Avenue. This access drive serves the staff parking lot on the south side of the school building and provides one inbound lane and two outbound lanes striped to provide an exclusive left-turn lane and an exclusive right-turn lane.



- An outbound only access drive on Wilson Avenue located approximately 280 feet west of Main Street. This access drives provides two outbound lanes striped to provide an exclusive left-turn lane and an exclusive right-turn lane with outbound movements under stop sign control.
- A full movement access drive on Wilson Avenue that is located 40 feet east of Lincoln Avenue and 690 feet west of Main Street. This access drive provides one inbound lane and one outbound lane with outbound movements under stop sign control.
- A full movement access drive on Wilson Avenue located 165 feet west of Lincoln Avenue which provides one inbound lane and one outbound lane with outbound movements under stop sign control.

Furthermore, there is a circulation road located on the east side of the school building that is utilized during student drop-off and pick-up. This circulation drive provides a single inbound access drive located 245 feet south of Wilson Avenue and three outbound only access drives located approximately 335, 525, and 700 feet south of Wilson Avenue.

Existing Drop-Off/Pick-Up Procedures

Drop-off activities for passenger vehicles primarily occur within the following locations:

- The parking lot located on the north side of the school building with queues forming within the vicinity of Door 1. A dedicated student loading area is provided within the north parking lot dedicated to drop-off/pick-up activities that keeps activities separate from the faculty/staff parking spaces. Vehicles enter the parking lot via the access drive located opposite Lincoln Avenue and exit via the eastern outbound only access drive.
- Within the circulation drive located on the east side of the school building within the vicinity of Door 24. Vehicles enter the circulation drive via the northern inbound only access drive and exit via either of the three outbound only access drives.

Secondary drop-off activities for passenger vehicles also occur within the following locations:

• The southern faculty/staff parking lot near the sidewalk connecting to Door 24 on the east side of the school building, within the student permit parking lot in which students will walk to Door 1, and along Wilson Avenue and Lincoln Avenue.

Drop-off activities for buses also occur within the staff/visitor parking lot located on the north side of the school building within the vicinity of Door 1.

Pick-up activities for buses occur within the northern parking lot and along the circulation drive located on the east side of the school building. As a result, pick-up activities for passenger vehicles primarily occur within the southern faculty/staff parking lot, behind buses in the circulation drive on the east side of the school building, within the faculty/staff parking areas within the north parking lot, along Wilson Avenue, and along Lincoln Avenue.



Existing Traffic Volumes

In order to determine current traffic conditions in the vicinity of the site, KLOA, Inc. conducted peak period vehicle, pedestrian, and bicycle movement traffic counts on Thursday, April 4, 2024 during the weekday morning (6:30 A.M. to 8:30 A.M.) and weekday afternoon (1:30 P.M. to 3:30 P.M.) peak periods at the following intersections:

- Main Street with Wilson Avenue
- Main Street with the Glenbard East Signalized Access Drive
- Wilson Avenue with Lincoln Avenue/Middle Access Drive
- Wilson Avenue with Elizabeth Street
- Wilson Avenue with West Access Drive
- Wilson Avenue with East Access Drive
- Main Street with Inbound Only Access Drive
- Main Street with Main Drive/North Outbound Only Access Drive
- Main Street with Middle Outbound Only Access Drive
- Main Street with South Outbound Only Access Drive

Based on the results of the traffic counts the weekday morning peak hour of school activity occurs between 6:45 A.M. and 7:45 A.M. and that the weekday afternoon peak hour of school occurs between 2:30 P.M. and 3:30 P.M. These traffic counts coincide with the arrival and dismissal start time of the school and were utilized for the purposes of this evaluation.

The existing peak hour traffic volumes are illustrated in **Figure 3** and the existing pedestrian and bicycle volumes are illustrated in **Figure 4**. Copies of the traffic count summary sheets are included in the Appendix.







LOMBARD, ILLINOIS

Job No: 24-093

Figure: 4

On-Site Campus Observations

Observations were conducted during the arrival and dismissal periods on the same day as the traffic counts to determine the existing operations of arrival and dismissal procedures, determine on-site vehicle stacking, and to identify issues related to vehicle, bus, and pedestrian conflicts. The results of the observations and traffic counts indicated the following:

- During the morning arrival period, there were approximately 502 drop-off vehicles of which 308 vehicles utilized the northern staff/faculty parking lot, 137 vehicles utilized the drive aisle on east side of the school building, 42 vehicles utilized the southern staff/faculty parking lot, and 15 vehicles utilized the student permit parking lot.
 - The peak 30 minutes of activity occurs between 7:05 A.M. and 7:35 A.M. with all major activity concluding by 7:35 A.M. (school start time).
 - At the western access drive on Wilson Avenue, approximately 55 percent of the inbound trips arrived from the west, 40 percent arrived from the east, and five percent arrived from Lincoln Avenue.
 - During peak inbound school activity, inbound left-turn movements are required to yield for right-turn vehicles which results in vehicle queues on Wilson Avenue. Since no left-turn lane is provided, these queues would block the progression of through traffic along Wilson Avenue.
 - These queues extended beyond the east outbound only access drive two times momentarily (less than one minute) and one time for an extended duration for approximately five minutes between 7:25 A.M. and 7:30 A.M. However, courtesy gaps were provided for outbound left-turn vehicles from the access drive and outbound queues do not impact on-campus circulation of vehicles.
 - These queues were not observed to extend into the signalized intersection of Main Street with Wilson Avenue.
 - During these queues, students were observed to exit vehicles while the vehicle was stationary on Wilson Avenue.
 - These queues were a combination of westbound left-turn vehicles onto the campus at the middle access drive, outbound left-turn vehicles at the east access drive that are then traveling straight at Lincoln Avenue, and westbound through traffic on Wilson Avenue.





- During the peak queue, there were approximately 43 vehicles (no buses) queued on the north side of the campus consisting of 14 vehicles queued within the north parking lot, 18 vehicles queued to the east on Wilson Avenue, five vehicles queued to the west on Wilson Avenue, and six vehicles queued to the north on Lincoln Avenue. This peak queue is illustrated in **Figure A.** All lettered figures are included in the Appendix.
- While not striped, inbound vehicles at the middle access drive were observed to utilize the outbound lane of the access drive to either create a side-by-side loading queue or to access the staff/faculty parking spaces. No outbound vehicles were counted at the middle access drive during the peak hours.
- At the circulation drive located on the east side of the school building, approximately 73 percent of the inbound trips arrived from the north on Main Street and 27 percent of the inbound trips arrived from the south on Main Street.
 - The majority of the vehicles utilizing the circulation drive arrive from the north given the orientation of the single inbound only access drive.
 - 85 percent (33) of the vehicles arriving from the south utilize the northern outbound only access drive. This is likely due to the orientation of the inbound only access drive and/or the queue of vehicles limiting the ability for vehicles to turn left onto the circulation drive.
 - The majority of students are dropped off at Door 24 and as such, the majority of outbound traffic was utilizing the southern outbound only access drive. Due to the width of this driveway, for passenger vehicles it functions as two outbound lanes utilized as an exclusive left-turn lane and an exclusive right-turn lane.
 - During the peak queue, there were approximately 14 vehicles queued within the circulation drive of which one vehicle was queued back onto Main Street. At this time, there was one bus stopped on the circulation drive, but passenger vehicles were queued around it. This peak queue is illustrated in **Figure B**.
- At the signalized access drive on Main Street, approximately 70 percent of the inbound trips arrived from the south on Main Street and 30 percent arrived from the north on Main Street.
 - Vehicles utilizing this access drive for drop-off activities were observed to stop near the sidewalk that connects to Door 24 and inbound queues were not observed to extend to the signalized intersection of the access drive with Main Street.



- In lieu of circulating the parking lot via the designated drive aisles, the majority of drop-off vehicles were observed to utilize the vacant ADA accessible parking spaces to access the outbound lanes.
- At the access drive serving the student permit parking lot, approximately 54 percent of vehicles arrived from the west and 46 percent arrived from the east.
 - Inbound vehicles (both left turns and right turns) must yield to pedestrians crossing the driveway.
 - Inbound left-turn vehicles must also yield to eastbound through movements which at times during the arrival period extended from the middle access drive west past the access drive serving the student permit lot.
 - When westbound left-turn vehicles are queued on Wilson Avenue waiting to turn left onto the access drives, these queues do not extend east to Lincoln Avenue/middle access drive.
- During the afternoon dismissal period, there were approximately 162 pick-up vehicles on campus of which 74 vehicles utilized the southern staff/faculty parking lot, 64 vehicles utilized the northern staff/faculty parking lot, 14 vehicles utilized the circulation drive on the east side of the school building, and 13 vehicles utilized the student permit parking lot. The remaining pick-up activity occurred along the south side of Wilson Avenue and on Lincoln Avenue with a few vehicles picking up students on Main Street.
 - The peak departure activity occurs between 2:35 P.M. (school dismissal) and 2:50 P.M.
 - At the signalized access drive on Main Street, approximately 26 percent of vehicles depart to the north and 74 percent of vehicles depart to the south.
 - Vehicles begin queueing at Door 20 starting at 2:05 P.M.
 - By 2:32 P.M., this queue extends onto Main Street with vehicles queueing in the southbound curbside lane.
 - Peak inbound queue occurs at 2:38 P.M. with three vehicles queueing on Main Street and these queues clear Main Street by 2:39 P.M. This peak queue is illustrated in Figure C.
 - Given the width of the inbound (westbound) lane, this queue blocks inbound vehicles from accessing the parking spaces within the southern parking lot. As such, inbound vehicles that are not queueing for pick-up activities and trying to access available parking spaces utilize the outbound only lane (south of the landscape median) and traverse the underutilized ADA accessible parking spaces to bypass the queue lane.



- At times inbound vehicles utilize any available lane to access the parking lot, as shown in **Figure D**, due to the queue of inbound vehicles.
- It should be noted that on occasion, a police officer will pull in behind this vehicle queue with their lights on to bring awareness of the queue to southbound vehicles on Main Street.
- Furthermore, on occasion, given the queue of vehicles originating from the south parking lot and the circulation drive (as discussed later), southbound queues on Main Street can extend north beyond Wilson Avenue.
- At the three outbound only access drives serving the circulation drive, approximately 32 percent of vehicles depart to the north and 68 percent of vehicles depart to the south.
 - The circulation drive is primarily utilized by buses (as discussed later) with vehicles filling in behind the buses starting at 2:00 P.M.
 - Given that buses queue and block outbound movements from the middle and south outbound only access drives, passenger vehicles queues start at the northern outbound only access drive, limiting available on-campus queueing (four or five vehicles).
 - By 2:24 P.M., this queue was observed to extend onto Main Street with vehicles queueing along the southbound curb lane.
 - The peak queue occurred at 2:36 P.M. with seven vehicles queued on Main Street. These queues extended to the signalized intersection of Main Street with Wilson Avenue, as illustrated in **Figure D**.
 - These queues cleared Main Street by 2:43 P.M. with vehicles either leaving the queue to proceed south onto Main Street or turning right from the northern outbound only access drive onto Main Street.
 - Several parents/guardians were observed to pick up students along Main Street.
- At the northern parking lot one third of the outbound vehicles exited at the middle access drive and two thirds of the outbound vehicle exited at the eastern access drive. 48 percent of outbound vehicles departed to the west and 52 percent of vehicles departed to the east.
 - The circulation drives within the northern parking lot are primarily utilized by buses (as discussed later). However, some of the space within the drive aisles serving the parking lot or vacant parking spaces is utilized by passenger vehicles at locations that do not block parking spaces with vehicles arriving before 2:00 P.M.



- At the student permit parking lot, 53 percent of vehicles departed to the west and 47 percent of vehicles departed to the east
 - Overall, while outbound movements from the parking lot must yield to eastbound and westbound through traffic on Wilson Avenue as well as pedestrians crossing the driveway on the sidewalk, outbound movements from this parking lot operated with limited delays given at least 53 percent of vehicles were departing to the west and were not intermixing with the queueing of vehicles along Wilson Avenue east of the access drive.
- During the morning arrival period, there were 26 buses of which 25 buses utilized the northern faculty/staff parking lot and one bus utilized the circulation drive. Bus activity generally occurred between 7:05 A.M. and 7:20 A.M.
- During the afternoon dismissal period, there were 25 buses of which 11 buses utilized the circulation drive and 14 buses utilized the northern parking lot.
 - Buses begin queueing within the circulation drive starting at 2:00 P.M. with buses pulling up all the way to Main Street. These buses departed at 2:45 P.M. with the circulation drive clear by 2:47 P.M. Three buses turned right onto Main Street and eight buses turned left onto Main Street.
 - Supplemental observations during the afternoon dismissal period indicated that at times when a police office is present along Main Street providing awareness to passenger vehicles queued within the roadway, this officer will help facilitate the outbound movements of buses from the circulation drive.
 - Buses begin queueing within the northern parking lot starting at 2:10 P.M. with buses pulling all the way forward to Wilson Avenue within the outbound right-turn only lane.
 - The peak bus queue within the northern parking lot was 14 vehicles, as illustrated in **Figure E**. Buses depart the parking lot at 2:45 P.M. with all bus activity concluding by 2:46 P.M. with buses able to enter Wilson Avenue with vehicles yielding right-of-way.
 - Given that buses queue within the outbound right-turn only lane during the bus dismissal period, conflicting left-turn movements occur between outbound buses and passenger vehicles exiting the north parking lot and turning onto Wilson Avenue.
- While the bus activity is separated within two locations to minimize impacts of bus queueing on the area roadway network, the utilization of two of the four available off-street loading areas for bus loading results in vehicles staging on Wilson Avenue (particularly with the no-parking zones provided on the south side of the roadway), along Lincoln Avene, and may contribute to the number of passenger vehicles utilizing the south parking lot.



- Vehicles begin queueing along the south side of Wilson Avenue starting at 2:00 P.M.
- Vehicles queued along the south side of Wilson Avenue reduce the effective width of the roadway, particularly within the proximity of Lincoln Avenue in which the eastbound travel lane is only 14 feet wide.
- **Figure F** illustrates the peak queue for vehicles utilizing Wilson Avenue and Lincoln Avenue for afternoon pick-up activities.
- The majority of pedestrian activity was observed to occur at the following locations:
 - The sidewalk located on the south side of Wilson Avenue (up to 70 pedestrians)
 - The south leg of the intersection of Main Street with Wilson Avenue (up to 50 pedestrians)
 - The west leg of the intersection of Main Street with the signalized access drive (up to 25 pedestrians)
 - The west leg of the intersection of Wilson Avenue with Lincoln Avenue (up to 20 pedestrians)
 - Furthermore, approximately 20 pedestrians were observed crossing Wilson Avenue at Elizabeth Street.
- Students crossing Wilson Avenue at Lincoln Avenue mostly relied on vehicles yielding to allow them to cross.
- There were a limited number of bicyclists accessing the campus during the arrival and departure periods.



3. Evaluation of Existing Conditions

The following provides an evaluation conducted for the weekday morning (school arrival) and weekday afternoon (school dismissal) peak hours. The analysis includes conducting capacity analyses to determine how well the roadway system and access drives operate and an evaluation of the on-site queueing and stacking. These observations will be utilized to determine any roadway improvements or modifications to the arrival and dismissal procedures that may be required to minimize the arrival and dismissal activities on the area roadway network.

Traffic Analyses

Roadway and adjacent or nearby intersection analyses were performed for the weekday morning and weekday afternoon peak hours for the existing traffic volumes.

The traffic analyses were performed using the methodologies outlined in the Transportation Research Board's *Highway Capacity Manual (HCM)*, 6th Edition and analyzed using Synchro/SimTraffic 11 software. To determine the average overall vehicle delay and levels of service for the traffic signal-controlled intersections, actual cycle lengths and phasings were utilized for the intersections of Main Street with Wilson Avenue and the Glenbard East High School signalized access drive.

The analyses for the unsignalized intersections determine the average control delay to vehicles at an intersection. Control delay is the elapsed time from a vehicle joining the queue at a stop sign (includes the time required to decelerate to a stop) until its departure from the stop sign and resumption of free flow speed. The methodology analyzes each intersection approach controlled by a stop sign and considers traffic volumes on all approaches and lane characteristics.

The ability of an intersection to accommodate traffic flow is expressed in terms of level of service, which is assigned a letter from A to F based on the average control delay experienced by vehicles passing through the intersection. The *Highway Capacity Manual* definitions for levels of service and the corresponding control delay for signalized intersections and unsignalized intersections are included in the Appendix of this report.

Summaries of the traffic analysis results showing the level of service and overall intersection delay (measured in seconds) for the existing conditions are presented in **Tables 1** through **3**. A discussion for the operations of the campus area follows. Summary sheets for the capacity analyses are included in the Appendix.



Table 1 CAPACITY ANALYSIS RESULTS – MAIN STREET WITH WILSON AVENUE

	Peak Hour Weekday Morning	Eastbound			Westbound			Northbound			Southbound			Overall
		L	Т	R	L	Т	R	L	Т	R	L	Т	R	Overall
		D 38.0	E 17	-	C 34.0	D 41.1		A 9.0	B 13.7		A 8.0	B 19.3		C – 20.1
Existing		C – 27.4			D-37.5			B – 13.0			B – 18.8		C - 20.1	
Existing Conditions	Weekday Afternoon	C C 29.1 22.3		C 24.7			B 11.8			B 10.6	B 19.2		C – 21.9	
		C – 25.3 C – 28					i		C – 22.5		-	B – 18.4		C - 21.9
	tes Level of Serv easured in second		- Left Turi - Through		Right Tu	rn								

Table 2

CAPACITY ANALYSIS RESULTS – MAIN STREET WITH SIGNALIZED ACCESS DRIVE

	Peak Hour	Easth	oound	North	bound	South	Overall	
	reak nour	L	R	L	Т	T R		Overall
S	Weekday Morning	D A 44.1 7.8		A 5.1	A 2.4	A –	A – 6.1	
ting ition		B –	19.2	A –	3.0			
Existing Conditions	Weekday Afternoon	D 39.8	A 6.2	A 3.5	A 4.4	A – 9.1		A - 8.0
		B-19.2		A – 4.3		11	<i>)</i> ,,,	11 0.0
Letter denotes Level of Service Delay is measured in seconds.		L – Left Turn T – Through		R – Right Turn				



Table 3 CAPACITY ANALYSIS RESULTS – UNSIGNALIZED INTERSECTIONS

	Intersection	Weekday	y Morning Hour	Weekday Afternoon Peak Hour			
		LOS	Delay	LOS	Delay		
W	ilson Avenue with East Access Drive						
•	Northbound Left Turn	В	13.4	В	13.8		
•	Northbound Right-Turn	В	10.8	В	10.8		
W	ilson Avenue with Lincoln Avenue/Mi	ddle Access l	Drive				
•	Northbound Approach			С	15.0		
•	Southbound Approach	D	32.8	В	13.3		
•	Eastbound Left Turn	А	7.8	А	7.7		
•	Westbound Left Turn	А	8.6	А	7.8		
W	ilson Avenue with West Access Drive						
•	Northbound Approach	С	21.6	С	15.2		
•	Westbound Left Turn	А	9.5	А	8.1		
W	ilson Avenue with Elizabeth Street						
•	Southbound Approach	В	14.0	В	11.8		
•	Eastbound Left Turn	А	7.6	А	7.8		
M	ain Street with South Outbound Only	Access Drive	<u>à</u>				
•	Eastbound Approach	В	12.7	С	15.7		
Μ	ain Street with Middle Outbound Only	y Access Driv	ve				
•	Eastbound Approach	D	32.6	В	12.3		
Μ	ain Street with Main Drive/North Out	bound Only	Access Drive				
•	Eastbound Approach			В	12.6		
•	Westbound Approach			В	12.1		
•	Southbound Left Turn						
M	ain Street with Inbound Only Access I	Drive					
•	Northbound Left Turn	А	0.1	А	0.2		
	OS = Level of Service lay is measured in seconds.						



Evaluation of Existing Conditions

As can be seen from the results of the capacity analyses, the study area intersections and existing access drives serving the campus generally have sufficient capacity to accommodate the traffic generated during the weekday morning and afternoon peak periods of traffic. However, as previously indicated, based on a review of drop-off/pick-up activities for the school, the following results in capacity constraints for the campus and area roadway network, particularly during the weekday afternoon dismissal peak period:

- Westbound left-turn vehicles from Wilson Avenue onto the middle access drive are required to yield to eastbound right-turn vehicles and pedestrians on the sidewalk.
- The configuration of the intersection of Wilson Avenue with Lincoln Avenue/middle access drive results in numerous conflict points between inbound and outbound vehicles as well as pedestrians on the sidewalk. During peak times of activity, when on-site queueing results in queues extending onto Wilson Avenue, the right-of-way for conflicting vehicle movements (particularly northbound/southbound through movements) are not well established for all approaches.
- During the afternoon dismissal period, queueing of passenger vehicles in the south parking lot and circulation drive extended onto Main Street resulting in only a single southbound travel lane for through traffic along the school frontage, restricting the capacity of the roadway.
- While 45 percent of all outbound traffic on Wilson Avenue during the afternoon dismissal period departs to the west, the remaining surge of outbound traffic combined with the queue of pick-up vehicles on the south wide of Wilson Avenue results in an eastbound vehicle queue that originates from the signalized intersection of Main Street with Wilson Avenue west of Lincoln Avenue. **Figure G** illustrates the peak queueing on Wilson Avenue that occurs due to the surge of outbound vehicles during the afternoon peak period.



4. Circulation and Parking Recommendations

In order to enhance the drop-off/pick-up activities for the school and limit their impact on the area roadway network, several recommendations were identified that focus on:

- Improving the circulation of traffic.
- Separating passenger vehicle and bus loading activities.
- Creating a single designated loading and staging areas for buses.
- Enhancing the available on-campus parking for staff and students.
- Reducing the need for student parking on Wilson Avenue.
- Limiting the opportunity for drop-off/pick-up queues to extend onto Main Street.

The following provides a description of identified alternatives/enhancements to the campus.

Option 1 – South Side (Exhibit 1): This option focuses on creating a dedicated drop-off/pick-up location for passenger vehicles, improving the flow along Main Street, and reducing the likelihood of vehicle queues extending into Main Street by:

- Eliminating the three outbound only curb cuts on Main Street serving the circulation driveaisle, resulting in a single inbound only curb cut.
- Outbound movements from the drive aisle will be accommodated via a new proposed connection to the southern parking lot, which will allow vehicles to exit the campus via the signalized access drive on Main Street.
- Providing dedicated visitor parking spaces along the one-way circulation drive.
- In conjunction with this option, consideration should be given to acquiring the vacant lot located along Main Street, just south of the campus, and developing the lot to provide parking.
- The additional parking should be utilized for staff parking, which will consolidate as much staff parking as possible to the south parking lot resulting in additional parking on the north side of campus to be utilized for student parking.



Option 2 – North Side: In combination with Option 1, this option focuses on creating a dedicated drop-off/pick-up area for buses, taking into consideration the proposed building expansion.

Several alternatives have been prepared for Option 2 that look at utilizing the existing infrastructure (short term), to fully redeveloping the existing parking field (long term), as follows:

- Option 2a (**Exhibit 2**) modifies the existing loop drive to accommodate the proposed building expansion and dedicates the modified loop drive to bus activity only. The existing staff and student parking lots will remain as is and can continue to function as they do under existing conditions.
- Option 2b (Exhibit 3) builds on Option 2a by minimizing the interaction between passenger vehicles and buses by closing off the west connection of the eastern parking lot to the loop drive with cones during afternoon loading activities. This would result in the middle access drive on Wilson Avenue being restricted to inbound movements only and will allow staff to exit via the eastern access drive.
- Option 2c (**Exhibit 4**) builds on Option 2b by completely isolating the eastern parking lot from the loop drive with the construction of new landscaped medians and creating a fourth Wilson Avenue curb cut that would serve the parking lot only.
- Option 2d (Exhibit 5) consists of completely consolidating/redeveloping the available area to the north/west of the school building to create a longer dedicated bus loop that will be able to accommodate all but five existing school buses along the curbside, without the need to stack buses side by side. The remaining five buses can queue on Wilson Avenue, or can be accommodated on site through the use of double stacking.
 - This bus loop would utilize the existing western access drive on Wilson Avenue (which serves the student parking lot) as the inbound access drive and will continue to utilize the existing eastern access drive on Wilson Avenue as the outbound access drive.
 - The balance of the campus located between the bus loop and Wilson Avenue would be developed to provide a single parking lot. Access to this parking lot could continue to be provided via the existing middle access driveway on Wilson Avenue, however, it is recommended that this access drive be relocated and aligned opposite Lincoln Avenue.
 - The remaining parking from the existing student parking lot should be utilized for staff or other campus vehicles such as maintenance or transport vehicles.
- If needed, the existing student parking lot can be expanded to the south/west of the tennis courts to provide additional spaces.
 - Under Option 2a/b/c, this expanded parking lot could be utilized for off-street student parking.



- With Option 2d, this expanded parking lot should only be utilized for staff/miscellaneous campus vehicles given its connection to the one-way bus loop.
- With any alternative of Option 2, should the additional parking lot be constructed on the south side of campus, staff parking should be relocated from the north side of campus to the south side of campus. The surplus of available parking that would result on the north side of campus should be utilized for additional off-street student parking.

Option 3 (Exhibit 6): This option focuses on connecting the campus parking lots, potentially limiting the volume of traffic traversing Main Street and its signalized intersections with the campus access drive and Wilson Avenue, by connecting the north and south parking lots. This connection would occur via a circulation roadway traversing the west side of the school building.

Option 4 (Exhibit 7): This option also focuses on increasing campus connectivity by creating an access roadway that connects the south parking lot to Wilson Avenue. This access roadway would traverse the south and west sides of the football field, connecting to Wilson Avenue aligned opposite Elizabeth Street. In the design of this roadway, parallel parking spaces could be provided to supplement available off-street student parking locations, reducing the number of vehicles parked on Wilson Avenue.

Table 4 summarizes the alternatives and their benefits and drawbacks.









PROJECT #: 24-093 EXHIBIT: 2





DRAWN: MD DATE: 07-25-24 PROJECT #: 24-093 EXHIBIT: 3





GLENBARD EAST OPERATIONAL STUDY LOMBARD, ILLINOIS

OPTION 2C - NORTH SIDE

PARKING

RELOCATION OF ADA AND POLICE

 DRAWN:
 MD
 CHECKED:
 BM

 DATE:
 07-25-24
 REV:
 03-12-25
 PROJECT #: 24-093 EXHIBIT: 4



SCALE: 1" = 60"



GLENBARD EAST OPERATIONAL STUDY LOMBARD, ILLINOIS

OPTION 2D - NORTH SIDE

DATE: 07-25-24 PROJECT #: 24-093 EXHIBIT: 5





GLENBARD EAST OPERATIONAL STUDY LOMBARD, ILLINOIS

OPTION 3A

DRAWN: MD DATE: 07-25-24 PROJECT #: 24-093 EXHIBIT: 6A

CHECKED: BM REV: 03-12-25







Table 4 SUMMARY OF ALTERNATIVES

JUNIN	ARY OF ALTERNATIVES	Benefits
	Description	
	• Connection of the one-way circulation roadway on the east side of the campus to the parking lot located on the south side of campus.	• Allows for on-site stacking for 42 personal vehicles.
	Provide all passenger vehicle drop-off/pick-up via Main Street	• The additional southern parking lot allows for 46 additional parking space to be provided on-site
	Provide all bus drop-off/pick-up via Wilson Avenue	• The additional northern parking lot south of the tennis courts allows for 40 additional parking space to be provided on site
	• Modify the parking lot on the north side of school building to provide a dedicated bus lane that is wide enough to accommodate side-by-side buses.	• The reconfiguration of the parking fields north of the school building, taking into consideration the building expansion, can
1	• This option can be enhanced by:	accommodate between 11 and 19 additional parking spaces.
Option 1	• Providing angled guest parking along the one-way drive-aisle	• The elimination of the three outbound curb cuts on Main Street allows for 15 angled visitor parking spaces to be provided on the
0	• Creating a second parking lot on the south side of the school building in the parcel of land located in the southwest corner of the intersection of Main	east side of the school building.
	Street with the signalized access drive.	• The provision of additional parking lots will allow for the reconfiguration of parking areas to allow for additional students
	 Creating an expansion of the student parking lot to the south side of the tennis courts. 	to park on-campus in lieu of Wilson Avenue
	 Fully isolating the staff/visitor parking on the north side of the school building and creating its own curb cut. 	• Reduces the number of curb cuts on Main Street, particularly the curb cut located 100 feet north of the signalized access drive.
	 Restricting the Wilson Avenue access drive aligned opposite Lincoln Avenue to one-way in only during afternoon dismissal period. 	• Consolidates all of the bus activity into one location, with approximately 25 buses able to be stacked on site through double stacking.
2	• Circulation roadway connecting the student parking lot with the parking lot on the south side of the school building.	• Will allow for alternative ingress and egress for the student parking lot and the south parking lot.
Option	• This circulation roadway should be gated and only utilized on-school days and sporting events to prevent regional cut through of local traffic.	• Will reduce the volume of traffic traversing the intersection of Wilson Avenue with Main Street
	• Circulation roadway connecting the parking lot on the south side of the school building to Wilson Avenue aligned opposite Elizabeth Street.	• Will allow for alternative ingress and egress for the south parking lot.
Option 3	• This circulation roadway should be gated and only utilized on school days to prevent regional cut through of local traffic.	• Can be utilized for off-street parallel parking for students if restricted to one-way or widened
		• Can be utilized for bus staging for visiting teams during sporting events

Drawbacks

- Increases the volume of passenger vehicle traffic utilizing the parking lot on the south side of the school building.
- Requires all outbound passenger vehicle pick-up and drop off to utilize the signalized access drive on Main Street.

- Increases the volume of traffic traversing the south campus parking lot and its signalized access drive with Main Street
- Results in a modification to the existing sports fields located to the west of the school building
- Increases the volume of traffic traversing the south campus parking lot and its signalized access drive with Main Street
- Creates a fourth leg to the signalized intersection of Wilson Avenue with Elizabeth Street



5. Evaluation of Drop-off/Pick-up Alternatives

To determine the viability of Options 1 and 2, the existing drop-off/pick-up traffic was reassigned to the area roadway network and capacity analyses were conducted to determine the impact on Main Street. Furthermore, stacking exhibits were prepared to determine the projected off-street queueing spaces for vehicles and the existing and projected parking inventories were summarized to document the net change in available campus parking.

Total Projected Traffic Volumes

The estimated weekday morning and weekday afternoon peak hour traffic volumes that are generated by Glenbard East High School were reassigned to the roadway system taking into consideration the separation of drop-off/pick-up activities for passenger vehicles and buses and consolidation of access driveways on Main Street. **Figure 5** illustrates the traffic reassignment. The traffic reassignment was added to the existing traffic volumes to determine the total projected traffic volumes as illustrated in **Figure 6**.

Capacity Analysis of Projected Conditions

Roadway and adjacent or nearby intersection analyses were performed for the weekday morning and weekday afternoon peak hours for the projected traffic volumes. Summaries of the traffic analysis results showing the level of service and overall intersection delay (measured in seconds) for the projected conditions of the signalized intersections along Main Street are presented in **Tables 5** and 6, respectively. A discussion for the operations of the intersections follows. Summary sheets for the capacity analyses are included in the Appendix.

As can be seen from Table 5, the intersection of Main Street with Wilson Avenue overall and all of the approaches are projected to continue operating at acceptable levels of service during the morning arrival and afternoon dismissal periods. The eastbound approach is anticipated to experience an approximately 25 percent reduction in delays, which is primarily a result of the reassignment of additional passenger vehicles to the Main Street drop-off/pick-up area. As can be seen from Table 6, the intersection of Main Street with the access drive serving Glenbard East High School overall and all of the approaches are projected to continue operating at acceptable levels of service during the morning arrival and afternoon dismissal periods. The northbound left-turn movement and southbound approach are projected to operate at an acceptable LOS D during the morning peak hour (LOS A under existing conditions), however, this level of service is a result of the additional green time allocated to the eastbound approach to accommodate the additional demand with the reassignment of passenger vehicles.

Overall, the signalized intersections along Main Street will be adequate in accommodating the reassignment of traffic volumes to the campus intersections. However, these signals should be monitored once projected traffic volumes are realized to determine if signal timing adjustments are required.





Job No: 24-093



Table 5 CAPACITY ANALYSIS RESULTS – MAIN STREET WITH WILSON AVENUE

	Peak Hour Weekday Morning Weekday Afternoon	Eastbound			Westbound			Northbound			Southbound			Overall
		\mathbf{L}	Т	R	L	Т	R	L	Т	R	L	Т	R	Overall
		C 30.4	E 16	-	D 52.8			B B 16.0 11.0		A 8.1		C .1	В	
Existing		C – 20.1			D-43.9			B-12.0			C – 21.5			19.6
Existing Conditions		C C 29.4 20.5		C 28.8			B 15.7			A 9.9	B 18.5		С	
		C – 24.3 C – 27.7					,	C – 27.6			B-17.7			23.3
	tes Level of Serv asured in second		- Left Turi - Through		Right Tu	rn								

Table 6

CAPACITY ANALYSIS RESULTS – MAIN STREET WITH SIGNALIZED ACCESS DRIVE

	Peak Hour	Easth	oound	North	bound	South	Overall	
	reak nour	L	R	L	Т	Т	R	Overaii
S	Weekday Morning	C A 29.2 2.0		D 35.5	B 18.9	D – 39.8		С
ting ition		B –	16.3	C –	22.6		26.6	
Existing Conditions	Weekday Afternoon	D 37.7	A 4.3	A 5.5	A 7.4	B –	В	
		C – 21.3		A –	7.4	D	10.0	13.0
	otes Level of Service easured in seconds.	L – Left Turn T – Through		R – Right Turn				


Proposed Drop-Off and Pick-Up Stacking

Stacking exhibits were prepared to estimate the number of vehicles able to stack in each of the designated areas. **Exhibits 8** through **11** illustrate the stacking for Option 1, Option 2a, Option 2c, and Option 2d, respectively. The following provides a summary of findings for the stacking exhibits.

- Option 1 provides stacking for approximately 42 passenger vehicles before vehicle queues extend onto Main Street. As observed, the peak queue of passenger vehicles at the Main Street access drives was approximately 34 vehicles (of which 10 vehicles extended onto Main Street). While the proposed 42-vehicle stacking on the east/south side of campus will accommodate this queue, the modifications to the north parking lot will reorient additional passenger vehicles to the proposed Main Street access drives. Should queues continue to extend onto Main Street, consideration should be given to the following:
 - Relocating the miscellaneous campus vehicles parked along the southwest side of the school building to the north parking lot and advancing the start of the passenger vehicle queue further to the west.
 - Widening the inbound (westbound) lanes at the signalized access drive on Main Street to provide two inbound travel lanes. This widening can occur via widening the drive aisle to the north or by reducing the width of the center channelizing island.
- Option 2a provides stacking of 21 buses on campus through the utilization of double stacking buses. As previously indicated, a total of 25 buses serve the campus and as such, four buses will be required to stage on Wilson Avenue.
- Option 2c, which isolates the staff parking lot, expands the available stacking for buses on campus to 25 with the use of double stacking and as a result, no buses will be staged on Wilson Avenue.
- Option 2d allows a total of 20 buses to stack in a single file on campus, with 17 buses staged along the walkway connecting to the school building. While this would result in five buses staging on Wilson Avenue, it eliminates the need for students that take buses to walk into the drive aisle to board or depart a bus. Should it be desired that all buses stage on campus, these five buses can be accommodated through double stacking at the rear of the line, which can then advance to a single file line of eight buses, following the dismissal of the first 17 buses.

Summary of Existing and Projected Parking Inventories

Under existing conditions, the campus provides a total of 412 parking spaces consisting of 267 staff parking spaces, 113 student parking spaces, six visitor parking spaces, and 26 miscellaneous parking spaces (Lombard Police Department, transport buses, maintenance personnel, and driver's education vehicles).







GLENBARD EAST OPERATIONAL STUDY LOMBARD, ILLINOIS

OPTION 2A - BUS STACKING

DRAWN: MD CHECKED: BM DATE: 07-25-24 REV: 03-12-25 PROJECT #: 24-093 EXHIBIT: 9





GLENBARD EAST OPERATIONAL STUDY LOMBARD, ILLINOIS

OPTION 2C - BUS STACKING

DRAWN: MD DATE: 07-25-24 PROJECT #: 24-093 EXHIBIT: 10

CHECKED: BM REV: 03-12-25





GLENBARD EAST OPERATIONAL STUDY LOMBARD, ILLINOIS

OPTION 2D - BUS STACKING

DRAWN: MD DATE: 07-25-24 PROJECT #: 24-093 EXHIBIT: 11



Of the 412 total parking spaces, 162 of the parking spaces are located on the south side of the school building with the remainder located along the north side of the school building. Additionally there are 51 on-street parking spaces along Wilson Avenue that are utilized for student parking.

With the implementation of Option 1 (without acquiring additional parcels) a total of 15 visitor parking spaces will be added to the campus. By acquiring the parcel of land to the south of the campus, approximately 46 additional staff parking spaces can be provided. With the full implementation of Option 1, 61 parking spaces become available on the north side of campus, which can accommodate the 51 on-street parking spaces being utilized by students.

The implementation of Option 2a, 2b, and 2c, taking into consideration the proposed building expansion, results in an additional 9 parking spaces. Option 2d results in a total of 252 parking spaces, which is a net increase of 19 parking spaces over existing conditions.

If additional on-campus parking is desired or should the parcel of land located on the west side of Main Street south of the campus not be available, the parcel of land located to the south of the tennis courts can be developed to provide an additional 40 parking spaces. When taking into consideration the 9 parking spaces created as a function of Options 2a, 2b, and 2c and the 15 visitor parking spaces on the east side of the school building as part of Option 1, up to 64 additional parking spaces can be provided on campus. This additional parking can accommodate the 51 on-street student parking spaces.

Overall, with the implementation of Option 1 (inclusive of the parcel of land south of the campus) and Option 2a/b/c, a minimum of 70 additional parking spaces can be provided on campus. With the implementation of Option 1 (inclusive of the parcel of land south of the campus), Option 2d, and inclusion of the parcel of land to the south of the tennis courts, up to 120 additional parking spaces can be provided on campus.

Summary of Findings

As can be seen from the preceding evaluation, the recommended Options 1 and 2 to enhance pickup/drop-off operations will be beneficial for the campus as they will:

- Separate student loading for passenger vehicles and buses
- Create a dedicated loading and staging area for buses
- Increase the availability of on-campus parking for staff, visitors, and students, which will allow for parking on Wilson Avenue to be restricted.
- Increase the available on-campus storage for passenger vehicles, reducing the likelihood of vehicle queues extending onto Main Street
- Improve the flow of traffic along Main Street through the elimination of curb cuts.



Appendix

Traffic Count Summary Sheets Drop-Off/Pick-Up Peak Queue Aerial Images Level of Service Criteria Capacity Analysis Summary Sheets

Traffic Count Summary Sheets



Rosemont, Illinois, United States 60018 (847)518-9990 mmendoza@kloainc.com

Count Name: Wilson Avenue with Main Street TMC Site Code: Start Date: 04/04/2024 Page No: 1

																									1
				Avenue bound					Wilson Westl	Avenue bound						Street bound					Main South	Street bound			
Start Time	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	Int. Total
6:30 AM	0	6	5	9	0	20	0	4	11	5	0	20	0	11	53	0	1	64	0	9	73	15	0	97	201
6:45 AM	0	14	9	15	0	38	0	11	14	5	0	30	0	16	58	1	3	75	0	11	85	18	0	114	257
Hourly Total	0	20	14	24	0	58	0	15	25	10	0	50	0	27	111	1	4	139	0	20	158	33	0	211	458
7:00 AM	0	20	2	27	1	49	0	17	9	10	0	36	1	13	57	3	1	74	0	10	124	16	0	150	309
7:15 AM	0	50	9	42	3	101	0	33	22	6	3	61	0	30	108	10	8	148	0	12	163	38	1	213	523
7:30 AM	0	23	8	28	0	59	0	23	13	11	0	47	0	15	139	14	2	168	0	9	207	13	0	229	503
7:45 AM	0	6	7	7	0	20	0	18	12	10	0	40	0	14	189	23	0	226	0	13	185	8	0	206	492
Hourly Total	0	99	26	104	4	229	0	91	56	37	3	184	1	72	493	50	11	616	0	44	679	75	1	798	1827
8:00 AM	0	3	6	4	0	13	0	16	12	13	0	41	0	9	120	11	0	140	0	12	144	6	0	162	356
8:15 AM	0	19	5	10	0	34	0	16	13	12	0	41	0	8	131	11	0	150	0	5	168	9	0	182	407
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	0	22	11	14	0	47	0	32	25	25	0	82	0	17	251	22	0	290	0	17	312	15	0	344	763
1:30 PM	0	8	9	6	0	23	0	13	6	10	0	29	0	6	110	13	2	129	0	7	118	10	0	135	316
1:45 PM	0	15	7	11	0	33	0	13	10	15	0	38	0	8	126	5	0	139	0	14	125	2	0	141	351
Hourly Total	0	23	16	17	0	56	0	26	16	25	0	67	0	14	236	18	2	268	0	21	243	12	0	276	667
2:00 PM	0	1	11	8	2	20	0	12	3	19	0	34	0	6	150	8	0	164	0	9	123	8	0	140	358
2:15 PM	0	2	10	9	0	21	0	16	5	13	0	34	0	14	152	15	0	181	0	8	133	8	0	149	385
2:30 PM	0	46	10	41	4	97	0	20	4	16	4	40	0	15	166	14	46	195	0	16	148	12	0	176	508
2:45 PM	0	25	17	25	1	67	0	15	7	14	0	36	0	11	208	21	1	240	0	10	122	16	0	148	491
Hourly Total	0	74	48	83	7	205	0	63	19	62	4	144	0	46	676	58	47	780	0	43	526	44	0	613	1742
3:00 PM	0	11	11	12	0	34	1	12	17	16	1	46	0	13	182	12	1	207	0	11	151	6	0	168	455
3:15 PM	0	7	9	12	4	28	0	26	22	11	1	59	0	12	169	9	0	190	0	25	217	8	2	250	527
Grand Total	0	256	135	266	15	657	1	265	180	186	9	632	1	201	2118	170	65	2490	0	181	2286	193	3	2660	6439
Approach %	0.0	39.0	20.5	40.5	-	-	0.2	41.9	28.5	29.4	-	-	0.0	8.1	85.1	6.8	-	-	0.0	6.8	85.9	7.3	-	-	-
Total %	0.0	4.0	2.1	4.1	-	10.2	0.0	4.1	2.8	2.9	-	9.8	0.0	3.1	32.9	2.6	-	38.7	0.0	2.8	35.5	3.0	-	41.3	-
Lights	0	243	132	257	-	632	1	259	176	177	-	613	1	196	2062	162	-	2421	0	173	2221	186	-	2580	6246
% Lights	-	94.9	97.8	96.6	-	96.2	100.0	97.7	97.8	95.2	-	97.0	100.0	97.5	97.4	95.3	-	97.2	-	95.6	97.2	96.4	-	97.0	97.0
Buses	0	13	3	9	-	25	0	5	4	4	-	13	0	5	27	5	-	37	0	2	38	3	-	43	118
% Buses	-	5.1	2.2	3.4	-	3.8	0.0	1.9	2.2	2.2	-	2.1	0.0	2.5	1.3	2.9	-	1.5	-	1.1	1.7	1.6	-	1.6	1.8
Single-Unit Trucks	0	0	0	0	-	0	0	1	0	4	-	5	0	0	24	3	-	27	0	3	25	1	-	29	61
% Single-Unit Trucks	-	0.0	0.0	0.0	-	0.0	0.0	0.4	0.0	2.2	-	0.8	0.0	0.0	1.1	1.8	-	1.1	-	1.7	1.1	0.5	-	1.1	0.9
Articulated Trucks	0	0	0	0	-	0	0	0	0	1	-	1	0	0	4	0	-	4	0	3	2	3	-	8	13
% Articulated Trucks	-	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	0.5	-	0.2	0.0	0.0	0.2	0.0	-	0.2	-	1.7	0.1	1.6	-	0.3	0.2
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	1	0	-	1	0	0	0	0	-	0	1

% Bicycles on Road	-	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	-	-	15	-	-	-	-	-	9	-	-	-	-	-	65	-	-	-	-	-	3	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-



Rosemont, Illinois, United States 60018 (847)518-9990 mmendoza@kloainc.com

Count Name: Wilson Avenue with Main Street TMC Site Code: Start Date: 04/04/2024 Page No: 3

								Tun	iiriy iv	IOVEII		Cari	JUUI	Jala	0.45	AIVI)			i						
			Wilson	Avenue					Wilson	Avenue					Main	Street					Main	Street			
			East	bound					West	bound					North	bound					South	bound			
Start Time	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	Int. Total
6:45 AM	0	14	9	15	0	38	0	11	14	5	0	30	0	16	58	1	3	75	0	11	85	18	0	114	257
7:00 AM	0	20	2	27	1	49	0	17	9	10	0	36	1	13	57	3	1	74	0	10	124	16	0	150	309
7:15 AM	0	50	9	42	3	101	0	33	22	6	3	61	0	30	108	10	8	148	0	12	163	38	1	213	523
7:30 AM	0	23	8	28	0	59	0	23	13	11	0	47	0	15	139	14	2	168	0	9	207	13	0	229	503
Total	0	107	28	112	4	247	0	84	58	32	3	174	1	74	362	28	14	465	0	42	579	85	1	706	1592
Approach %	0.0	43.3	11.3	45.3	-	-	0.0	48.3	33.3	18.4	-	-	0.2	15.9	77.8	6.0	-	-	0.0	5.9	82.0	12.0	-	-	-
Total %	0.0	6.7	1.8	7.0	-	15.5	0.0	5.3	3.6	2.0	-	10.9	0.1	4.6	22.7	1.8	-	29.2	0.0	2.6	36.4	5.3	-	44.3	-
PHF	0.000	0.535	0.778	0.667	-	0.611	0.000	0.636	0.659	0.727	-	0.713	0.250	0.617	0.651	0.500	-	0.692	0.000	0.875	0.699	0.559	-	0.771	0.761
Lights	0	98	26	106	-	230	0	80	56	28	-	164	1	72	353	27	-	453	0	41	564	82	-	687	1534
% Lights	-	91.6	92.9	94.6	-	93.1	-	95.2	96.6	87.5	-	94.3	100.0	97.3	97.5	96.4	-	97.4	-	97.6	97.4	96.5	-	97.3	96.4
Buses	0	9	2	6	-	17	0	4	2	2	-	8	0	2	4	1	-	7	0	0	9	3	-	12	44
% Buses	-	8.4	7.1	5.4	-	6.9	-	4.8	3.4	6.3	-	4.6	0.0	2.7	1.1	3.6	-	1.5	-	0.0	1.6	3.5	-	1.7	2.8
Single-Unit Trucks	0	0	0	0	-	0	0	0	0	2	-	2	0	0	4	0	-	4	0	1	6	0	-	7	13
% Single-Unit Trucks	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	6.3	-	1.1	0.0	0.0	1.1	0.0	-	0.9	-	2.4	1.0	0.0	-	1.0	0.8
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Articulated Trucks	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.0
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	1	0	-	1	0	0	0	0	-	0	1
% Bicycles on Road	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.0	0.0	0.3	0.0	-	0.2	-	0.0	0.0	0.0	-	0.0	0.1
Pedestrians	-	-	-	-	4	-	-	-	-	-	3	-	-	-	-	-	14	-	-	-	-	-	1	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-



Rosemont, Illinois, United States 60018 (847)518-9990 mmendoza@kloainc.com

Count Name: Wilson Avenue with Main Street TMC Site Code: Start Date: 04/04/2024 Page No: 4

				Avenue						Avenue bound					Main	Street bound					Main South				
Start Time	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	Int. Total
2:30 PM	0	46	10	41	4	97	0	20	4	16	4	40	0	15	166	14	46	195	0	16	148	12	0	176	508
2:45 PM	0	25	17	25	1	67	0	15	7	14	0	36	0	11	208	21	1	240	0	10	122	16	0	148	491
3:00 PM	0	11	11	12	0	34	1	12	17	16	1	46	0	13	182	12	1	207	0	11	151	6	0	168	455
3:15 PM	0	7	9	12	4	28	0	26	22	11	1	59	0	12	169	9	0	190	0	25	217	8	2	250	527
Total	0	89	47	90	9	226	1	73	50	57	6	181	0	51	725	56	48	832	0	62	638	42	2	742	1981
Approach %	0.0	39.4	20.8	39.8	-	-	0.6	40.3	27.6	31.5	-	-	0.0	6.1	87.1	6.7	-	-	0.0	8.4	86.0	5.7	-	-	-
Total %	0.0	4.5	2.4	4.5	-	11.4	0.1	3.7	2.5	2.9	-	9.1	0.0	2.6	36.6	2.8	-	42.0	0.0	3.1	32.2	2.1	-	37.5	-
PHF	0.000	0.484	0.691	0.549	-	0.582	0.250	0.702	0.568	0.891	-	0.767	0.000	0.850	0.871	0.667	-	0.867	0.000	0.620	0.735	0.656	-	0.742	0.940
Lights	0	87	47	88	-	222	1	72	48	55	-	176	0	51	701	54	-	806	0	59	624	39	-	722	1926
% Lights	-	97.8	100.0	97.8	-	98.2	100.0	98.6	96.0	96.5	-	97.2	-	100.0	96.7	96.4	-	96.9	-	95.2	97.8	92.9	-	97.3	97.2
Buses	0	2	0	2	-	4	0	1	2	1	-	4	0	0	16	2	-	18	0	2	8	0	-	10	36
% Buses	-	2.2	0.0	2.2	-	1.8	0.0	1.4	4.0	1.8	-	2.2	-	0.0	2.2	3.6	-	2.2	-	3.2	1.3	0.0	-	1.3	1.8
Single-Unit Trucks	0	0	0	0	-	0	0	0	0	1	-	1	0	0	7	0	-	7	0	1	6	0	-	7	15
% Single-Unit Trucks	-	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	1.8	-	0.6	-	0.0	1.0	0.0	-	0.8	-	1.6	0.9	0.0	-	0.9	0.8
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	1	0	-	1	0	0	0	3	-	3	4
% Articulated Trucks	-	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0	-	0.0	-	0.0	0.1	0.0	-	0.1	-	0.0	0.0	7.1	-	0.4	0.2
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	-	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	-	-	9	-	-	-	-	-	6	-	-	-	-	-	48	-	-	-	-	-	2	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-



Rosemont, Illinois, United States 60018 (847)518-9990 mmendoza@kloainc.com

Count Name: Main Street with High School North Access Drive TMC Site Code: Start Date: 04/04/2024 Page No: 1

				Street bound						Drive bound					-	Access Dri bound	ve				Main South				
Start Time	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	Int. Total
6:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
6:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Hourly Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Hourly Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	0	1	0	1	0	0	28	0	0	28	0	0	30	0	0	30	59
8:15 AM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	154	0	0	154	0	0	199	0	0	199	353
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	0	0	0	0	1	0	0	0	0	1	0	1	0	0	182	0	0	182	0	0	229	0	0	229	412
1:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	142	0	0	142	0	0	148	0	0	148	290
1:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	143	0	0	143	0	0	148	2	0	150	293
Hourly Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	285	0	0	285	0	0	296	2	0	298	583
2:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	174	0	0	174	0	0	165	5	0	170	344
2:15 PM	0	0	0	0	0	0	0	1	0	0	0	1	0	0	194	0	0	194	0	0	156	5	0	161	356
2:30 PM	0	0	0	8	3	8	0	0	0	5	2	5	0	1	199	0	0	200	0	0	214	3	0	217	430
2:45 PM	0	0	0	2	0	2	0	0	0	2	0	2	0	2	243	0	0	245	0	0	162	1	0	163	412
Hourly Total	0	0	0	10	3	10	0	1	0	7	2	8	0	3	810	0	0	813	0	0	697	14	0	711	1542
3:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	214	1	0	216	0	0	170	1	0	171	387
3:15 PM	0	0	0	0	2	0	0	0	0	1	1	1	0	0	190	0	0	190	0	0	260	1	0	261	452
Grand Total	0	0	0	10	6	10	0	1	0	9	3	10	0	4	1681	1	0	1686	0	0	1652	18	0	1670	3376
Approach %	0.0	0.0	0.0	100.0	-	-	0.0	10.0	0.0	90.0	-	-	0.0	0.2	99.7	0.1	-	-	0.0	0.0	98.9	1.1	-	-	-
Total %	0.0	0.0	0.0	0.3	-	0.3	0.0	0.0	0.0	0.3	-	0.3	0.0	0.1	49.8	0.0	-	49.9	0.0	0.0	48.9	0.5	-	49.5	-
Lights	0	0	0	10	-	10	0	1	0	9	-	10	0	4	1627	1	-	1632	0	0	1618	8	-	1626	3278
% Lights	-	-	-	100.0	-	100.0	-	100.0	-	100.0	-	100.0	-	100.0	96.8	100.0	-	96.8	-	-	97.9	44.4	-	97.4	97.1
Buses	0	0	0	0	-	0	0	0	0	0	-	0	0	0	29	0	-	29	0	0	15	10	-	25	54
% Buses	-	-	-	0.0	-	0.0	-	0.0	-	0.0	-	0.0	-	0.0	1.7	0.0	-	1.7	-	-	0.9	55.6	-	1.5	1.6
Single-Unit Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	25	0	-	25	0	0	19	0	-	19	44
% Single-Unit Trucks	-	-	-	0.0	-	0.0	-	0.0	-	0.0	-	0.0	-	0.0	1.5	0.0	-	1.5	-	-	1.2	0.0	-	1.1	1.3
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Articulated Trucks	-	-	-	0.0	-	0.0	-	0.0	-	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	-	0.0	0.0	-	0.0	0.0
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0

% Bicycles on Road	-	-	-	0.0	-	0.0	-	0.0	-	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	-	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	-	-	6	-	-	-	-	-	3	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	-	-	-	-	-	-	-	-	-



Rosemont, Illinois, United States 60018 (847)518-9990 mmendoza@kloainc.com

Count Name: Main Street with High School North Access Drive TMC Site Code: Start Date: 04/04/2024 Page No: 3

				Street bound					Main Westl	Drive				Н	igh School	Access Dri bound	ve				Main South				
Start Time	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	Int. Total
6:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Approach %	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	-
Total %	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0	-	0.0	-
PHF	0.000	0.000	0.000	0.000	-	0.000	0.000	0.000	0.000	0.000	-	0.000	0.000	0.000	0.000	0.000	-	0.000	0.000	0.000	0.000	0.000	-	0.000	0.000
Lights	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Lights	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Buses	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Buses	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Single-Unit Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Single-Unit Trucks	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Articulated Trucks	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



Rosemont, Illinois, United States 60018 (847)518-9990 mmendoza@kloainc.com

Count Name: Main Street with High School North Access Drive TMC Site Code: Start Date: 04/04/2024 Page No: 4

			Main	Street					-	Drive				Hi	ah School	Access Dri	Ve				Main	Street			1
				bound					West						North							bound			
Start Time						Ann						App						App						App	
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	Int. Total
2:30 PM	0	0	0	8	3	8	0	0	0	5	2	5	0	1	199	0	0	200	0	0	214	3	0	217	430
2:45 PM	0	0	0	2	0	2	0	0	0	2	0	2	0	2	243	0	0	245	0	0	162	1	0	163	412
3:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	214	1	0	216	0	0	170	1	0	171	387
3:15 PM	0	0	0	0	2	0	0	0	0	1	1	1	0	0	190	0	0	190	0	0	260	1	0	261	452
Total	0	0	0	10	5	10	0	0	0	8	3	8	0	4	846	1	0	851	0	0	806	6	0	812	1681
Approach %	0.0	0.0	0.0	100.0	-	-	0.0	0.0	0.0	100.0	-	-	0.0	0.5	99.4	0.1	-	-	0.0	0.0	99.3	0.7	-	-	-
Total %	0.0	0.0	0.0	0.6	-	0.6	0.0	0.0	0.0	0.5	-	0.5	0.0	0.2	50.3	0.1	-	50.6	0.0	0.0	47.9	0.4	-	48.3	-
PHF	0.000	0.000	0.000	0.313	-	0.313	0.000	0.000	0.000	0.400	-	0.400	0.000	0.500	0.870	0.250	-	0.868	0.000	0.000	0.775	0.500	-	0.778	0.930
Lights	0	0	0	10	-	10	0	0	0	8	-	8	0	4	813	1	-	818	0	0	792	5	-	797	1633
% Lights	-	-	-	100.0	-	100.0	-	-	-	100.0	-	100.0	-	100.0	96.1	100.0	-	96.1	-	-	98.3	83.3	-	98.2	97.1
Buses	0	0	0	0	-	0	0	0	0	0	-	0	0	0	21	0	-	21	0	0	8	1	-	9	30
% Buses	-	-	-	0.0	-	0.0	-	-	-	0.0	-	0.0	-	0.0	2.5	0.0	-	2.5	-	-	1.0	16.7	-	1.1	1.8
Single-Unit Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	12	0	-	12	0	0	6	0	-	6	18
% Single-Unit Trucks	-	-	-	0.0	-	0.0	-	-	-	0.0	-	0.0	-	0.0	1.4	0.0	-	1.4	-	-	0.7	0.0	-	0.7	1.1
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Articulated Trucks	-	-	-	0.0	-	0.0	-	-	-	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	-	0.0	0.0	-	0.0	0.0
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	-	-	-	0.0	-	0.0	-	-	-	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	-	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	-	-	5	-	-	-	-	-	3	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	-	-	-	-	-	-	-	-	-



Rosemont, Illinois, United States 60018 (847)518-9990 mmendoza@kloainc.com

Count Name: Main Street with High School North Access Drive TMC Site Code: Start Date: 04/04/2024 Page No: 1

Start Time U-Turm 6:30 AM 0 6:45 AM 0 Hourly Total 0 7:00 AM 0 7:15 AM 0 7:30 AM 0 7:45 AM 0 Hourly Total 0 0 7:45 AM 0 8:00 AM 0 0	Left 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Thru 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Right 0	Peds 0 0 0 0 0 1 1 2 0 0 0 0 0 0 0 0 0 0 0 0	App. Total 0 0 0 0 0 0 0 0 0 0 0 0	U-Turn 0 0 0 0 0 0 0 0 0 0 0 0 0	Left 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Thru 0 0 0 0 0 0 0 0	Right 0 0 0 0 0 0 0	Peds 1 0 1 0 0 0 0	App. Total 0 0 0 0 0	U-Turn 0 0 0 0	Left 0 2 2 12	Thru 66 79 145	Right 0 0 0	Peds 0 0 0 0	App. Total 66 81 147	U-Turn 0 0 0	Left 0 0 0	Thru 82 120 202	Right 0 1 1	Peds 0 0 0 0 0	App. Total 82 121 203	Int. Total 148 202
6:45 AM 0 Hourly Total 0 7:00 AM 0 7:15 AM 0 7:30 AM 0 7:45 AM 0 Hourly Total 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 -	0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0	0 0 0 1 1 2 0	0 0 0 0 0 0 0	0 0 0 0 0 0	0 0 0 0 0 0	0 0 0 0 0	0 0 0 0	1	0 0 0	0	2	79	0	0	81	0	0	120	1 1	0	121 203	202
Hourly Total 0 7:00 AM 0 7:15 AM 0 7:30 AM 0 7:45 AM 0 Hourly Total 0 8:00 AM 0	0 0 0 0 0 0 0 0 0 -	0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0	0 0 0 1 1 2 0	0 0 0 0 0 0	0 0 0 0 0	0 0 0 0 0	0 0 0 0	0 0 0 0	1	0	0	2					-			1	0	203	
7:00 AM 0 7:15 AM 0 7:30 AM 0 7:45 AM 0 Hourly Total 0 8:00 AM 0	0 0 0 0 0 0 0 0 -	0 0 0 0 0 0 0	0 0 0 0 0 0	0 0 1 1 2 0	0 0 0 0 0	0 0 0 0	0 0 0 0	0 0 0	0		0			145	0	0	147	0	0	202				250
7:15 AM 0 7:30 AM 0 7:45 AM 0 Hourly Total 0 8:00 AM 0	0 0 0 0 0 0 -	0 0 0 0 0 0	0 0 0 0 0	0 1 1 2 0	0 0 0 0	0 0 0	0 0 0 0	0	0		· · · ·	0	10									0		350
7:30 AM 0 7:45 AM 0 Hourly Total 0 8:00 AM 0	0 0 0 0 0 -	0 0 0 0 0	0 0 0 0	1 1 2 0	0 0 0	0	0 0	0		0	0		12	82	0	0	94	0	0	141	24	0	165	259
7:45 AM 0 Hourly Total 0 8:00 AM 0	0 0 0 0 -	0 0 0 0	0 0 0	1 2 0	0	0	0		0		~	0	22	180	0	0	202	0	0	205	65	0	270	472
Hourly Total 0 8:00 AM 0	0 0 0 -	0 0 0	0	2	0			0		1	0	0	5	173	0	0	178	0	0	247	3	0	250	428
8:00 AM 0	0 0 -	0	0	0		0	0		0	0	0	0	0	222	0	0	222	0	0	221	0	0	221	443
	0	0	-		0		0	0	0	1	0	0	39	657	0	0	696	0	0	814	92	0	906	1602
0.45.444	-	·	0			0	0	0	0	0	0	0	0	112	0	0	112	0	0	139	0	0	139	251
8:15 AM 0		-		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
*** BREAK *** -	0		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total 0	0	0	0	0	0	0	0	0	0	0	0	0	0	112	0	0	112	0	0	139	0	0	139	251
1:30 PM 0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:45 PM 0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Hourly Total 0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2:00 PM 0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2:15 PM 0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2:30 PM 0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2:45 PM 0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Hourly Total 0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3:00 PM 0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3:15 PM 0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Grand Total 0	0	0	0	2	0	0	0	0	0	2	0	0	41	914	0	0	955	0	0	1155	93	0	1248	2203
Approach % 0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	4.3	95.7	0.0	-	-	0.0	0.0	92.5	7.5	-	-	-
Total % 0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0	-	0.0	0.0	1.9	41.5	0.0	-	43.3	0.0	0.0	52.4	4.2	-	56.7	-
Lights 0	0	0	0	-	0	0	0	0	0	-	0	0	41	893	0	-	934	0	0	1123	91	-	1214	2148
% Lights -	-	-	-	-	-	-	-	-	-	-	-	-	100.0	97.7	-	-	97.8	-	-	97.2	97.8	-	97.3	97.5
Buses 0	0	0	0	-	0	0	0	0	0	-	0	0	0	11	0	-	11	0	0	23	2	-	25	36
% Buses -	-	-	-	-	-	-	-	-	-	-	-	-	0.0	1.2	-	-	1.2	-	-	2.0	2.2	-	2.0	1.6
Single-Unit Trucks 0	0	0	0	-	0	0	0	0	0	-	0	0	0	9	0	-	9	0	0	8	0	-	8	17
% Single-Unit Trucks	-	-	-	-	-	-	-	-	-	-	-	-	0.0	1.0	-	-	0.9	-	-	0.7	0.0	-	0.6	0.8
Articulated Trucks 0	0	0	0	-	0	0	0	0	0	-	0	0	0	1	0	-	1	0	0	1	0	-	1	2
% Articulated Trucks	-	-	-	-	-	-	-	-	-	-	-	-	0.0	0.1	-	-	0.1	-	-	0.1	0.0	-	0.1	0.1
Bicycles on Road 0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0

% Bicycles on Road	-	-	-	-	-	-	-	-	-	-	-	-	-	0.0	0.0	-	-	0.0	-	-	0.0	0.0	-	0.0	0.0
Pedestrians	- 1	-	-	-	2	-	-	-	-	-	2	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	-	-	-	-	-	-	-	-	-



Rosemont, Illinois, United States 60018 (847)518-9990 mmendoza@kloainc.com

Count Name: Main Street with High School North Access Drive TMC Site Code: Start Date: 04/04/2024 Page No: 3

Main Street Main Drive High School Access Drive Eastbound Westbound Northbound								
Eastbound Westbound Northbound		1			n Street			
				Sout	hbound			
Start Time U-Turn Left Thru Right Peds App. U-Turn Left Thru Right Peds App. U-Turn Left Thru Right Peds	ds App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	Int. Total
6:45 AM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	81	0	0	120	1	0	121	202
7:00 AM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 12 82 0 0	94	0	0	141	24	0	165	259
7:15 AM 0 0 0 0 0 0 0 0 0 0 0 22 180 0 0	202	0	0	205	65	0	270	472
7:30 AM 0 0 0 0 1 0 0 0 0 1 0 0 0 1 0 0 5 173 0 0	178	0	0	247	3	0	250	428
Total 0 0 0 1 0 0 0 0 1 0 0 0 1 0 0 41 514 0 0	555	0	0	713	93	0	806	1361
Approach % 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 7.4 92.6 0.0 -	-	0.0	0.0	88.5	11.5	-	-	-
Total % 0.0 0.0 0.0 0.0 - 0.0 0.0 0.0 0.0 - 0.0 0.0	40.8	0.0	0.0	52.4	6.8	-	59.2	-
PHF 0.000 0.000 0.000 0.000 - 0.000 0.000 0.000 0.000 0.000 - 0.000 0.000 0.000 - 0.000 0.466 0.714 0.000 -	0.687	0.000	0.000	0.722	0.358	-	0.746	0.721
Lights 0 0 0 0 - 0 0 0 0 - 0 0 41 504 0 -	545	0	0	693	91	-	784	1329
<u>Kights</u> 100.0 98.1	98.2	-	-	97.2	97.8	-	97.3	97.6
Buses 0 0 0 0 - 0 0 0 0 - 0 0 0 - 0 0 0 - 0 0 0 - 0	6	0	0	15	2	-	17	23
<u>% Buses</u> 0.0 1.2	1.1	-	-	2.1	2.2	-	2.1	1.7
Single-Unit Trucks 0 0 0 0 - 0 0 0 0 0 - 0 0 0 4 0 -	4	0	0	5	0	-	5	9
% Single-Unit - - - - - 0.0 0.8 - - Trucks - - - - - 0.0 0.8 - -	0.7	-	-	0.7	0.0	-	0.6	0.7
Articulated Trucks 0 0 0 0 - 0 0 0 0 0 - 0 0 0 0 - 0 0 0 0 -	0	0	0	0	0	-	0	0
% Articulated Trucks - - - - - 0.0 0.0 - -	0.0	-	-	0.0	0.0	-	0.0	0.0
Bicycles on Road 0 0 0 0 - 0 0 0 0 0 - 0 0 0 0 - 0 0 0 0 -	0	0	0	0	0	-	0	0
% Bicycles on Road - - - - - 0.0 0.0 - -	0.0	-	-	0.0	0.0	-	0.0	0.0
Pedestrians 1 - 1 0	-	-	-	-	-	0	-	-
							_	



Rosemont, Illinois, United States 60018 (847)518-9990 mmendoza@kloainc.com

Count Name: Main Street with High School North Access Drive TMC Site Code: Start Date: 04/04/2024 Page No: 4

				Street					Main Westt	Drive				Hi	gh School North		ive				Main South				
Start Time	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	Int. Total
2:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Approach %	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	-
Total %	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0	-	0.0	-
PHF	0.000	0.000	0.000	0.000	-	0.000	0.000	0.000	0.000	0.000	-	0.000	0.000	0.000	0.000	0.000	-	0.000	0.000	0.000	0.000	0.000	-	0.000	0.000
Lights	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Lights	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Buses	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Buses	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
Single-Unit Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Single-Unit Trucks	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Articulated Trucks	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



Rosemont, Illinois, United States 60018 (847)518-9990 mmendoza@kloainc.com

Count Name: Main Street with High School Middle Access Drive TMC Site Code: Start Date: 04/04/2024 Page No: 1

Otest Time		High	h School Access I Eastbound	Drive			ing me	Main Street Northbound	Julia				Main Street Southbound			
Start Time	U-Turn	Left	Right	Peds	App. Total	U-Turn	Left	Thru	Peds	App. Total	U-Turn	Thru	Right	Peds	App. Total	Int. Total
6:30 AM	0	0	0	1	0	0	0	58	0	58	0	81	0	0	81	139
6:45 AM	0	0	0	0	0	0	0	72	0	72	0	115	0	0	115	187
Hourly Total	0	0	0	1	0	0	0	130	0	130	0	196	0	0	196	326
7:00 AM	0	0	0	1	0	0	0	79	0	79	0	141	0	0	141	220
7:15 AM	0	5	2	0	7	0	0	190	0	190	0	168	0	0	168	365
7:30 AM	0	1	2	0	3	0	0	164	0	164	0	246	0	0	246	413
7:45 AM	0	0	0	1	0	0	0	222	0	222	0	251	0	0	251	473
Hourly Total	0	6	4	2	10	0	0	655	0	655	0	806	0	0	806	1471
8:00 AM	0	0	0	0	0	0	0	149	0	149	0	160	0	0	160	309
8:15 AM	0	0	0	1	0	0	0	152	0	152	0	197	0	0	197	349
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	0	0	0	1	0	0	0	301	0	301	0	357	0	0	357	658
1:30 PM	0	1	0	2	1	0	0	120	0	120	0	145	0	0	145	266
1:45 PM	0	0	0	1	0	0	0	138	0	138	0	151	0	0	151	289
Hourly Total	0	1	0	3	1	0	0	258	0	258	0	296	0	0	296	555
2:00 PM	0	0	0	1	0	0	0	156	0	156	0	157	0	0	157	313
2:15 PM	0	0	0	0	0	0	0	178	0	178	0	165	0	0	165	343
2:30 PM	0	0	0	4	0	0	0	199	0	199	0	207	0	0	207	406
2:45 PM	0	0	4	0	4	0	0	233	0	233	0	173	0	0	173	410
Hourly Total	0	0	4	5	4	0	0	766	0	766	0	702	0	0	702	1472
3:00 PM	0	0	0	0	0	0	0	221	0	221	0	162	0	0	162	383
3:15 PM	0	0	0	0	0	0	0	190	0	190	0	247	0	0	247	437
Grand Total	0	7	8	12	15	0	0	2521	0	2521	0	2766	0	0	2766	5302
Approach %	0.0	46.7	53.3	-	-	0.0	0.0	100.0	-	-	0.0	100.0	0.0	-	-	-
Total %	0.0	0.1	0.2	-	0.3	0.0	0.0	47.5	-	47.5	0.0	52.2	0.0	-	52.2	-
Lights	0	6	8	-	14	0	0	2466	-	2466	0	2697	0	-	2697	5177
% Lights	-	85.7	100.0	-	93.3	-	-	97.8	-	97.8	-	97.5	-	-	97.5	97.6
Buses	0	0	0	-	0	0	0	33	-	33	0	40	0	-	40	73
% Buses	-	0.0	0.0	-	0.0	-	-	1.3	-	1.3	-	1.4		-	1.4	1.4
Single-Unit Trucks	0	1	0	-	1	0	0	20	-	20	0	27	0	-	27	48
% Single-Unit Trucks	-	14.3	0.0	-	6.7	-	-	0.8	-	0.8	-	1.0	-	-	1.0	0.9
Articulated Trucks	0	0	0	-	0	0	0	2	-	2	0	2	0	-	2	4
% Articulated Trucks	-	0.0	0.0	-	0.0	-	-	0.1	-	0.1	-	0.1	-	-	0.1	0.1
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	-	0.0	0.0	-	0.0	-	-	0.0	-	0.0	-	0.0	-	-	0.0	0.0
Pedestrians	-	-	-	12	-	-	-	-	0	-	-	-	-	0	-	-

|--|



Rosemont, Illinois, United States 60018 (847)518-9990 mmendoza@kloainc.com

Count Name: Main Street with High School Middle Access Drive TMC Site Code: Start Date: 04/04/2024 Page No: 3

Start Time		High	School Access Eastbound	Drive		,,		Main Street Northbound	,	,			Main Street Southbound			
Start Time	U-Turn	Left	Right	Peds	App. Total	U-Turn	Left	Thru	Peds	App. Total	U-Turn	Thru	Right	Peds	App. Total	Int. Total
6:45 AM	0	0	0	0	0	0	0	72	0	72	0	115	0	0	115	187
7:00 AM	0	0	0	1	0	0	0	79	0	79	0	141	0	0	141	220
7:15 AM	0	5	2	0	7	0	0	190	0	190	0	168	0	0	168	365
7:30 AM	0	1	2	0	3	0	0	164	0	164	0	246	0	0	246	413
Total	0	6	4	1	10	0	0	505	0	505	0	670	0	0	670	1185
Approach %	0.0	60.0	40.0	-	-	0.0	0.0	100.0	-	-	0.0	100.0	0.0	-	-	-
Total %	0.0	0.5	0.3	-	0.8	0.0	0.0	42.6	-	42.6	0.0	56.5	0.0	-	56.5	-
PHF	0.000	0.300	0.500	-	0.357	0.000	0.000	0.664	-	0.664	0.000	0.681	0.000	-	0.681	0.717
Lights	0	6	4	-	10	0	0	500	-	500	0	651	0	-	651	1161
% Lights	-	100.0	100.0	-	100.0	-	-	99.0	-	99.0	-	97.2	-	-	97.2	98.0
Buses	0	0	0	-	0	0	0	3	-	3	0	13	0	-	13	16
% Buses	-	0.0	0.0	-	0.0	-	-	0.6	-	0.6	-	1.9	-	-	1.9	1.4
Single-Unit Trucks	0	0	0	-	0	0	0	2	-	2	0	6	0	-	6	8
% Single-Unit Trucks	-	0.0	0.0	-	0.0	-	-	0.4	-	0.4	-	0.9	-	-	0.9	0.7
Articulated Trucks	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Articulated Trucks	-	0.0	0.0	-	0.0	-	-	0.0	-	0.0	-	0.0	-	-	0.0	0.0
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	-	0.0	0.0	-	0.0	-	-	0.0	-	0.0	-	0.0	-	-	0.0	0.0
Pedestrians	-	-	-	1	-	-	-	-	0	-	-	-	-	0	-	-
% Pedestrians	-	-	-	100.0	-	-	-	-	-	-	-	-	-	-	-	-



Rosemont, Illinois, United States 60018 (847)518-9990 mmendoza@kloainc.com

Count Name: Main Street with High School Middle Access Drive TMC Site Code: Start Date: 04/04/2024 Page No: 4

		High	School Access Eastbound	Drive		,		Main Street Northbound	X	,			Main Street Southbound			
Start Time	U-Turn	Left	Right	Peds	App. Total	U-Turn	Left	Thru	Peds	App. Total	U-Turn	Thru	Right	Peds	App. Total	Int. Total
2:30 PM	0	0	0	4	0	0	0	199	0	199	0	207	0	0	207	406
2:45 PM	0	0	4	0	4	0	0	233	0	233	0	173	0	0	173	410
3:00 PM	0	0	0	0	0	0	0	221	0	221	0	162	0	0	162	383
3:15 PM	0	0	0	0	0	0	0	190	0	190	0	247	0	0	247	437
Total	0	0	4	4	4	0	0	843	0	843	0	789	0	0	789	1636
Approach %	0.0	0.0	100.0	-	-	0.0	0.0	100.0	-	-	0.0	100.0	0.0	-	-	-
Total %	0.0	0.0	0.2	-	0.2	0.0	0.0	51.5	-	51.5	0.0	48.2	0.0	-	48.2	-
PHF	0.000	0.000	0.250	-	0.250	0.000	0.000	0.905	-	0.905	0.000	0.799	0.000	-	0.799	0.936
Lights	0	0	4	-	4	0	0	820	-	820	0	774	0	-	774	1598
% Lights	-	-	100.0	-	100.0	-	-	97.3	-	97.3	-	98.1	-	-	98.1	97.7
Buses	0	0	0	-	0	0	0	19	-	19	0	10	0	-	10	29
% Buses	-	-	0.0	-	0.0	-	-	2.3	-	2.3	-	1.3	-	-	1.3	1.8
Single-Unit Trucks	0	0	0	-	0	0	0	4	-	4	0	5	0	-	5	9
% Single-Unit Trucks	-	-	0.0	-	0.0	-	-	0.5	-	0.5	-	0.6	-	-	0.6	0.6
Articulated Trucks	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Articulated Trucks	-	-	0.0	-	0.0	-	-	0.0	-	0.0	-	0.0	-	-	0.0	0.0
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	-	-	0.0	-	0.0	-	-	0.0	-	0.0	-	0.0	-	-	0.0	0.0
Pedestrians	-	-	-	4	-	-	-	-	0	-	-	-	-	0	-	-
% Pedestrians	-	-	-	100.0	-	-	-	-	-	-	-	-	-	-	-	-



Rosemont, Illinois, United States 60018 (847)518-9990 mmendoza@kloainc.com

Count Name: Main Street with High School Parking Lot Access Drive TMC Site Code: Start Date: 04/04/2024 Page No: 1

	1	Par	king Lot Access I	Drive			ing mo	Main Street					Main Street			
			Eastbound	5				Northbound					Southbound			
Start Time	U-Turn	Left	Right	Peds	App. Total	U-Turn	Left	Thru	Peds	App. Total	U-Turn	Thru	Right	Peds	App. Total	Int. Total
6:30 AM	0	0	1	1	1	0	17	64	0	81	0	78	3	0	81	163
6:45 AM	0	1	0	0	1	0	12	79	0	91	0	114	6	0	120	212
Hourly Total	0	1	1	1	2	0	29	143	0	172	0	192	9	0	201	375
7:00 AM	0	1	2	1	3	0	37	84	0	121	0	157	8	1	165	289
7:15 AM	0	8	18	3	26	0	49	170	0	219	0	200	25	2	225	470
7:30 AM	0	4	8	7	12	0	8	161	0	169	0	265	4	1	269	450
7:45 AM	0	1	1	1	2	0	1	223	0	224	0	224	0	0	224	450
Hourly Total	0	14	29	12	43	0	95	638	0	733	0	846	37	4	883	1659
8:00 AM	0	0	1	0	1	0	1	139	0	140	0	163	4	0	167	308
8:15 AM	0	1	2	1	3	0	0	155	0	155	0	198	1	0	199	357
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	0	1	3	1	4	0	1	294	0	295	0	361	5	0	366	665
1:30 PM	0	2	1	4	3	0	1	136	0	137	0	146	0	0	146	286
1:45 PM	0	1	3	1	4	0	1	145	0	146	0	155	0	0	155	305
Hourly Total	0	3	4	5	7	0	2	281	0	283	0	301	0	0	301	591
2:00 PM	0	2	3	1	5	0	7	169	0	176	0	158	6	0	164	345
2:15 PM	0	0	3	0	3	0	11	205	0	216	0	145	12	0	157	376
2:30 PM	0	21	33	14	54	0	11	170	0	181	0	209	13	6	222	457
2:45 PM	0	7	24	5	31	0	8	236	0	244	0	159	9	0	168	443
Hourly Total	0	30	63	20	93	0	37	780	0	817	0	671	40	6	711	1621
3:00 PM	0	1	17	1	18	0	2	211	0	213	0	164	9	0	173	404
3:15 PM	0	1	10	5	11	0	2	184	0	186	0	245	20	0	265	462
Grand Total	0	51	127	45	178	0	168	2531	0	2699	0	2780	120	10	2900	5777
Approach %	0.0	28.7	71.3	-	-	0.0	6.2	93.8	-	-	0.0	95.9	4.1	-	-	-
Total %	0.0	0.9	2.2	-	3.1	0.0	2.9	43.8	-	46.7	0.0	48.1	2.1	-	50.2	-
Lights	0	50	122	-	172	0	163	2466	-	2629	0	2717	118	-	2835	5636
% Lights	-	98.0	96.1	-	96.6	-	97.0	97.4	-	97.4	-	97.7	98.3	-	97.8	97.6
Buses	0	0	3	-	3	0	3	33	-	36	0	36	1	-	37	76
% Buses	-	0.0	2.4	-	1.7	-	1.8	1.3	-	1.3	-	1.3	0.8	-	1.3	1.3
Single-Unit Trucks	0	1	2	-	3	0	2	29	-	31	0	24	1	-	25	59
% Single-Unit Trucks	-	2.0	1.6	-	1.7	-	1.2	1.1	-	1.1	-	0.9	0.8	-	0.9	1.0
Articulated Trucks	0	0	0	-	0	0	0	3	-	3	0	3	0	-	3	6
% Articulated Trucks	-	0.0	0.0	-	0.0	-	0.0	0.1	-	0.1	-	0.1	0.0	-	0.1	0.1
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	-	45	-	-	-	-	0	-	-	-	-	10	-	-

	% Pedestrians	-	-	-	100.0	-	-	-	-	-	-	-	-	-	100.0	-	-
--	---------------	---	---	---	-------	---	---	---	---	---	---	---	---	---	-------	---	---



Rosemont, Illinois, United States 60018 (847)518-9990 mmendoza@kloainc.com

Count Name: Main Street with High School Parking Lot Access Drive TMC Site Code: Start Date: 04/04/2024 Page No: 3

Start Time		Parl	king Lot Access Eastbound	Drive				Main Street Northbound	,	,			Main Street Southbound			
Start Time	U-Turn	Left	Right	Peds	App. Total	U-Turn	Left	Thru	Peds	App. Total	U-Turn	Thru	Right	Peds	App. Total	Int. Total
6:45 AM	0	1	0	0	1	0	12	79	0	91	0	114	6	0	120	212
7:00 AM	0	1	2	1	3	0	37	84	0	121	0	157	8	1	165	289
7:15 AM	0	8	18	3	26	0	49	170	0	219	0	200	25	2	225	470
7:30 AM	0	4	8	7	12	0	8	161	0	169	0	265	4	1	269	450
Total	0	14	28	11	42	0	106	494	0	600	0	736	43	4	779	1421
Approach %	0.0	33.3	66.7	-	-	0.0	17.7	82.3	-	-	0.0	94.5	5.5	-	-	-
Total %	0.0	1.0	2.0	-	3.0	0.0	7.5	34.8	-	42.2	0.0	51.8	3.0	-	54.8	-
PHF	0.000	0.438	0.389	-	0.404	0.000	0.541	0.726	-	0.685	0.000	0.694	0.430	-	0.724	0.756
Lights	0	13	27	-	40	0	104	486	-	590	0	717	43	-	760	1390
% Lights	-	92.9	96.4	-	95.2	-	98.1	98.4	-	98.3	-	97.4	100.0	-	97.6	97.8
Buses	0	0	1	-	1	0	1	6	-	7	0	14	0	-	14	22
% Buses	-	0.0	3.6	-	2.4	-	0.9	1.2	-	1.2	-	1.9	0.0	-	1.8	1.5
Single-Unit Trucks	0	1	0	-	1	0	1	2	-	3	0	5	0	-	5	9
% Single-Unit Trucks	-	7.1	0.0	-	2.4	-	0.9	0.4	-	0.5	-	0.7	0.0	-	0.6	0.6
Articulated Trucks	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Articulated Trucks	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	0.0
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	-	11	-	-	-	-	0	-	-	-	-	4	-	-
% Pedestrians	-	-	-	100.0	-	-	-	-	-	-	-	-	-	100.0	-	-



Rosemont, Illinois, United States 60018 (847)518-9990 mmendoza@kloainc.com

Count Name: Main Street with High School Parking Lot Access Drive TMC Site Code: Start Date: 04/04/2024 Page No: 4

		Parl	king Lot Access I Eastbound	Drive				Main Street Northbound	,	,			Main Street Southbound			
Start Time	U-Turn	Left	Right	Peds	App. Total	U-Turn	Left	Thru	Peds	App. Total	U-Turn	Thru	Right	Peds	App. Total	Int. Total
2:30 PM	0	21	33	14	54	0	11	170	0	181	0	209	13	6	222	457
2:45 PM	0	7	24	5	31	0	8	236	0	244	0	159	9	0	168	443
3:00 PM	0	1	17	1	18	0	2	211	0	213	0	164	9	0	173	404
3:15 PM	0	1	10	5	11	0	2	184	0	186	0	245	20	0	265	462
Total	0	30	84	25	114	0	23	801	0	824	0	777	51	6	828	1766
Approach %	0.0	26.3	73.7	-	-	0.0	2.8	97.2	-	-	0.0	93.8	6.2	-	-	-
Total %	0.0	1.7	4.8	-	6.5	0.0	1.3	45.4	-	46.7	0.0	44.0	2.9	-	46.9	-
PHF	0.000	0.357	0.636	-	0.528	0.000	0.523	0.849	-	0.844	0.000	0.793	0.638	-	0.781	0.956
Lights	0	30	82	-	112	0	21	776	-	797	0	759	50	-	809	1718
% Lights	-	100.0	97.6	-	98.2	-	91.3	96.9	-	96.7	-	97.7	98.0	-	97.7	97.3
Buses	0	0	2	-	2	0	2	14	-	16	0	12	1	-	13	31
% Buses	-	0.0	2.4	-	1.8	-	8.7	1.7	-	1.9	-	1.5	2.0	-	1.6	1.8
Single-Unit Trucks	0	0	0	-	0	0	0	10	-	10	0	5	0	-	5	15
% Single-Unit Trucks	-	0.0	0.0	-	0.0	-	0.0	1.2	-	1.2	-	0.6	0.0	-	0.6	0.8
Articulated Trucks	0	0	0	-	0	0	0	1	-	1	0	1	0	-	1	2
% Articulated Trucks	-	0.0	0.0	-	0.0	-	0.0	0.1	-	0.1	-	0.1	0.0	-	0.1	0.1
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	-	25	-	-	-	-	0	-	-	-	-	6	-	-
% Pedestrians	-	-	-	100.0	-	-	-	-	-	-	-	-	-	100.0	-	-



Rosemont, Illinois, United States 60018 (847)518-9990 mmendoza@kloainc.com

Count Name: Wilson Avenue with High School East Access Drive TMC Site Code: Start Date: 04/04/2024 Page No: 1

Start Time			Wilson Avenue Eastbound				ing me	Wilson Avenue Westbound					Access Drive Northbound			
	U-Turn	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Peds	App. Total	U-Turn	Left	Right	Peds	App. Total	Int. Total
6:30 AM	0	8	0	0	8	0	0	33	0	33	0	9	13	0	22	63
6:45 AM	0	9	0	0	9	0	0	47	0	47	0	11	25	1	36	92
Hourly Total	0	17	0	0	17	0	0	80	0	80	0	20	38	1	58	155
7:00 AM	0	8	0	0	8	0	0	41	1	41	0	37	42	0	79	128
7:15 AM	0	11	0	2	11	0	0	89	3	89	0	28	90	1	118	218
7:30 AM	0	14	0	0	14	1	0	40	4	41	0	26	49	0	75	130
7:45 AM	0	15	0	0	15	0	0	35	0	35	0	2	6	1	8	58
Hourly Total	0	48	0	2	48	1	0	205	8	206	0	93	187	2	280	534
8:00 AM	0	10	0	0	10	0	0	27	0	27	0	6	3	0	9	46
8:15 AM	0	17	0	0	17	0	0	32	0	32	0	5	19	0	24	73
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	0	27	0	0	27	0	0	59	0	59	0	11	22	0	33	119
1:30 PM	0	18	0	0	18	1	0	20	0	21	0	8	4	0	12	51
1:45 PM	0	26	0	0	26	0	0	23	1	23	0	1	6	1	7	56
Hourly Total	0	44	0	0	44	1	0	43	1	44	0	9	10	1	19	107
2:00 PM	0	15	0	0	15	0	0	15	0	15	0	1	5	0	6	36
2:15 PM	0	19	0	0	19	0	0	29	0	29	0	0	2	0	2	50
2:30 PM	0	79	0	0	79	0	0	31	3	31	0	7	24	16	31	141
2:45 PM	0	44	0	0	44	0	0	35	1	35	0	23	24	0	47	126
Hourly Total	0	157	0	0	157	0	0	110	4	110	0	31	55	16	86	353
3:00 PM	0	23	2	0	25	0	0	37	0	37	0	6	5	1	11	73
3:15 PM	0	23	0	0	23	0	0	36	0	36	0	5	7	0	12	71
Grand Total	0	339	2	2	341	2	0	570	13	572	0	175	324	21	499	1412
Approach %	0.0	99.4	0.6	-	-	0.3	0.0	99.7	-	-	0.0	35.1	64.9	-	-	-
Total %	0.0	24.0	0.1	-	24.2	0.1	0.0	40.4	-	40.5	0.0	12.4	22.9	-	35.3	-
Lights	0	333	2	-	335	2	0	551	-	553	0	148	306	-	454	1342
% Lights	-	98.2	100.0	-	98.2	100.0	-	96.7	-	96.7	-	84.6	94.4	-	91.0	95.0
Buses	0	6	0	-	6	0	0	14	-	14	0	27	17	-	44	64
% Buses	-	1.8	0.0	-	1.8	0.0	-	2.5	-	2.4	-	15.4	5.2	-	8.8	4.5
Single-Unit Trucks	0	0	0	-	0	0	0	2	-	2	0	0	0	-	0	2
% Single-Unit Trucks	-	0.0	0.0	-	0.0	0.0	-	0.4	-	0.3	-	0.0	0.0	-	0.0	0.1
Articulated Trucks	0	0	0	-	0	0	0	3	-	3	0	0	0	-	0	3
% Articulated Trucks	-	0.0	0.0	-	0.0	0.0	-	0.5	-	0.5	-	0.0	0.0	-	0.0	0.2
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	1	-	1	1
% Bicycles on Road	-	0.0	0.0	-	0.0	0.0	-	0.0	-	0.0	-	0.0	0.3	-	0.2	0.1
Pedestrians	-	-	-	2	-	-	-	-	13	-	-	-	-	21	-	-

% Pedestrians - - 100.0 - 100.0 - 100.0 - 100.0 - 100.0 - 100.0 - 100.0 - 100.0	% Pedestrians	-	-	-	100.0	-	-	-	-	100.0	-	-	-	-	100.0	-	-
---	---------------	---	---	---	-------	---	---	---	---	-------	---	---	---	---	-------	---	---



Rosemont, Illinois, United States 60018 (847)518-9990 mmendoza@kloainc.com

Count Name: Wilson Avenue with High School East Access Drive TMC Site Code: Start Date: 04/04/2024 Page No: 3

			Wilson Avenue Eastbound					Wilson Avenue Westbound	•				Access Drive Northbound			
Start Time	U-Turn	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Peds	App. Total	U-Turn	Left	Right	Peds	App. Total	Int. Total
6:45 AM	0	9	0	0	9	0	0	47	0	47	0	11	25	1	36	92
7:00 AM	0	8	0	0	8	0	0	41	1	41	0	37	42	0	79	128
7:15 AM	0	11	0	2	11	0	0	89	3	89	0	28	90	1	118	218
7:30 AM	0	14	0	0	14	1	0	40	4	41	0	26	49	0	75	130
Total	0	42	0	2	42	1	0	217	8	218	0	102	206	2	308	568
Approach %	0.0	100.0	0.0	-	-	0.5	0.0	99.5	-	-	0.0	33.1	66.9	-	-	-
Total %	0.0	7.4	0.0	-	7.4	0.2	0.0	38.2	-	38.4	0.0	18.0	36.3	-	54.2	-
PHF	0.000	0.750	0.000	-	0.750	0.250	0.000	0.610	-	0.612	0.000	0.689	0.572	-	0.653	0.651
Lights	0	40	0	-	40	1	0	210	-	211	0	90	193	-	283	534
% Lights	-	95.2	-	-	95.2	100.0	-	96.8	-	96.8	-	88.2	93.7	-	91.9	94.0
Buses	0	2	0	-	2	0	0	7	-	7	0	12	13	-	25	34
% Buses	-	4.8	-	-	4.8	0.0	-	3.2	-	3.2	-	11.8	6.3	-	8.1	6.0
Single-Unit Trucks	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Single-Unit Trucks	-	0.0	-	-	0.0	0.0	-	0.0	-	0.0	-	0.0	0.0	-	0.0	0.0
Articulated Trucks	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Articulated Trucks	-	0.0	-	-	0.0	0.0	-	0.0	-	0.0	-	0.0	0.0	-	0.0	0.0
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	-	0.0	-	-	0.0	0.0	-	0.0	-	0.0	-	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	-	2	-	-	-	-	8	-	-	-	-	2	-	-
% Pedestrians	-	-	-	100.0	-	-	-	-	100.0	-	-	-	-	100.0	-	-



Rosemont, Illinois, United States 60018 (847)518-9990 mmendoza@kloainc.com

Count Name: Wilson Avenue with High School East Access Drive TMC Site Code: Start Date: 04/04/2024 Page No: 4

Out of Time			Wilson Avenue Eastbound					Wilson Avenue Westbound	,	,			Access Drive Northbound			
Start Time	U-Turn	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Peds	App. Total	U-Turn	Left	Right	Peds	App. Total	Int. Total
2:30 PM	0	79	0	0	79	0	0	31	3	31	0	7	24	16	31	141
2:45 PM	0	44	0	0	44	0	0	35	1	35	0	23	24	0	47	126
3:00 PM	0	23	2	0	25	0	0	37	0	37	0	6	5	1	11	73
3:15 PM	0	23	0	0	23	0	0	36	0	36	0	5	7	0	12	71
Total	0	169	2	0	171	0	0	139	4	139	0	41	60	17	101	411
Approach %	0.0	98.8	1.2	-	-	0.0	0.0	100.0	-	-	0.0	40.6	59.4	-	-	-
Total %	0.0	41.1	0.5	-	41.6	0.0	0.0	33.8	-	33.8	0.0	10.0	14.6	-	24.6	-
PHF	0.000	0.535	0.250	-	0.541	0.000	0.000	0.939	-	0.939	0.000	0.446	0.625	-	0.537	0.729
Lights	0	168	2	-	170	0	0	134	-	134	0	27	56	-	83	387
% Lights	-	99.4	100.0	-	99.4	-	-	96.4	-	96.4	-	65.9	93.3	-	82.2	94.2
Buses	0	1	0	-	1	0	0	2	-	2	0	14	3	-	17	20
% Buses	-	0.6	0.0	-	0.6	-	-	1.4	-	1.4	-	34.1	5.0	-	16.8	4.9
Single-Unit Trucks	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Single-Unit Trucks	-	0.0	0.0	-	0.0	-	-	0.0	-	0.0	-	0.0	0.0	-	0.0	0.0
Articulated Trucks	0	0	0	-	0	0	0	3	-	3	0	0	0	-	0	3
% Articulated Trucks	-	0.0	0.0	-	0.0	-	-	2.2	-	2.2	-	0.0	0.0	-	0.0	0.7
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	1	-	1	1
% Bicycles on Road	-	0.0	0.0	-	0.0	-	-	0.0	-	0.0	-	0.0	1.7	-	1.0	0.2
Pedestrians	-	-	-	0	-	-	-	-	4	-	-	-	-	17	-	-
% Pedestrians	-	-	-	-	-	-	-	-	100.0	-	-	-	-	100.0	-	-



Rosemont, Illinois, United States 60018 (847)518-9990 mmendoza@kloainc.com

Count Name: Wilson Avenue with Lincoln Avenue TMC Site Code: Start Date: 04/04/2024 Page No: 1

				Avenue bound						Avenue bound					igh School North	Access Dri bound	ve		Lincoln Avenue Southbound								
Start Time	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	Int. Total		
6:30 AM	0	0	6	12	0	18	0	26	16	5	0	47	0	0	0	0	0	0	0	0	2	0	0	2	67		
6:45 AM	0	1	9	32	0	42	0	35	26	1	0	62	0	0	0	0	1	0	0	1	2	0	0	3	107		
Hourly Total	0	1	15	44	0	60	0	61	42	6	0	109	0	0	0	0	1	0	0	1	4	0	0	5	174		
7:00 AM	0	0	8	75	0	83	0	28	30	19	0	77	0	0	0	0	0	0	0	0	1	1	0	2	162		
7:15 AM	0	1	8	76	15	85	0	52	49	4	0	105	0	0	0	0	1	0	0	4	18	21	0	43	233		
7:30 AM	0	4	10	21	2	35	0	25	40	5	0	70	0	0	0	0	0	0	0	3	1	3	1	7	112		
7:45 AM	0	2	10	1	1	13	0	10	22	3	0	35	0	0	0	0	1	0	0	1	0	0	0	1	49		
Hourly Total	0	7	36	173	18	216	0	115	141	31	0	287	0	0	0	0	2	0	0	8	20	25	1	53	556		
8:00 AM	0	0	9	8	0	17	0	5	26	2	0	33	0	0	0	0	0	0	0	0	1	1	0	2	52		
8:15 AM	0	0	12	11	. 1	23	0	13	23	1	0	37	0	0	0	1	0	1	0	0	1	4	1	5	66		
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-		
Hourly Total	0	0	21	19	1	40	0	18	49	3	0	70	0	0	0	1	0	1	0	0	2	5	1	7	118		
1:30 PM	0	2	22	8	0	32	0	4	24	1	0	29	0	2	0	0	0	2	0	0	1	0	0	1	64		
1:45 PM	0	1	27	3	0	31	0	6	14	2	0	22	0	2	0	0	0	2	0	0	0	1	0	1	56		
Hourly Total	0	3	49	11	0	63	0	10	38	3	0	51	0	4	0	0	0	4	0	0	1	1	0	2	120		
2:00 PM	0	0	13	9	0	22	0	2	15	2	0	19	0	0	0	0	0	0	0	4	1	1	0	6	47		
2:15 PM	0	0	20	17	0	37	0	4	18	7	0	29	0	2	0	4	1	6	0	3	0	0	0	3	75		
2:30 PM	0	15	51	16	0	82	0	10	29	5	0	44	0	13	0	10	0	23	0	5	1	7	5	13	162		
2:45 PM	0	2	30	9	0	41	0	8	41	4	0	53	0	13	1	7	0	21	0	2	2	5	0	9	124		
Hourly Total	0	17	114	51	0	182	0	24	103	18	0	145	0	28	1	21	1	50	0	14	4	13	5	31	408		
3:00 PM	0	1	21	4	0	26	0	3	36	3	0	42	0	2	0	1	0	3	0	3	0	0	0	3	74		
3:15 PM	0	1	22	4	0	27	0	7	35	4	0	46	0	5	0	0	0	5	0	0	0	0	0	0	78		
Grand Total	0	30	278	306	19	614	0	238	444	68	0	750	0	39	1	23	4	63	0	26	31	44	7	101	1528		
Approach %	0.0	4.9	45.3	49.8	-	-	0.0	31.7	59.2	9.1	-	-	0.0	61.9	1.6	36.5	-	-	0.0	25.7	30.7	43.6	-	-	-		
Total %	0.0	2.0	18.2	20.0	-	40.2	0.0	15.6	29.1	4.5	-	49.1	0.0	2.6	0.1	1.5	-	4.1	0.0	1.7	2.0	2.9	-	6.6	-		
Lights	0	30	272	267	-	569	0	231	421	54	-	706	0	35	1	23	-	59	0	25	29	44	-	98	1432		
% Lights	-	100.0	97.8	87.3	-	92.7	-	97.1	94.8	79.4	-	94.1	-	89.7	100.0	100.0	-	93.7	-	96.2	93.5	100.0	-	97.0	93.7		
Buses	0	0	6	39	-	45	0	7	19	14	-	40	0	3	0	0	-	3	0	1	1	0	-	2	90		
% Buses	-	0.0	2.2	12.7	-	7.3	-	2.9	4.3	20.6	-	5.3	-	7.7	0.0	0.0	-	4.8	-	3.8	3.2	0.0	-	2.0	5.9		
Single-Unit Trucks	0	0	0	0	-	0	0	0	1	0	-	1	0	0	0	0	-	0	0	0	0	0	-	0	1		
% Single-Unit Trucks	-	0.0	0.0	0.0	-	0.0	-	0.0	0.2	0.0	-	0.1	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.1		
Articulated Trucks	0	0	0	0	-	0	0	0	3	0	-	3	0	0	0	0	-	0	0	0	0	0	-	0	3		
% Articulated Trucks	-	0.0	0.0	0.0	-	0.0	-	0.0	0.7	0.0	-	0.4	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.2		
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	1	0	0	-	1	0	0	1	0	-	1	2		

% Bicycles on Road	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	2.6	0.0	0.0	-	1.6	-	0.0	3.2	0.0	-	1.0	0.1
Pedestrians	-	-	-	-	19	- '	-	-	-	-	0	_	-	-	-	-	4	-	-	-	-	-	7	-	-
% Pedestrians	-	-	-	-	100.0	- '	-	-	-	-	-	1	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-



Rosemont, Illinois, United States 60018 (847)518-9990 mmendoza@kloainc.com

Count Name: Wilson Avenue with Lincoln Avenue TMC Site Code: Start Date: 04/04/2024 Page No: 3

			Wilcon	Avenue					Wilson	Avenue				Ц		Access Dri	NO				Lincoln	Δνορμο			l	
														п	•		ve		Lincoln Avenue Southbound							
Start Time			East	bound					West	bound					North	bound					South	bound				
Start Time	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	Int. Total	
6:45 AM	0	1	9	32	0	42	0	35	26	1	0	62	0	0	0	0	1	0	0	1	2	0	0	3	107	
7:00 AM	0	0	8	75	0	83	0	28	30	19	0	77	0	0	0	0	0	0	0	0	1	1	0	2	162	
7:15 AM	0	1	8	76	15	85	0	52	49	4	0	105	0	0	0	0	1	0	0	4	18	21	0	43	233	
7:30 AM	0	4	10	21	2	35	0	25	40	5	0	70	0	0	0	0	0	0	0	3	1	3	1	7	112	
Total	0	6	35	204	17	245	0	140	145	29	0	314	0	0	0	0	2	0	0	8	22	25	1	55	614	
Approach %	0.0	2.4	14.3	83.3	-	-	0.0	44.6	46.2	9.2	-	-	0.0	0.0	0.0	0.0	-	-	0.0	14.5	40.0	45.5	-	-	-	
Total %	0.0	1.0	5.7	33.2	-	39.9	0.0	22.8	23.6	4.7	-	51.1	0.0	0.0	0.0	0.0	-	0.0	0.0	1.3	3.6	4.1	-	9.0	-	
PHF	0.000	0.375	0.875	0.671	-	0.721	0.000	0.673	0.740	0.382	-	0.748	0.000	0.000	0.000	0.000	-	0.000	0.000	0.500	0.306	0.298	-	0.320	0.659	
Lights	0	6	32	183	-	221	0	136	141	18	-	295	0	0	0	0	-	0	0	7	21	25	-	53	569	
% Lights	-	100.0	91.4	89.7	-	90.2	-	97.1	97.2	62.1	-	93.9	-	-	-	-	-	-	-	87.5	95.5	100.0	-	96.4	92.7	
Buses	0	0	3	21	-	24	0	4	4	11	-	19	0	0	0	0	-	0	0	1	0	0	-	1	44	
% Buses	-	0.0	8.6	10.3	-	9.8	-	2.9	2.8	37.9	-	6.1	-	-	-	-	-	-	-	12.5	0.0	0.0	-	1.8	7.2	
Single-Unit Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	
% Single-Unit Trucks	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	-	-	-	-	-	-	0.0	0.0	0.0	-	0.0	0.0	
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	
% Articulated Trucks	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	-	-	-	-	-	-	0.0	0.0	0.0	-	0.0	0.0	
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	1	0	-	1	1	
% Bicycles on Road	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	-	-	-	-	-	-	0.0	4.5	0.0	-	1.8	0.2	
Pedestrians	-	-	-	-	17	-	-	-	-	-	0	-	-	-	-	-	2	-	-	-	-	-	1	-	-	
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	



Rosemont, Illinois, United States 60018 (847)518-9990 mmendoza@kloainc.com

Count Name: Wilson Avenue with Lincoln Avenue TMC Site Code: Start Date: 04/04/2024 Page No: 4

				Avenue bound				-		Avenue				Hi	gh School North	Access Dri	ive								
Start Time	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	bound Right	Peds	App. Total	Int. Total
2:30 PM	0	15	51	16	0	82	0	10	29	5	0	44	0	13	0	10	0	23	0	5	1	7	5	13	162
2:45 PM	0	2	30	9	0	41	0	8	41	4	0	53	0	13	1	7	0	21	0	2	2	5	0	9	124
3:00 PM	0	1	21	4	0	26	0	3	36	3	0	42	0	2	0	1	0	3	0	3	0	0	0	3	74
3:15 PM	0	1	22	4	0	27	0	7	35	4	0	46	0	5	0	0	0	5	0	0	0	0	0	0	78
Total	0	19	124	33	0	176	0	28	141	16	0	185	0	33	1	18	0	52	0	10	3	12	5	25	438
Approach %	0.0	10.8	70.5	18.8	-	-	0.0	15.1	76.2	8.6	-	-	0.0	63.5	1.9	34.6	-	-	0.0	40.0	12.0	48.0	-	-	-
Total %	0.0	4.3	28.3	7.5	-	40.2	0.0	6.4	32.2	3.7	-	42.2	0.0	7.5	0.2	4.1	-	11.9	0.0	2.3	0.7	2.7	-	5.7	-
PHF	0.000	0.317	0.608	0.516	-	0.537	0.000	0.700	0.860	0.800	-	0.873	0.000	0.635	0.250	0.450	-	0.565	0.000	0.500	0.375	0.429	-	0.481	0.676
Lights	0	19	123	32	-	174	0	28	126	13	-	167	0	30	1	18	-	49	0	10	2	12	-	24	414
% Lights	-	100.0	99.2	97.0	-	98.9	-	100.0	89.4	81.3	-	90.3	-	90.9	100.0	100.0	-	94.2	-	100.0	66.7	100.0	-	96.0	94.5
Buses	0	0	1	1	-	2	0	0	12	3	-	15	0	3	0	0	-	3	0	0	1	0	-	1	21
% Buses	-	0.0	0.8	3.0	-	1.1	-	0.0	8.5	18.8	-	8.1	-	9.1	0.0	0.0	-	5.8	-	0.0	33.3	0.0	-	4.0	4.8
Single-Unit Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Single-Unit Trucks	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.0
Articulated Trucks	0	0	0	0	-	0	0	0	3	0	-	3	0	0	0	0	-	0	0	0	0	0	-	0	3
% Articulated Trucks	-	0.0	0.0	0.0	-	0.0	-	0.0	2.1	0.0	-	1.6	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.7
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	5	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-


Rosemont, Illinois, United States 60018 (847)518-9990 mmendoza@kloainc.com

Count Name: Wilson Avenue with High School West Access Drive TMC Site Code: Start Date: 04/04/2024 Page No: 1

Turning Movement Data

Otest Time			Wilson Avenue Eastbound				ing we	Wilson Avenue Westbound				High	Northbound	Drive		
Start Time	U-Turn	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Peds	App. Total	U-Turn	Left	Right	Peds	App. Total	Int. Total
6:30 AM	0	21	3	0	24	0	2	17	0	19	0	0	0	19	0	43
6:45 AM	0	42	1	0	43	0	1	24	0	25	0	0	0	22	0	68
Hourly Total	0	63	4	0	67	0	3	41	0	44	0	0	0	41	0	111
7:00 AM	0	85	11	0	96	0	8	26	0	34	0	0	1	12	1	131
7:15 AM	0	88	28	2	116	0	39	37	0	76	0	6	3	35	9	201
7:30 AM	0	18	2	0	20	1	2	36	0	39	0	3	2	1	5	64
7:45 AM	0	14	2	0	16	0	2	21	0	23	0	0	0	3	0	39
Hourly Total	0	205	43	2	248	1	51	120	0	172	0	9	6	51	15	435
8:00 AM	0	18	1	0	19	0	1	21	0	22	0	0	0	4	0	41
8:15 AM	0	25	4	0	29	0	8	22	1	30	0	0	1	2	1	60
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	0	43	5	0	48	0	9	43	1	52	0	0	1	6	1	101
1:30 PM	0	21	3	0	24	0	5	19	0	24	0	6	14	3	20	68
1:45 PM	0	16	1	0	17	0	0	16	0	16	0	1	2	0	3	36
Hourly Total	0	37	4	0	41	0	5	35	0	40	0	7	16	3	23	104
2:00 PM	0	28	0	0	28	0	1	16	0	17	0	1	0	0	1	46
2:15 PM	0	28	7	0	35	3	3	16	0	22	0	3	0	2	3	60
2:30 PM	0	59	7	0	66	0	4	56	0	60	0	27	23	48	50	176
2:45 PM	0	38	1	0	39	0	1	47	0	48	0	0	2	3	2	89
Hourly Total	0	153	15	0	168	3	9	135	0	147	0	31	25	53	56	371
3:00 PM	0	23	0	0	23	0	0	38	0	38	0	1	0	2	1	62
3:15 PM	0	28	0	0	28	0	0	36	0	36	0	0	0	0	0	64
Grand Total	0	552	71	2	623	4	77	448	1	529	0	48	48	156	96	1248
Approach %	0.0	88.6	11.4	-	-	0.8	14.6	84.7	-	-	0.0	50.0	50.0	-	-	-
Total %	0.0	44.2	5.7	-	49.9	0.3	6.2	35.9	-	42.4	0.0	3.8	3.8	-	7.7	-
Lights	0	509	69	-	578	4	75	423	-	502	0	46	46	-	92	1172
% Lights	-	92.2	97.2	-	92.8	100.0	97.4	94.4	-	94.9	-	95.8	95.8	-	95.8	93.9
Buses	0	43	2	-	45	0	2	20	-	22	0	2	2	-	4	71
% Buses	-	7.8	2.8	-	7.2	0.0	2.6	4.5	-	4.2	-	4.2	4.2	-	4.2	5.7
Single-Unit Trucks	0	0	0	-	0	0	0	2	-	2	0	0	0	-	0	2
% Single-Unit Trucks	-	0.0	0.0	-	0.0	0.0	0.0	0.4	-	0.4	-	0.0	0.0	-	0.0	0.2
Articulated Trucks	0	0	0	-	0	0	0	3	-	3	0	0	0	-	0	3
% Articulated Trucks	-	0.0	0.0	-	0.0	0.0	0.0	0.7	-	0.6	-	0.0	0.0	-	0.0	0.2
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	-	0.0	0.0	-	0.0	0.0	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	-	2	-	-	-	-	1	-	-	-	-	156	_	-

% Pedestrians - - 100.0 - 100.0 - 100.0 - 100.0 - 100.0 - 100.0 - 100.0 - 100.0	% Pedestrians	-	-	-	100.0	-	-	-	-	100.0	-	-	-	-	100.0	-	-
---	---------------	---	---	---	-------	---	---	---	---	-------	---	---	---	---	-------	---	---



Rosemont, Illinois, United States 60018 (847)518-9990 mmendoza@kloainc.com

Count Name: Wilson Avenue with High School West Access Drive TMC Site Code: Start Date: 04/04/2024 Page No: 3

Turning Movement Peak Hour Data (6:45 AM)

Chart Time			Wilson Avenue Eastbound		,			Wilson Avenue Westbound	,	,		High	School Access Northbound	Drive		
Start Time	U-Turn	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Peds	App. Total	U-Turn	Left	Right	Peds	App. Total	Int. Total
6:45 AM	0	42	1	0	43	0	1	24	0	25	0	0	0	22	0	68
7:00 AM	0	85	11	0	96	0	8	26	0	34	0	0	1	12	1	131
7:15 AM	0	88	28	2	116	0	39	37	0	76	0	6	3	35	9	201
7:30 AM	0	18	2	0	20	1	2	36	0	39	0	3	2	1	5	64
Total	0	233	42	2	275	1	50	123	0	174	0	9	6	70	15	464
Approach %	0.0	84.7	15.3	-	-	0.6	28.7	70.7	-	-	0.0	60.0	40.0	-	-	-
Total %	0.0	50.2	9.1	-	59.3	0.2	10.8	26.5	-	37.5	0.0	1.9	1.3	-	3.2	-
PHF	0.000	0.662	0.375	-	0.593	0.250	0.321	0.831	-	0.572	0.000	0.375	0.500	-	0.417	0.577
Lights	0	211	40	-	251	1	48	121	-	170	0	7	4	-	11	432
% Lights	-	90.6	95.2	-	91.3	100.0	96.0	98.4	-	97.7	-	77.8	66.7	-	73.3	93.1
Buses	0	22	2	-	24	0	2	2	-	4	0	2	2	-	4	32
% Buses	-	9.4	4.8	-	8.7	0.0	4.0	1.6	-	2.3	-	22.2	33.3	-	26.7	6.9
Single-Unit Trucks	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Single-Unit Trucks	-	0.0	0.0	-	0.0	0.0	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	0.0
Articulated Trucks	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Articulated Trucks	-	0.0	0.0	-	0.0	0.0	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	0.0
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	-	0.0	0.0	-	0.0	0.0	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	-	2	-	-	-	-	0	-	-	-	-	70	-	-
% Pedestrians	-	-	-	100.0	-	-	-	-	-	-	-	-	-	100.0	-	-



Rosemont, Illinois, United States 60018 (847)518-9990 mmendoza@kloainc.com

Count Name: Wilson Avenue with High School West Access Drive TMC Site Code: Start Date: 04/04/2024 Page No: 4

Turning Movement Peak Hour Data (2:30 PM)

			Wilson Avenue Eastbound					Wilson Avenue Westbound	,			High	School Access Northbound	Drive		
Start Time	U-Turn	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Peds	App. Total	U-Turn	Left	Right	Peds	App. Total	Int. Total
2:30 PM	0	59	7	0	66	0	4	56	0	60	0	27	23	48	50	176
2:45 PM	0	38	1	0	39	0	1	47	0	48	0	0	2	3	2	89
3:00 PM	0	23	0	0	23	0	0	38	0	38	0	1	0	2	1	62
3:15 PM	0	28	0	0	28	0	0	36	0	36	0	0	0	0	0	64
Total	0	148	8	0	156	0	5	177	0	182	0	28	25	53	53	391
Approach %	0.0	94.9	5.1	-	-	0.0	2.7	97.3	-	-	0.0	52.8	47.2	-	-	-
Total %	0.0	37.9	2.0	-	39.9	0.0	1.3	45.3	-	46.5	0.0	7.2	6.4	-	13.6	-
PHF	0.000	0.627	0.286	-	0.591	0.000	0.313	0.790	-	0.758	0.000	0.259	0.272	-	0.265	0.555
Lights	0	146	8	-	154	0	5	160	-	165	0	28	25	-	53	372
% Lights	-	98.6	100.0	-	98.7	-	100.0	90.4	-	90.7	-	100.0	100.0	-	100.0	95.1
Buses	0	2	0	-	2	0	0	15	-	15	0	0	0	-	0	17
% Buses	-	1.4	0.0	-	1.3	-	0.0	8.5	-	8.2	-	0.0	0.0	-	0.0	4.3
Single-Unit Trucks	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Single-Unit Trucks	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	0.0
Articulated Trucks	0	0	0	-	0	0	0	2	-	2	0	0	0	-	0	2
% Articulated Trucks	-	0.0	0.0	-	0.0	-	0.0	1.1	-	1.1	-	0.0	0.0	-	0.0	0.5
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	0.0
Pedestrians	-	-		0	-	-	-	-	0	-	-	-	-	53	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-



Rosemont, Illinois, United States 60018 (847)518-9990 mmendoza@kloainc.com

Count Name: Wilson Avenue with Elizabeth Street TMC Site Code: Start Date: 04/04/2024 Page No: 1

Turning Movement Data

			Wilson Avenue Eastbound				ing me	Wilson Avenue Westbound					Elizabeth Street Southbound			
Start Time	U-Turn	Left	Thru	Peds	App. Total	U-Turn	Thru	Right	Peds	App. Total	U-Turn	Left	Right	Peds	App. Total	Int. Total
6:30 AM	0	1	16	1	17	0	14	0	0	14	0	3	0	1	3	34
6:45 AM	0	1	33	0	34	0	19	4	0	23	0	5	2	0	7	64
Hourly Total	0	2	49	1	51	0	33	4	0	37	0	8	2	1	10	98
7:00 AM	0	0	76	1	76	0	25	2	0	27	0	20	1	0	21	124
7:15 AM	0	3	91	17	94	0	32	6	1	38	0	23	3	1	26	158
7:30 AM	0	1	27	1	28	0	41	3	0	44	0	5	3	1	8	80
7:45 AM	0	3	13	0	16	0	22	1	0	23	0	2	2	0	4	43
Hourly Total	0	7	207	19	214	0	120	12	1	132	0	50	9	2	59	405
8:00 AM	0	2	18	0	20	0	22	3	0	25	0	1	2	0	3	48
8:15 AM	0	4	20	1	24	0	17	3	0	20	0	6	2	0	8	52
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hourly Total	0	6	38	1	44	0	39	6	0	45	0	7	4	0	11	100
1:30 PM	0	0	21	0	21	0	18	3	0	21	0	2	0	0	2	44
1:45 PM	0	0	15	0	15	0	22	3	0	25	0	0	2	0	2	42
Hourly Total	0	0	36	0	36	0	40	6	0	46	0	2	2	0	4	86
2:00 PM	0	0	21	0	21	0	11	3	0	14	0	2	0	0	2	37
2:15 PM	0	2	34	0	36	0	13	4	0	17	0	4	3	0	7	60
2:30 PM	0	4	36	0	40	0	49	13	1	62	0	16	5	2	21	123
2:45 PM	0	1	29	0	30	0	52	12	2	64	0	6	3	0	9	103
Hourly Total	0	7	120	0	127	0	125	32	3	157	0	28	11	2	39	323
3:00 PM	0	1	20	0	21	0	32	7	13	39	0	2	0	0	2	62
3:15 PM	0	2	20	0	22	0	37	1	1	38	0	5	2	0	7	67
Grand Total	0	25	490	21	515	0	426	68	18	494	0	102	30	5	132	1141
Approach %	0.0	4.9	95.1	-	-	0.0	86.2	13.8	-	-	0.0	77.3	22.7	-	-	-
Total %	0.0	2.2	42.9	-	45.1	0.0	37.3	6.0	-	43.3	0.0	8.9	2.6	-	11.6	-
Lights	0	25	451	-	476	0	404	62	-	466	0	98	29	-	127	1069
% Lights	-	100.0	92.0	-	92.4	-	94.8	91.2	-	94.3	-	96.1	96.7	-	96.2	93.7
Buses	0	0	38	-	38	0	18	5	-	23	0	4	0	-	4	65
% Buses	-	0.0	7.8	-	7.4	-	4.2	7.4	-	4.7	-	3.9	0.0	-	3.0	5.7
Single-Unit Trucks	0	0	1	-	1	0	1	1	-	2	0	0	1	-	1	4
% Single-Unit Trucks	-	0.0	0.2	-	0.2	-	0.2	1.5	-	0.4	-	0.0	3.3	-	0.8	0.4
Articulated Trucks	0	0	0	-	0	0	3	0	-	3	0	0	0	-	0	3
% Articulated Trucks	-	0.0	0.0	-	0.0	-	0.7	0.0	-	0.6	-	0.0	0.0	-	0.0	0.3
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	-	21	-	-	-	-	18	-	-	-	-	5	-	-

% Pedestrians - - 100.0 - 100.0 - 100.0 - 100.0 - 100.0 - 100.0 - 100.0 - 100.0	% Pedestrians	-	-	-	100.0	-	-	-	-	100.0	-	-	-	-	100.0	-	-
---	---------------	---	---	---	-------	---	---	---	---	-------	---	---	---	---	-------	---	---



Rosemont, Illinois, United States 60018 (847)518-9990 mmendoza@kloainc.com

Count Name: Wilson Avenue with Elizabeth Street TMC Site Code: Start Date: 04/04/2024 Page No: 3

Turning Movement Peak Hour Data (6:45 AM)

Start Time			Wilson Avenue Eastbound					Wilson Avenue Westbound	•	,			Elizabeth Street Southbound			
Start Time	U-Turn	Left	Thru	Peds	App. Total	U-Turn	Thru	Right	Peds	App. Total	U-Turn	Left	Right	Peds	App. Total	Int. Total
6:45 AM	0	1	33	0	34	0	19	4	0	23	0	5	2	0	7	64
7:00 AM	0	0	76	1	76	0	25	2	0	27	0	20	1	0	21	124
7:15 AM	0	3	91	17	94	0	32	6	1	38	0	23	3	1	26	158
7:30 AM	0	1	27	1	28	0	41	3	0	44	0	5	3	1	8	80
Total	0	5	227	19	232	0	117	15	1	132	0	53	9	2	62	426
Approach %	0.0	2.2	97.8	-	-	0.0	88.6	11.4	-	-	0.0	85.5	14.5	-	-	-
Total %	0.0	1.2	53.3	-	54.5	0.0	27.5	3.5	-	31.0	0.0	12.4	2.1	-	14.6	-
PHF	0.000	0.417	0.624	-	0.617	0.000	0.713	0.625	-	0.750	0.000	0.576	0.750	-	0.596	0.674
Lights	0	5	207	-	212	0	113	15	-	128	0	52	9	-	61	401
% Lights	-	100.0	91.2	-	91.4	-	96.6	100.0	-	97.0	-	98.1	100.0	-	98.4	94.1
Buses	0	0	20	-	20	0	4	0	-	4	0	1	0	-	1	25
% Buses	-	0.0	8.8	-	8.6	-	3.4	0.0	-	3.0	-	1.9	0.0	-	1.6	5.9
Single-Unit Trucks	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Single-Unit Trucks	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	0.0
Articulated Trucks	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Articulated Trucks	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	0.0
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	-	19	-	-	-	-	1	-	-	-	-	2	-	-
% Pedestrians	-	-	-	100.0	-	-	-	-	100.0	-	-	-	-	100.0	-	-



Rosemont, Illinois, United States 60018 (847)518-9990 mmendoza@kloainc.com

Count Name: Wilson Avenue with Elizabeth Street TMC Site Code: Start Date: 04/04/2024 Page No: 4

Turning Movement Peak Hour Data (2:30 PM)

Start Time			Wilson Avenue Eastbound		,			Wilson Avenue Westbound	•	,			Elizabeth Street Southbound			
Start Time	U-Turn	Left	Thru	Peds	App. Total	U-Turn	Thru	Right	Peds	App. Total	U-Turn	Left	Right	Peds	App. Total	Int. Total
2:30 PM	0	4	36	0	40	0	49	13	1	62	0	16	5	2	21	123
2:45 PM	0	1	29	0	30	0	52	12	2	64	0	6	3	0	9	103
3:00 PM	0	1	20	0	21	0	32	7	13	39	0	2	0	0	2	62
3:15 PM	0	2	20	0	22	0	37	1	1	38	0	5	2	0	7	67
Total	0	8	105	0	113	0	170	33	17	203	0	29	10	2	39	355
Approach %	0.0	7.1	92.9	-	-	0.0	83.7	16.3	-	-	0.0	74.4	25.6	-	-	-
Total %	0.0	2.3	29.6	-	31.8	0.0	47.9	9.3	-	57.2	0.0	8.2	2.8	-	11.0	-
PHF	0.000	0.500	0.729	-	0.706	0.000	0.817	0.635	-	0.793	0.000	0.453	0.500	-	0.464	0.722
Lights	0	8	104	-	112	0	156	28	-	184	0	28	10	-	38	334
% Lights	-	100.0	99.0	-	99.1	-	91.8	84.8	-	90.6	-	96.6	100.0	-	97.4	94.1
Buses	0	0	1	-	1	0	11	5	-	16	0	1	0	-	1	18
% Buses	-	0.0	1.0	-	0.9	-	6.5	15.2	-	7.9	-	3.4	0.0	-	2.6	5.1
Single-Unit Trucks	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Single-Unit Trucks	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	0.0
Articulated Trucks	0	0	0	-	0	0	3	0	-	3	0	0	0	-	0	3
% Articulated Trucks	-	0.0	0.0	-	0.0	-	1.8	0.0	-	1.5	-	0.0	0.0	-	0.0	0.8
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	-	0	-	-	-	-	17	-	-	-	-	2	-	-
% Pedestrians	-	-	-	-	-	-	-	-	100.0	-	-	-	-	100.0	-	-

Drop-Off/Pick-Up Peak Queue Aerial Images



Weekday Morning Peak Queue – Wilson Avenue

Figure A



Weekday Morning Peak Queue – Main Street

Figure B



Weekday Afternoon Peak Queue – Main Street

Figure C



Inbound Movements During Afternoon Queuing

Figure D



Weekday Afternoon Peak Queue – Wilson Avenue

Figure E



Weekday Afternoon Peak Queue – Lincoln Avenue

Figure F



Weekday Afternoon Dismissal Queue – Wilson Avenue

Figure G

Level of Service Criteria

LEVEL OF SERVICE CRITERIA

	Signalized Inters	sections	
Level of Service	Interpretation		Average Control Delay (seconds per vehicle)
A	Favorable progression. Most vehicle green indication and travel through the stopping.	-	≤10
В	Good progression, with more vehicle Level of Service A.	s stopping than for	>10 - 20
C	Individual cycle failures (i.e., one or m are not able to depart as a result of i during the cycle) may begin to appear. stopping is significant, although many through the intersection without stopping	nsufficient capacity Number of vehicles y vehicles still pass	>20 - 35
D	The volume-to-capacity ratio is high ar is ineffective or the cycle length is too stop and individual cycle failures are n	long. Many vehicles	>35 - 55
E	Progression is unfavorable. The volume high and the cycle length is long. Indi- are frequent.		>55 - 80
F	The volume-to-capacity ratio is very very poor, and the cycle length is long clear the queue.		>80.0
	Unsignalized Inter		
	Level of Service	Average Total Del	lay (SEC/VEH)
	А	0 -	10
	В	> 10 -	15
	С	> 15 -	25
	D	> 25 -	35
	E	> 35 -	50
	F	> 50	0
Source: Highwa	y Capacity Manual, 2010.		

<u>Capacity Analysis Summary Reports</u> Weekday Morning Peak Hour – Existing Conditions

Lanes, Volumes, Timings 1: Main Street & Wilson Avenue

	۶	+	*	4	Ļ	•	•	†	1	*	ţ	-√
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	4Î		ሻ	4Î		<u> </u>	†î≽		5	†î≽	
Traffic Volume (vph)	107	29	112	84	58	32	74	457	28	42	591	85
Future Volume (vph)	107	29	112	84	58	32	74	457	28	42	591	85
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	100		0	165		0	150		0	110		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	100			200			110			90		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Ped Bike Factor	1.00	0.98		0.99	1.00		1.00	1.00		1.00	1.00	
Frt		0.877			0.950			0.989			0.977	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1671	1545	0	1719	1728	0	1752	3457	0	1770	3410	0
Flt Permitted	0.605			0.396			0.193			0.342		
Satd. Flow (perm)	1063	1545	0	708	1728	0	356	3457	0	636	3410	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		167			25			9			22	
Link Speed (mph)		25			30			35			35	
Link Distance (ft)		291			419			247			516	
Travel Time (s)		7.9			9.5			4.8			10.1	
Confl. Peds. (#/hr)	1		14	14		1	4		3	3		4
Confl. Bikes (#/hr)									1			
Peak Hour Factor	0.54	0.78	0.67	0.64	0.66	0.73	0.62	0.65	0.50	0.88	0.70	0.56
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	8%	7%	5%	5%	3%	6%	3%	3%	4%	2%	3%	3%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	198	204	0	131	132	0	119	759	0	48	996	0
Turn Type	pm+pt	NA		pm+pt	NA		pm+pt	NA		pm+pt	NA	
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4			8			2			6		
Detector Phase	7	4		3	8		5	2		1	6	_
Switch Phase												
Minimum Initial (s)	3.0	8.0		3.0	8.0		3.0	15.0		3.0	15.0	
Minimum Split (s)	6.5	33.0		6.5	33.0		6.5	39.0		6.5	38.0	
Total Split (s)	13.0	34.0		13.0	34.0		15.0	40.0		13.0	38.0	
Total Split (%)	13.0%	34.0%		13.0%	34.0%		15.0%	40.0%		13.0%	38.0%	
Yellow Time (s)	3.0	4.0		3.0	4.0		3.0	4.0		3.0	4.0	
All-Red Time (s)	0.5	2.0		0.5	2.0		0.5	2.0		0.5	2.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	3.5	6.0		3.5	6.0		3.5	6.0		3.5	6.0	
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag		Lead	Lag	
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes		Yes	Yes	
Recall Mode	None	None		None	None		None	C-Max		None	C-Max	_
Act Effct Green (s)	25.1	13.1		24.2	12.7		64.0	56.0		59.7	50.7	
Actuated g/C Ratio	0.25	0.13		0.24	0.13		0.64	0.56		0.60	0.51	

24-093 - Glenbard East High School - Lombard Existing Weekday Morning Peak Hour

Synchro 11 Report

Lanes, Volumes, Timings 1: Main Street & Wilson Avenue

	٦	-	\mathbf{F}	∢	-	*	•	1	1	1	ţ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
v/c Ratio	0.61	0.59		0.50	0.55		0.35	0.39		0.11	0.57	
Control Delay	38.0	17.2		34.0	41.1		9.0	13.7		8.0	19.3	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	38.0	17.2		34.0	41.1		9.0	13.7		8.0	19.3	
LOS	D	В		С	D		А	В		А	В	
Approach Delay		27.4			37.5			13.0			18.8	
Approach LOS		С			D			В			В	
Queue Length 50th (ft)	103	21		65	64		16	150		10	216	
Queue Length 95th (ft)	86	58		73	79		28	147		26	225	
Internal Link Dist (ft)		211			339			167			436	
Turn Bay Length (ft)	100			165			150			110		
Base Capacity (vph)	324	552		270	501		391	1939		506	1738	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.61	0.37		0.49	0.26		0.30	0.39		0.09	0.57	
Intersection Summary												
Area Type:	Other											
Cycle Length: 100												
Actuated Cycle Length: 100)											
Offset: 49 (49%), Referenc	ed to phase	2:NBTL a	and 6:SB	FL, Start o	of Green							
Natural Cycle: 85												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.61												
Intersection Signal Delay: 2	20.1			In	tersectior	LOS: C						
Intersection Capacity Utilization	ation 62.1%			IC	U Level o	of Service	В					
Analysis Period (min) 15												

Splits and Phases: 1: Main Street & Wilson Avenue

Ø1	📢 ø2 (R)	√ Ø3	<u>↓</u> ₀₄
13 s	40 s	13 s	34 s
▲ Ø5	🖉 🕶 🖉 @6 (R)		★ Ø8
15 s	38 s	13 s	34 s

	۶	$\mathbf{\hat{v}}$	1	Ť	Ļ	~		
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø3	
Lane Configurations	ኘ	1	5	<u>†</u> †				
Traffic Volume (vph)	14	28	106	494	736	43		
Future Volume (vph)	14	28	106	494	736	43		
Ideal Flow (vphpl)	1900	1900	1900	2000	1900	1900		
Lane Width (ft)	12	12	11	11	11	11		
Grade (%)	0%			0%	0%			
Storage Length (ft)	60	0	95			0		
Storage Lanes	1	1	1			0		
Taper Length (ft)	55		110					
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	0.95		
Ped Bike Factor	0.99			0170	1.00	0170		
Frt	0177	0.850			0.987			
Flt Protected	0.950	0.000	0.950		01707			
Satd. Flow (prot)	1687	1553	1711	3601	3340	0		
Flt Permitted	0.950	1000	0.201	0001	0010	U		
Satd. Flow (perm)	1667	1553	362	3601	3340	0		
Right Turn on Red	1007	Yes	002	0001	0010	Yes		
Satd. Flow (RTOR)		70			10	105		
Link Speed (mph)	15	70		35	35			
Link Distance (ft)	509			375	98			
Travel Time (s)	23.1			7.3	1.9			
Confl. Peds. (#/hr)	4		11	7.5	1.7	11		
Confl. Bikes (#/hr)	Т							
Peak Hour Factor	0.44	0.40	0.54	0.73	0.69	0.43		
Growth Factor	100%	100%	100%	100%	100%	100%		
Heavy Vehicles (%)	7%	4%	2%	2%	3%	0%		
Bus Blockages (#/hr)	0	0	0	0	0	0		
Parking (#/hr)	0	0	0	0	U	0		
Mid-Block Traffic (%)	0%			0%	0%			
Shared Lane Traffic (%)	070			070	070			
Lane Group Flow (vph)	32	70	196	677	1167	0		
Turn Type	Prot	pm+ov	pm+pt	NA	NA	0		
Protected Phases	4	5	5	2	6		3	
Permitted Phases	T	4	2	2	0		5	
Detector Phase	4	5	5	2	6			
Switch Phase	4	J	J	2	0			
Minimum Initial (s)	8.0	3.0	3.0	15.0	15.0		8.0	
Minimum Split (s)	16.0	6.5	6.5	26.0	29.0		32.0	
Total Split (s)	16.0	15.0	15.0	52.0	37.0		32.0	
Total Split (%)	16.0%	15.0%	15.0%	52.0%	37.0%		32.0	
Yellow Time (s)	4.5	3.0	3.0	4.5	4.5		4.0	
All-Red Time (s)	4.5	0.0	0.0	4.5	4.5		4.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		0.0	
	0.0 6.0	3.0	3.0	6.0	6.0			
Total Lost Time (s)				0.0			Load	
Lead/Lag	Lag Yes	Lead	Lead		Lag		Lead	
Lead-Lag Optimize?		Yes	Yes	C Mov	Yes		Yes	
Recall Mode	None	None	None	C-Max	C-Max		None	
Act Effct Green (s)	9.5	18.7	87.1	86.5	72.3			
Actuated g/C Ratio	0.10	0.19	0.87	0.86	0.72			

24-093 - Glenbard East High School - Lombard Existing Weekday Morning Peak Hour

Synchro 11 Report

Lanes, Volumes, Timings 2: Main Street & Signalized Access Drive

	٦	\mathbf{r}	1	1	Ļ	∢	
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø3
v/c Ratio	0.20	0.20	0.45	0.22	0.48		
Control Delay	44.1	7.8	5.1	2.4	7.3		
Queue Delay	0.0	0.0	0.0	0.0	0.0		
Total Delay	44.1	7.8	5.1	2.4	7.3		
LOS	D	А	А	А	А		
Approach Delay	19.2			3.0	7.3		
Approach LOS	В			А	А		
Queue Length 50th (ft)	19	0	19	46	126		
Queue Length 95th (ft)	23	0	21	54	169		
Internal Link Dist (ft)	429			295	18		
Turn Bay Length (ft)	60		95				
Base Capacity (vph)	175	400	483	3116	2417		
Starvation Cap Reductn	0	0	0	0	0		
Spillback Cap Reductn	0	0	0	0	0		
Storage Cap Reductn	0	0	0	0	0		
Reduced v/c Ratio	0.18	0.17	0.41	0.22	0.48		
Intersection Summary							
Area Type:	Other						
Cycle Length: 100							
Actuated Cycle Length: 10							
Offset: 47 (47%), Reference	ced to phase	2:NBTL a	and 6:SB	T, Start of	Green		
Natural Cycle: 95							
Control Type: Actuated-Co	ordinated						
Maximum v/c Ratio: 0.48							
Intersection Signal Delay:					tersection		
Intersection Capacity Utiliz	ation 47.6%			IC	U Level o	f Service	A
Analysis Period (min) 15							

Splits and Phases: 2: Main Street & Signalized Access Drive



04/29/2024

Intersection Int Delay, s/veh 1.6 EBL EBR NBL SBT SBR Movement NBT **††** 702 Lane Configurations ٦ ۴ ħħ 50 Traffic Vol, veh/h 77 508 0 0 Future Vol, veh/h 50 77 0 508 702 0 Conflicting Peds, #/hr 0 0 3 3 0 0 Sign Control Stop Stop Free Free Free Free RT Channelized None -None -None -Storage Length 0 0 _ ---Veh in Median Storage, # 0 0 0 ---Grade, % 0 0 0 ---Peak Hour Factor 50 50 73 69 100 100

ajor/Minor	Minor2	Μ	ajor1	Μ	ajor2		
onflicting Flow All	1365	509	-	0	-	0	
Stage 1	1017	-	-	-	-	-	
Stage 2	348	-	-	-	-	-	
itical Hdwy	6.84	6.9	-	-	-	-	
itical Hdwy Stg 1	5.84	-	-	-	-	-	
itical Hdwy Stg 2	5.84	-	-	-	-	-	
llow-up Hdwy	3.52	3.3	-	-	-	-	
t Cap-1 Maneuver	*420	*803	0	-	-	0	
Stage 1	*529	-	0	-	-	0	
Stage 2	*879	-	0	-	-	0	
atoon blocked, %	1	1		-	-		
ov Cap-1 Maneuver		*803	-	-	-	-	
ov Cap-2 Maneuver	*420	-	-	-	-	-	
Stage 1	*529	-	-	-	-	-	
Stage 2	*879	-	-	-	-	-	
oroach	EB		NB		SB		
M Control Delay, s	12.7		0		0		
MLOS	В						
nor Lane/Major Mvr	nt	NBT E	BLn1 E	BLn2	SBT		
pacity (veh/h)		-	420	803	-		
CM Lane V/C Ratio		- ().238	0.192	-		
M Control Delay (s)	-	16.2	10.5	-		
M Lane LOS		-	С	В	-		
M 95th %tile Q(veh	ı)	-	0.9	0.7	-		
otes							

-: Volume exceeds capacity\$: Delay exceeds 300s

\$: Delay exceeds 300s +: Computation Not Defined

*: All major volume in platoon

Heavy Vehicles, %

Mvmt Flow

2

100

0

154

0

0

2

696 1017

3

0

0

Intersection Int Delay, s/veh 0.5 EBL EBR NBL NBT SBT SBR Movement Lane Configurations ¥ ħħ **↑**↑ 698 558 Traffic Vol, veh/h 6 4 0 0 Future Vol, veh/h 6 4 0 558 698 0 0 Conflicting Peds, #/hr 0 1 0 0 1 Sign Control Stop Stop Free Free Free Free RT Channelized None -None -None -Storage Length 0 -_ ---Veh in Median Storage, # 0 -0 0 --Grade, % 0 0 0 ---Peak Hour Factor 30 50 100 66 68 100 Heavy Vehicles, % 0 0 0 3 0 1 Mvmt Flow 20 8 0 845 1026 0

Major/Minor	Minor2	Μ	ajor1	Ма	ijor2	
Conflicting Flow All	1449	513	-	0	-	0
Stage 1	1026	-	-	-	-	-
Stage 2	423	-	-	-	-	-
Critical Hdwy	6.8	6.9	-	-	-	-
Critical Hdwy Stg 1	5.8	-	-	-	-	-
Critical Hdwy Stg 2	5.8	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	-	-
Pot Cap-1 Maneuver	124	512	0	-	-	0
Stage 1	311	-	0	-	-	0
Stage 2	635	-	0	-	-	0
Platoon blocked, %				-	-	
Mov Cap-1 Maneuver	124	512	-	-	-	-
Mov Cap-2 Maneuver	124	-	-	-	-	-
Stage 1	311	-	-	-	-	-
Stage 2	635	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	32.6		0		0	
HCM LOS	D					

Minor Lane/Major Mvmt	NBT EBLn1	SBT
Capacity (veh/h)	- 158	-
HCM Lane V/C Ratio	- 0.177	-
HCM Control Delay (s)	- 32.6	-
HCM Lane LOS	- D	-
HCM 95th %tile Q(veh)	- 0.6	-

Intersection

Int Delay, s/veh

0

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations			1		\$			∱î ≽			- 4 ↑		
Traffic Vol, veh/h	0	0	0	0	0	0	0	531	0	0	698	0	
Future Vol, veh/h	0	0	0	0	0	0	0	531	0	0	698	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	1	0	1	1	0	1	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	-	-	0	-	-	-	-	-	-	-	-	-	
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	100	100	100	100	100	100	47	71	100	100	72	36	
Heavy Vehicles, %	0	0	0	0	0	0	0	2	0	0	3	0	
Mvmt Flow	0	0	0	0	0	0	0	748	0	0	969	0	

Major/Minor	Minor2		Ν	/linor1		Ν	1ajor1		N	lajor2			
Conflicting Flow All	-	-	485	1234	1718	375	-	0	0	749	0	0	
Stage 1	-	-	-	749	749	-	-	-	-	-	-	-	
Stage 2	-	-	-	485	969	-	-	-	-	-	-	-	
Critical Hdwy	-	-	6.9	7.5	6.5	6.9	-	-	-	4.1	-	-	
Critical Hdwy Stg 1	-	-	-	6.5	5.5	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	-	-	-	6.5	5.5	-	-	-	-	-	-	-	
Follow-up Hdwy	-	-	3.3	3.5	4	3.3	-	-	-	2.2	-	-	
Pot Cap-1 Maneuver	0	0	533	135	91	628	0	-	-	869	-	0	
Stage 1	0	0	-	375	422	-	0	-	-	-	-	0	
Stage 2	0	0	-	537	334	-	0	-	-	-	-	0	
Platoon blocked, %								-	-		-		
Mov Cap-1 Maneuver	-	-	533	135	91	627	-	-	-	868	-	-	
Mov Cap-2 Maneuver	-	-	-	135	91	-	-	-	-	-	-	-	
Stage 1	-	-	-	375	422	-	-	-	-	-	-	-	
Stage 2	-	-	-	537	334	-	-	-	-	-	-	-	
Approach	EB			WB			NB			SB			
HCM Control Delay, s	0			0			0			0			
HCM LOS	А			А									
Minor Lane/Major Mvm	nt	NBT	NBR E	EBLn1V	VBLn1	SBL	SBT						
Capacity (yoh/h)						040							

Capacity (veh/h)	-	-	-	-	868	-	
HCM Lane V/C Ratio	-	-	-	-	-	-	
HCM Control Delay (s)	-	-	0	0	0	-	
HCM Lane LOS	-	-	А	А	А	-	
HCM 95th %tile Q(veh)	-	-	-	-	0	-	

04/29/2024	1
------------	---

Intersection						
Int Delay, s/veh	6.3					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑			↑	<u>۲</u>	1
Traffic Vol, veh/h	42	0	0	217	102	206
Future Vol, veh/h	42	0	0	217	102	206
Conflicting Peds, #/hr	0	2	2	0	2	8
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	0
Veh in Median Storage	, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	75	100	100	61	70	57
Heavy Vehicles, %	5	0	0	3	12	6
Mvmt Flow	56	0	0	356	146	361

Major/Minor M	lajor1		Major2		Minor1	
Conflicting Flow All	0	-	-	-	414	64
Stage 1	-	-	-	-	56	-
Stage 2	-	-	-	-	358	-
Critical Hdwy	-	-	-	-	6.52	6.26
Critical Hdwy Stg 1	-	-	-	-	5.52	-
Critical Hdwy Stg 2	-	-	-	-	5.52	-
Follow-up Hdwy	-	-	-	-	3.608	
Pot Cap-1 Maneuver	-	0	0	-		989
Stage 1	-	0	0	-	942	-
Stage 2	-	0	0	-	686	-
Platoon blocked, %	-			-		
Mov Cap-1 Maneuver	-	-	-	-	575	981
Mov Cap-2 Maneuver	-	-	-	-	575	-
Stage 1	-	-	-	-	, 12	-
Stage 2	-	-	-	-	685	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		11.5	
HCM LOS					В	
Minor Lang/Major Mymt		NBLn1	NDI n0	EDT		
Minor Lane/Major Mvmt				EBT	WBT	
Capacity (veh/h)		575	981	-	-	
HCM Lane V/C Ratio				-	-	
HCM Control Delay (s)		13.4	10.8	-	-	
HCM Lane LOS		B	B	-	-	
HCM 95th %tile Q(veh)		1	1.7	-	-	

Intersection

Int Delay, s/veh

7.4

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4			4			4			4		
Traffic Vol, veh/h	6	35	204	140	149	29	0	0	0	8	22	25	
Future Vol, veh/h	6	35	204	140	149	29	0	0	0	8	22	25	
Conflicting Peds, #/hr	1	0	2	2	0	1	17	0	0	0	0	17	
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized	-	-	None										
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-	
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	38	88	67	67	74	38	66	66	66	50	31	30	
Heavy Vehicles, %	0	9	10	3	3	38	0	0	0	13	4	0	
Mvmt Flow	16	40	304	209	201	76	0	0	0	16	71	83	

Major/Minor N	Major1		ľ	Major2		1	Minor1			Minor2			
Conflicting Flow All	278	0	0	346	0	0	977	922	194	882	1036	257	
Stage 1	-	-	-	-	-	-	226	226	-	658	658	-	
Stage 2	-	-	-	-	-	-	751	696	-	224	378	-	
Critical Hdwy	4.1	-	-	4.13	-	-	7.1	6.5	6.2	7.23	6.54	6.2	
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.23	5.54	-	
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.23	5.54	-	
Follow-up Hdwy	2.2	-	-	2.227	-	-	3.5	4	3.3	3.617	4.036	3.3	
Pot Cap-1 Maneuver	1296	-	-	1207	-	-	232	272	853	255	230	787	
Stage 1	-	-	-	-	-	-	781	721	-	436	458	-	
Stage 2	-	-	-	-	-	-	406	446	-	754	612	-	
Platoon blocked, %		-	-		-	-							
Mov Cap-1 Maneuver	1295	-	-	1205	-	-	118	212	851	212	179	774	
Mov Cap-2 Maneuver	-	-	-	-	-	-	118	212	-	212	179	-	
Stage 1	-	-	-	-	-	-	767	708	-	429	363	-	
Stage 2	-	-	-	-	-	-	227	353	-	742	601	-	
Approach	EB			WB			NB			SB			
HCM Control Delay, s	0.3			3.7			0			32.8			
HCM LOS							A			D			
Minor Lane/Major Mvm	t NB	Ln1	EBL	EBT	EBR	WBL	WBT	WBR S	SBLn1				
Capacity (veh/h)		-	1295	-	-	1205	-	-	294				
HCM Lane V/C Ratio		-	0.012	-	-	0.173	-	-	0.579				
HCM Control Delay (s)		0	7.8	0	-	8.6	0	-	32.8				
HCM Lane LOS		А	А	А	-	А	А	-	D				

0.6

-

3.4

0

-

-

HCM 95th %tile Q(veh)

Intersection							
Int Delay, s/veh	2.8						
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	4			- सी	۰¥		
Traffic Vol, veh/h	239	42	50	123	9	6	,
Future Vol, veh/h	239	42	50	123	9	6)
Conflicting Peds, #/hr	0	70	70	0	2	0	
Sign Control	Free	Free	Free	Free	Stop	Stop)
RT Channelized	-	None	-	None	-	None	,
Storage Length	-	-	-	-	0	-	
Veh in Median Storage	e, # 0	-	-	0	0	-	
Grade, %	0	-	-	0	0	-	
Peak Hour Factor	66	38	32	83	38	50	J
Heavy Vehicles, %	9	5	4	2	22	33	j
Mvmt Flow	362	111	156	148	24	12	

Major/Minor	Major1	Ν	Major2		Minor1		
Conflicting Flow All	0	0	543	0	950	488	
Stage 1	-	-	-	-	488	-	
Stage 2	-	-	-	-	462	-	
Critical Hdwy	-	-	4.14	-	6.62	6.53	
Critical Hdwy Stg 1	-	-	-	-	5.62	-	
Critical Hdwy Stg 2	-	-	-	-	5.62	-	
Follow-up Hdwy	-	-	2.236	-	3.698	3.597	
Pot Cap-1 Maneuver	-	-	1016	-	266	522	
Stage 1	-	-	-	-	578	-	
Stage 2	-	-	-	-	594	-	
Platoon blocked, %	-	-		-			
Mov Cap-1 Maneuver	-	-	948	-	203	487	
Mov Cap-2 Maneuver	-	-	-	-	203	-	
Stage 1	-	-	-	-	539	-	
Stage 2	-	-	-	-	486	-	
Approach	EB		WB		NB		
	0		4.9		21.6		-
HCM Control Delay, s HCM LOS	0		4.9		21.0 C		
					U		
Minor Lane/Major Mvn	nt N	BLn1	EBT	EBR	WBL	WBT	
Capacity (veh/h)		253	-	-	948	-	
HCM Lane V/C Ratio	(0.141	-	-	0.165	-	
LION Or a hard Data (a)							
HCM Control Delay (s) HCM Lane LOS)	21.6 C	-	-	9.5	0	

0.5

_

0.6

-

_

HCM 95th %tile Q(veh)

Intersection

Int Delay, s/veh	2.3						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		र्भ	et -		Y		
Traffic Vol, veh/h	5	227	117	15	53	9	
Future Vol, veh/h	5	227	117	15	53	9	
Conflicting Peds, #/hr	2	0	0	2	1	19	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	-	-	-	-	0	-	
Veh in Median Storage,	# -	0	0	-	0	-	
Grade, %	-	0	0	-	0	-	
Peak Hour Factor	42	62	71	63	58	75	
Heavy Vehicles, %	0	9	3	0	2	0	
Mvmt Flow	12	366	165	24	91	12	

Major/Minor	Major1	Ν	/lajor2	ſ	Vinor2		
Conflicting Flow All	191	0	-	0	570	198	}
Stage 1	-	-	-	-	179	-	-
Stage 2	-	-	-	-	391	-	
Critical Hdwy	4.1	-	-	-	6.42	6.2)
Critical Hdwy Stg 1	-	-	-	-	5.42	-	
Critical Hdwy Stg 2	-	-	-	-	5.42	-	
Follow-up Hdwy	2.2	-	-	-	3.518	3.3	
Pot Cap-1 Maneuver	1395	-	-	-	483	848	}
Stage 1	-	-	-	-	852	-	-
Stage 2	-	-	-	-	683	-	-
Platoon blocked, %		-	-	-			
Mov Cap-1 Maneuver		-	-	-	476	831	
Mov Cap-2 Maneuver	r -	-	-	-	476	-	-
Stage 1	-	-	-	-	841	-	-
Stage 2	-	-	-	-	682	-	-
Approach	EB		WB		SB		
HCM Control Delay, s	s 0.2		0		14		
HCM LOS					В		
Minor Lane/Major Mv	mt	EBL	EBT	WBT	WBR S	SBLn1	
Capacity (veh/h)		1392	-	-	-	501	-
HCM Lane V/C Ratio		0.009	-	-	-	0.206	
HCM Control Delay (s		7.6	0	-	-	14	
HCM Lane LOS	-,	A	A	-	-	В	
HCM 95th %tile Q(ve	h)	0	-	-	-	0.8	}

<u>Capacity Analysis Summary Reports</u> Weekday Afternoon Peak Hour – Existing Conditions

Lanes, Volumes, Timings 1: Main Street & Wilson Avenue

	٦	+	*	4	+	•	•	1	1	1	ţ	-√
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	<u>۲</u>	eî		7	eî 👘		7	∱ ⊅		<u>۲</u>	A⊅	
Traffic Volume (vph)	89	50	90	73	50	57	51	742	56	62	657	42
Future Volume (vph)	89	50	90	73	50	57	51	742	56	62	657	42
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	100		0	165		0	150		0	110		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	100			200			110			90		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Ped Bike Factor	1.00	0.96		0.97	0.99		1.00	1.00		1.00	1.00	
Frt		0.896			0.937			0.987			0.990	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1608	0	1787	1702	0	1805	3447	0	1719	3484	0
Flt Permitted	0.543			0.463			0.219			0.212		
Satd. Flow (perm)	1010	1608	0	842	1702	0	415	3447	0	383	3484	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		132			42			11			8	
Link Speed (mph)		25			30			35			35	
Link Distance (ft)		291			419			247			516	
Travel Time (s)		7.9			9.5			4.8			10.1	
Confl. Peds. (#/hr)	2		48	48		2	9		6	6		9
Confl. Bikes (#/hr)												
Peak Hour Factor	0.48	0.69	0.55	0.70	0.57	0.89	0.85	0.87	0.67	0.62	0.74	0.66
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	0%	2%	1%	4%	4%	0%	3%	4%	5%	2%	7%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	185	236	0	104	152	0	60	937	0	100	952	0
Turn Type	pm+pt	NA		pm+pt	NA		pm+pt	NA		pm+pt	NA	
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4			8			2			6		
Detector Phase	7	4		3	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	3.0	8.0		3.0	8.0		3.0	15.0		3.0	15.0	
Minimum Split (s)	6.5	33.0		6.5	33.0		6.5	31.0		6.5	31.0	
Total Split (s)	12.0	34.0		12.0	34.0		13.0	31.0		13.0	31.0	
Total Split (%)	13.3%	37.8%		13.3%	37.8%		14.4%	34.4%		14.4%	34.4%	
Yellow Time (s)	3.0	4.0		3.0	4.0		3.0	4.0		3.0	4.0	
All-Red Time (s)	0.5	2.0		0.5	2.0		0.5	2.0		0.5	2.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	3.5	6.0		3.5	6.0		3.5	6.0		3.5	6.0	
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag		Lead	Lag	
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes		Yes	Yes	
Recall Mode	None	None		None	None		None	C-Max		None	C-Max	
				23.8	13.3		51.5	43.3		53.3	44.2	
Act Effct Green (s)	25.1	15.8		23.0	13.3		51.5	43.5		55.5	44.Z	

24-093 - Glenbard East High School - Lombard Existing Weekday Afternoon Peak Hour

Synchro 11 Report

Lanes, Volumes, Timings 1: Main Street & Wilson Avenue

	٦	-	\mathbf{F}	∢	-	*	1	Ť	1	1	ţ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
v/c Ratio	0.53	0.61		0.34	0.53		0.18	0.56		0.29	0.56	
Control Delay	29.1	22.3		24.7	31.1		11.8	23.2		10.6	19.2	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	29.1	22.3		24.7	31.1		11.8	23.2		10.6	19.2	
LOS	С	С		С	С		В	С		В	В	
Approach Delay		25.3			28.5			22.5			18.4	
Approach LOS		С			С			С			В	
Queue Length 50th (ft)	82	55		44	58		11	188		21	193	
Queue Length 95th (ft)	59	69		55	56		40	314		34	234	
Internal Link Dist (ft)		211			339			167			436	
Turn Bay Length (ft)	100			165			150			110		
Base Capacity (vph)	352	591		316	558		392	1662		375	1715	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.53	0.40		0.33	0.27		0.15	0.56		0.27	0.56	
Intersection Summary												
Area Type:	Other											
Cycle Length: 90												
Actuated Cycle Length: 90												
Offset: 49 (54%), Reference	ed to phase	2:NBTL a	and 6:SB	TL, Start o	of Green							
Natural Cycle: 80												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.61												
Intersection Signal Delay: 2	21.9			In	tersectior	n LOS: C						
Intersection Capacity Utiliza	ation 66.9%			IC	U Level o	of Service	С					
Analysis Period (min) 15												

Splits and Phases: 1: Main Street & Wilson Avenue

Ø1	🚽 🔨 Ø2 (R)	√ Ø3	<u>⊿_</u> 94	
13 s	31 s	12 s	34 s	
▲ ø5	🚽 🕹 🖉 6 (R)		₹_Ø8	
13 s	31 s	12 s	34 s	

	≯	$\mathbf{\hat{v}}$	1	Ť	Ļ	~		
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø3	
Lane Configurations	ሻ	1	5	† †				
Traffic Volume (vph)	30	84	23	804	777	51		
Future Volume (vph)	30	84	23	804	777	51		
Ideal Flow (vphpl)	1900	1900	1900	2000	1900	1900		
Lane Width (ft)	12	12	11	11	11	11		
Grade (%)	0%			0%	0%			
Storage Length (ft)	60	0	95			0		
Storage Lanes	1	1	1			0		
Taper Length (ft)	55		110					
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	0.95		
Ped Bike Factor	0.98				0.99			
Frt		0.850			0.989			
Flt Protected	0.950		0.950					
Satd. Flow (prot)	1805	1583	1601	3566	3366	0		
Flt Permitted	0.950		0.221					
Satd. Flow (perm)	1776	1583	372	3566	3366	0		
Right Turn on Red		Yes				Yes		
Satd. Flow (RTOR)		131			9			
Link Speed (mph)	15			35	35			
Link Distance (ft)	509			375	98			
Travel Time (s)	23.1			7.3	1.9			
Confl. Peds. (#/hr)	6		25			25		
Confl. Bikes (#/hr)								
Peak Hour Factor	0.36	0.64	0.52	0.85	0.79	0.64		
Growth Factor	100%	100%	100%	100%	100%	100%		
Heavy Vehicles (%)	0%	2%	9%	3%	2%	2%		
Bus Blockages (#/hr)	0	0	0	0	0	0		
Parking (#/hr)								
Mid-Block Traffic (%)	0%			0%	0%			
Shared Lane Traffic (%)								
Lane Group Flow (vph)	83	131	44	946	1064	0		
Turn Type	Prot	pm+ov	pm+pt	NA	NA			
Protected Phases	4	5	5	2	6		3	
Permitted Phases		4	2					
Detector Phase	4	5	5	2	6			
Switch Phase	· ·	-	-		-			
Minimum Initial (s)	8.0	3.0	3.0	15.0	15.0		5.0	
Minimum Split (s)	16.0	6.5	6.5	26.0	29.0		32.0	
Total Split (s)	16.0	13.0	13.0	42.0	29.0		32.0	
Total Split (%)	17.8%	14.4%	14.4%	46.7%	32.2%		36%	
Yellow Time (s)	4.5	3.0	3.0	4.5	4.5		4.0	
All-Red Time (s)	1.5	0.0	0.0	1.5	1.5		0.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		0.0	
Total Lost Time (s)	6.0	3.0	3.0	6.0	6.0			
Lead/Lag	Lag	Lead	Lead	0.0	Lag		Lead	
Lead-Lag Optimize?	Yes	Yes	Yes		Yes		Yes	
Recall Mode	None	None	None	C-Max	C-Max		None	
Act Effct Green (s)	11.4	20.7	72.4	70.6	60.3			
Actuated g/C Ratio	0.13	0.23	0.80	0.78	0.67			
	0.13	0.20	0.00	0.70	0.07			

24-093 - Glenbard East High School - Lombard Existing Weekday Afternoon Peak Hour

Synchro 11 Report

Lanes, Volumes, Timings 2: Main Street & Signalized Access Drive

	≯	\mathbf{r}	1	1	Ļ	1	
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø3
v/c Ratio	0.36	0.28	0.12	0.34	0.47		
Control Delay	39.8	6.2	3.5	4.4	9.1		
Queue Delay	0.0	0.0	0.0	0.0	0.0		
Total Delay	39.8	6.2	3.5	4.4	9.1		
LOS	D	А	А	А	А		
Approach Delay	19.2			4.3	9.1		
Approach LOS	В			А	А		
Queue Length 50th (ft)	44	0	5	82	230		
Queue Length 95th (ft)	32	12	8	117	189		
Internal Link Dist (ft)	429			295	18		
Turn Bay Length (ft)	60		95				
Base Capacity (vph)	236	527	436	2798	2258		
Starvation Cap Reductn	0	0	0	0	0		
Spillback Cap Reductn	0	0	0	0	0		
Storage Cap Reductn	0	0	0	0	0		
Reduced v/c Ratio	0.35	0.25	0.10	0.34	0.47		
Intersection Summary							
51	Other						
Cycle Length: 90							
Actuated Cycle Length: 90							
Offset: 5 (6%), Referenced	to phase 2:I	NBTL and	16:SBT, S	Start of G	reen		
Natural Cycle: 85							
Control Type: Actuated-Coc	ordinated						
Maximum v/c Ratio: 0.47							
Intersection Signal Delay: 8					tersection		
Intersection Capacity Utiliza	ation 39.9%			IC	U Level c	f Service	A
Analysis Period (min) 15							

Splits and Phases: 2: Main Street & Signalized Access Drive

1 Ø2 (R)	•		Ø4
42 s		32 s	16 s
🗙 ø5	🛡 🕇 Ø6 (R)		
13 s	29 s		

04/29/2024

Intersection Int Delay, s/veh 0.2 EBL EBR NBL SBT SBR Movement NBT **`** 9 Lane Configurations ۴ ħħ **↑**↑ 824 Traffic Vol, veh/h 4 0 0 834 Future Vol, veh/h 9 4 0 834 824 0 Conflicting Peds, #/hr 0 0 7 0 0 7 Sign Control Stop Stop Free Free Free Free RT Channelized None None None ---Storage Length 0 0 _ ---Veh in Median Storage, # 0 0 0 ---Grade, % 0 0 0 ---Peak Hour Factor 50 50 85 79 100 100 Heavy Vehicles, % 89 50 0 3 2 0 Mvmt Flow 18 8 0 981 1043 0

Major/Minor	Minor2	Μ	lajor1	N	lajor2				
Conflicting Flow All	1534	522	-	0	-	0			
Stage 1	1043	-	-	-	-	-			
Stage 2	491	-	-	-	-	-			
Critical Hdwy	8.58	7.9	-	-	-	-			
Critical Hdwy Stg 1	7.58	-	-	-	-	-			
Critical Hdwy Stg 2	7.58	-	-	-	-	-			
Follow-up Hdwy	4.39	3.8	-	-	-	-			
Pot Cap-1 Maneuver	*295	*670	0	-	-	0			
Stage 1	*406	-	0	-	-	0			
Stage 2	*632	-	0	-	-	0			
Platoon blocked, %	1	1		-	-				
Nov Cap-1 Maneuver		*670	-	-	-	-			
Nov Cap-2 Maneuver		-	-	-	-	-			
Stage 1	*406	-	-	-	-	-			
Stage 2	*632	-	-	-	-	-			
Approach	EB		NB		SB				
HCM Control Delay, s	5 15.7		0		0				
ICM LOS	С								
linor Lane/Major Mv	mt	NBT E	BLn1 E	BLn2	SBT				
Capacity (veh/h)		-	295	670	-				
ICM Lane V/C Ratio		- (0.012	-				
HCM Control Delay (s	s)	-	18	10.4	-				
ICM Lane LOS		-	С	В	-				
ICM 95th %tile Q(vel	h)	-	0.2	0	-				
Notes									
·· Volume exceeds c:	anacity	\$∙ Dol	av exce	ode 20	Λs	+: Comp	utation Not Defi	ned *	· All major volume in platoon

~: Volume exceeds capacity

\$: Delay exceeds 300s +: Computation Not Defined *: All major volume in platoon

0

16

0

926

1025

Intersection Int Delay, s/veh 0.1 Movement EBL EBR NBL NBT SBT SBR **↑↑** 820 Y Lane Configurations ħħ Traffic Vol, veh/h 0 4 843 0 0 Future Vol, veh/h 0 4 0 843 820 0 Conflicting Peds, #/hr 0 0 1 0 0 1 Sign Control Stop Stop Free Free Free Free RT Channelized None -None -None -Storage Length 0 -----Veh in Median Storage, # 0 -0 0 --Grade, % 0 0 0 ---Peak Hour Factor 100 25 100 91 80 100 Heavy Vehicles, % 0 0 0 3 2 0

0

Major/Minor	Minor2	Μ	lajor1	Ma	ajor2	
Conflicting Flow All	1488	513	-	0	-	0
Stage 1	1025	-	-	-	-	-
Stage 2	463	-	-	-	-	-
Critical Hdwy	6.8	6.9	-	-	-	-
Critical Hdwy Stg 1	5.8	-	-	-	-	-
Critical Hdwy Stg 2	5.8	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	-	-
Pot Cap-1 Maneuver	117	512	0	-	-	0
Stage 1	312	-	0	-	-	0
Stage 2	606	-	0	-	-	0
Platoon blocked, %				-	-	
Mov Cap-1 Maneuver		512	-	-	-	-
Mov Cap-2 Maneuver		-	-	-	-	-
Stage 1	312	-	-	-	-	-
Stage 2	606	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	5 12.3		0		0	
HCM LOS	В					

Minor Lane/Major Mvmt	NBT EBLn1	SBT	
Capacity (veh/h)	- 512	-	
HCM Lane V/C Ratio	- 0.031	-	
HCM Control Delay (s)	- 12.3	-	
HCM Lane LOS	- B	-	
HCM 95th %tile Q(veh)	- 0.1	-	

Mvmt Flow

Intersection

Int Delay, s/veh

0.3

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations			1		\$			ħ ₽			-4 ↑		
Traffic Vol, veh/h	0	0	11	0	0	8	0	842	0	0	809	0	
Future Vol, veh/h	0	0	11	0	0	8	0	842	0	0	809	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	5	0	3	3	0	5	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	-	-	0	-	-	-	-	-	-	-	-	-	
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	100	100	31	100	100	40	100	87	100	100	78	50	
Heavy Vehicles, %	0	0	0	0	0	0	0	4	0	0	2	0	
Mvmt Flow	0	0	35	0	0	20	0	968	0	0	1037	0	

Major/Minor	Minor2		Ν	/linor1		N	lajor1		N	lajor2			
Conflicting Flow All	-	-	519	1490	2008	487	-	0	0	971	0	0	
Stage 1	-	-	-	971	971	-	-	-	-	-	-	-	
Stage 2	-	-	-	519	1037	-	-	-	-	-	-	-	
Critical Hdwy	-	-	6.9	7.5	6.5	6.9	-	-	-	4.1	-	-	
Critical Hdwy Stg 1	-	-	-	6.5	5.5	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	-	-	-	6.5	5.5	-	-	-	-	-	-	-	
Follow-up Hdwy	-	-	3.3	3.5	4	3.3	-	-	-	2.2	-	-	
Pot Cap-1 Maneuver	0	0	507	87	60	532	0	-	-	718	-	0	
Stage 1	0	0	-	275	334	-	0	-	-	-	-	0	
Stage 2	0	0	-	513	311	-	0	-	-	-	-	0	
Platoon blocked, %								-	-		-		
Mov Cap-1 Maneuver		-	507	81	60	530	-	-	-	716	-	-	
Mov Cap-2 Maneuver	-	-	-	81	60	-	-	-	-	-	-	-	
Stage 1	-	-	-	275	333	-	-	-	-	-	-	-	
Stage 2	-	-	-	477	311	-	-	-	-	-	-	-	
Approach	EB			WB			NB			SB			
HCM Control Delay, s	12.6			12.1			0			0			
HCM LOS	В			В									

Minor Lane/Major Mvmt	NBT	NBR E	EBLn1V	VBLn1	SBL	SBT	
Capacity (veh/h)	-	-	507	530	716	-	
HCM Lane V/C Ratio	-	-	0.07	0.038	-	-	
HCM Control Delay (s)	-	-	12.6	12.1	0	-	
HCM Lane LOS	-	-	В	В	А	-	
HCM 95th %tile Q(veh)	-	-	0.2	0.1	0	-	

Intersection						
Int Delay, s/veh	3.5					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	•			•	۳	1
Traffic Vol, veh/h	169	0	0	143	41	60
Future Vol, veh/h	169	0	0	143	41	60
Conflicting Peds, #/hr	0	17	17	0	0	4
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	0
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	54	25	100	94	45	63
Heavy Vehicles, %	1	0	0	4	34	5
Mvmt Flow	313	0	0	152	91	95

Major/Minor M	ajor1	1	Major2	1	Vinor1	
Conflicting Flow All	0	-	-	-	465	317
Stage 1	-	-	-	-	313	-
Stage 2	-	-	-	-	152	-
Critical Hdwy	-	-	-	-	6.74	6.25
Critical Hdwy Stg 1	-	-	-	-	5.74	-
Critical Hdwy Stg 2	-	-	-	-	5.74	-
Follow-up Hdwy	-	-	-	-	3.806	
Pot Cap-1 Maneuver	-	0	0	-	501	717
Stage 1	-	0	0	-	674	-
Stage 2	-	0	0	-	804	-
Platoon blocked, %	-			-		
Mov Cap-1 Maneuver	-	-	-	-	501	714
Mov Cap-2 Maneuver	-	-	-	-	501	-
Stage 1	-	-	-	-	674	-
Stage 2	-	-	-	-	804	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		12.3	
HCM LOS					В	
Minor Lane/Major Mvmt	N	RI n1 I	VBLn2	EBT	WBT	
Capacity (veh/h)	11	501	714		VUDI	
HCM Lane V/C Ratio	(-	-	
HCM Control Delay (s)		13.8	10.8	-	-	
HCM Lane LOS		13.0 B	10.8 B	-	_	
HCM 95th %tile Q(veh)		0.7	0.5	-	-	
Intersection

Int Delay, s/veh

4.2

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4			4			4			4		
Traffic Vol, veh/h	19	124	33	28	141	16	33	1	18	10	3	12	
Future Vol, veh/h	19	124	33	28	141	16	33	1	18	10	3	12	
Conflicting Peds, #/hr	5	0	0	0	0	5	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized	-	-	None										
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-	
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	32	61	52	70	86	80	64	25	45	50	38	43	
Heavy Vehicles, %	0	1	3	0	11	19	9	0	0	0	33	0	
Mvmt Flow	59	203	63	40	164	20	52	4	40	20	8	28	

Major/Minor	Major1		Ν	Major2			Minor1		Ν	/linor2		
Conflicting Flow All	189	0	0	266	0	0	625	622	235	634	643	
Stage 1	-	-	-	-	-	-	353	353	-	259	259	
Stage 2	-	-	-	-	-	-	272	269	-	375	384	
Critical Hdwy	4.1	-	-	4.1	-	-	7.19	6.5	6.2	7.1	6.83	
Critical Hdwy Stg 1	-	-	-	-	-	-	6.19	5.5	-	6.1	5.83	
Critical Hdwy Stg 2	-	-	-	-	-	-	6.19	5.5	-	6.1	5.83	
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.581	4	3.3	3.5	4.297	3.3
Pot Cap-1 Maneuver	1397	-	-	1310	-	-	387	405	809	395	354	869
Stage 1	-	-	-	-	-	-	650	634	-	750	640	-
Stage 2	-	-	-	-	-	-	719	690	-	650	561	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1390	-	-	1310	-	-	344	370	809	347	323	865
Mov Cap-2 Maneuver	-	-	-	-	-	-	344	370	-	347	323	-
Stage 1	-	-	-	-	-	-	618	602	-	709	615	-
Stage 2	-	-	-	-	-	-	664	663	-	583	533	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	1.4			1.4			15			13.3		
HCM LOS							С			В		
Minor Lane/Major Mvm	nt I	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR 3	SBLn1			
Capacity (veh/h)		455	1390	-	-	1310	-	-	488			
HCM Lane V/C Ratio		0.21	0.043	-	-	01001	-	-	0.114			
HCM Control Delay (s)		15	7.7	0	-	7.8	0	-	13.3			

А

0.1

-

-

А

-

В

0.4

-

-

С

0.8

А

0.1

А

-

HCM Lane LOS

HCM 95th %tile Q(veh)

Intersection						
Int Delay, s/veh	4.5					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	4			- सी	۰¥	
Traffic Vol, veh/h	148	8	5	178	28	25
Future Vol, veh/h	148	8	5	178	28	25
Conflicting Peds, #/hr	0	53	53	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	63	29	31	79	26	27
Heavy Vehicles, %	1	0	0	10	0	0
Mvmt Flow	235	28	16	225	108	93

Major/Minor M	lajor1	Ν	Najor2	ſ	Minor1	
Conflicting Flow All	0	0	316	0	559	302
Stage 1	-	-	-	-	302	-
Stage 2	-	-	-	-	257	-
Critical Hdwy	-	-	4.1	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	-	1256	-	494	742
Stage 1	-	-	-	-	755	-
Stage 2	-	-	-	-	791	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1193	-	462	705
Mov Cap-2 Maneuver	-	-	-	-	462	-
Stage 1	-	-	-	-	717	-
Stage 2	-	-	-	-	779	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.5		15.2	
HCM LOS					С	
Minor Long/Major Mumat			ГРТ			
Minor Lane/Major Mvmt		NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		550	-	-	1193	-
HCM Lane V/C Ratio		0.364	-		0.014	-
HCM Control Delay (s)		15.2	-	-	8.1	0
HCM Lane LOS		C	-	-	A	А
HCM 95th %tile Q(veh)		1.7	-	-	0	-

Intersection

Int Delay, s/veh	2.2						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		÷	et e		Y		
Traffic Vol, veh/h	8	105	170	33	29	10	
Future Vol, veh/h	8	105	170	33	29	10	
Conflicting Peds, #/hr	2	0	0	2	17	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	-	-	-	-	0	-	
Veh in Median Storage,	# -	0	0	-	0	-	
Grade, %	-	0	0	-	0	-	
Peak Hour Factor	50	73	82	64	45	50	
Heavy Vehicles, %	0	1	8	15	3	0	
Mvmt Flow	16	144	207	52	64	20	

Major/Minor	Major1	Ν	/lajor2	ľ	Minor2							
Conflicting Flow All	261	0	-	0	428	235						
Stage 1	-	-	-	-	235	-						
Stage 2	-	-	-	-	193	-						
Critical Hdwy	4.1	-	-	-	6.43	6.2						
Critical Hdwy Stg 1	-	-	-	-	5.43	-						
Critical Hdwy Stg 2	-	-	-	-	5.43	-						
Follow-up Hdwy	2.2	-	-	-	3.527	3.3						
Pot Cap-1 Maneuver	1315	-	-	-	582	809						
Stage 1	-	-	-	-	802	-						
Stage 2	-	-	-	-	837	-						
Platoon blocked, %		-	-	-								
Mov Cap-1 Maneuver		-	-	-	572	807						
Mov Cap-2 Maneuver	r -	-	-	-	572	-						
Stage 1	-	-	-	-	790	-						
Stage 2	-	-	-	-	835	-						
Approach	EB		WB		SB							
HCM Control Delay, s	6 0.8		0		11.8							
HCM LOS					В							
Minor Lane/Major Mv	mt	EBL	EBT	WBT	WBR S	SBLn1		 	ļ			
Capacity (veh/h)		1312	-	-	-	614						
HCM Lane V/C Ratio		0.012	-	-	-	0.138						
HCM Control Delay (s	s)	7.8	0	-	-	11.8						
HCM Lane LOS		А	А	-	-	В						
HCM 95th %tile Q(vel	h)	0	-	-	-	0.5						

<u>Capacity Analysis Summary Reports</u> Weekday Morning Peak Hour – Projected Conditions

01/22/20	25
----------	----

	٦	+	\mathbf{F}	4	+	•	1	1	1	1	ţ	-∢
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	4Î		ሻ	4Î		5	¥î≽		5	≜ †⊅	
Traffic Volume (vph)	50	13	170	117	25	32	140	514	44	42	639	37
Future Volume (vph)	50	13	170	117	25	32	140	514	44	42	639	37
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	100		0	165		0	150		0	110		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	100		Ū	200		Ū	110		Ū	90		Ū
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Ped Bike Factor	1.00	0.97		0.99	0.99		1.00	1.00	0.00	1.00	1.00	0.00
Frt	1.00	0.859		0.00	0.920		1.00	0.985		1.00	0.990	
Flt Protected	0.950	0.000		0.950	0.020		0.950	0.000		0.950	0.000	
Satd. Flow (prot)	1671	1512	0	1719	1659	0	1752	3440	0	1770	3463	0
Flt Permitted	0.704	1012	U	0.294	1000	0	0.185	0110	0	0.318	0400	U
Satd. Flow (perm)	1237	1512	0	527	1659	0	341	3440	0	592	3463	0
Right Turn on Red	1201	1012	Yes	521	1000	Yes	170	0440	Yes	002	0400	Yes
Satd. Flow (RTOR)		254	163		44	163		13	163		8	163
Link Speed (mph)		25			30			35			35	
Link Distance (ft)		291			419			247			516	
Travel Time (s)		7.9			9.5			4.8			10.1	
Confl. Peds. (#/hr)	1	1.9	14	14	9.0	1	4	4.0	3	3	10.1	4
()	I		14	14		I	4		1	3		4
Confl. Bikes (#/hr)	0.54	0.78	0.67	0.64	0.66	0.73	0.62	0.65	0.50	0.88	0.70	0.56
Peak Hour Factor								0.65	0.50 4%			
Heavy Vehicles (%)	8%	7%	5%	5%	3%	6%	3%	3%	4 %	2%	3%	3%
Shared Lane Traffic (%)	93	074	0	183	82	0	000	879	0	48	979	0
Lane Group Flow (vph)		271 NA	0			0	226		0			0
Turn Type Protected Phases	pm+pt			pm+pt	NA		pm+pt	NA		pm+pt	NA	
Protected Phases Permitted Phases	7	4		3	8		5 2	2		1	6	
		4		8 3	8		2 5	2		6 1	6	
Detector Phase Switch Phase	7	4		3	ð		5	2		1	0	
	2.0	0.0		2.0	0.0		2.0	45.0		2.0	45.0	
Minimum Initial (s)	3.0	8.0		3.0	8.0		3.0	15.0		3.0	15.0	
Minimum Split (s)	6.5	33.0		6.5	33.0		6.5	39.0		6.5	38.0	
Total Split (s)	13.0	34.0		13.0	34.0		15.0	40.0		13.0	38.0	
Total Split (%)	13.0%	34.0%		13.0%	34.0%		15.0%	40.0%		13.0%	38.0%	
Yellow Time (s)	3.0	4.0		3.0	4.0		3.0	4.0		3.0	4.0	
All-Red Time (s)	0.5	2.0		0.5	2.0		0.5	2.0		0.5	2.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	3.5	6.0		3.5	6.0		3.5	6.0		3.5	6.0	
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag		Lead	Lag	
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes		Yes	Yes	
Recall Mode	None	None		None	None		None	C-Max		None	C-Max	
Act Effct Green (s)	22.0	10.8		23.7	13.6		66.7	57.9		57.0	48.0	
Actuated g/C Ratio	0.22	0.11		0.24	0.14		0.67	0.58		0.57	0.48	
v/c Ratio	0.30	0.69		0.77	0.31		0.56	0.44		0.12	0.59	
Control Delay	30.4	16.5		52.8	24.2		16.0	11.0		8.1	22.1	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	30.4	16.5		52.8	24.2		16.0	11.0		8.1	22.1	
LOS	С	В		D	С		В	В		А	С	
Approach Delay		20.1			43.9			12.0			21.5	

PR AM 24-093 - Glenbard East High School - Lombard 6:45 am 04/04/2024 Projected Weekday Morning Peak Hour Synchro 11 Report BSM Page 1

01/22/2025)
------------	---

	٦	+	*	4	Ļ	•	•	1	*	1	ţ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Approach LOS		С			D			В			С	
Queue Length 50th (ft)	48	10		99	23		39	141		9	211	
Queue Length 95th (ft)	46	47		97	38		51	136		26	260	
Internal Link Dist (ft)		211			339			167			436	
Turn Bay Length (ft)	100			165			150			110		
Base Capacity (vph)	323	606		238	496		419	1998		467	1665	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.29	0.45		0.77	0.17		0.54	0.44		0.10	0.59	
Intersection Summary												
Area Type:	Other											
Cycle Length: 100												
Actuated Cycle Length: 10	00											
Offset: 49 (49%), Referen	ced to phase	2:NBTL a	and 6:SBT	L, Start	of Green							
Natural Cycle: 85												
Control Type: Actuated-Co	oordinated											
Maximum v/c Ratio: 0.77												
	10.0											

Intersection Signal Delay: 19.6	Intersection LOS: B	
Intersection Capacity Utilization 67.7%	ICU Level of Service C	
Analysis Period (min) 15		

Splits and Phases: 1: Main Street & Wilson Avenue

Ø1	1	Ø2 (R)	√ Ø3	▲ ₀₄
13 s	40 s	S	13 s	34 s
▲ ø5		Ø6 (R)	▶ Ø7	₩ Ø8
15 s		38 s	13 s	34 s

	۶	$\mathbf{\hat{z}}$	1	t	ţ	~			
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø3		
Lane Configurations	5	1	5	††	≜ †₽	-			
Traffic Volume (vph)	209	170	106	494	594	43			
Future Volume (vph)	209	170	106	494	594	43			
Ideal Flow (vphpl)	1900	1900	1900	2000	1900	1900			
Lane Width (ft)	12	12	11	11	11	11			
Storage Length (ft)	60	0	95			0			
Storage Lanes	1	1	1			0			
Taper Length (ft)	55		110			Ū			
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	0.95			
Ped Bike Factor	0.99	1.00	1.00	0.00	1.00	0.00			
Frt	0.00	0.850			0.984				
Flt Protected	0.950	0.000	0.950		0.304				
Satd. Flow (prot)	1687	1553	1711	3601	3329	0			
Flt Permitted	0.950	1555	0.115	3001	5525	0			
Satd. Flow (perm)	1667	1553	207	3601	3329	0			
Right Turn on Red	1007	Yes	207	3001	JJZ9	Yes			
					10	165			
Satd. Flow (RTOR)	15	425		35	13 35				
Link Speed (mph)	509			35 375	35 98				
Link Distance (ft)									
Travel Time (s)	23.1		44	7.3	1.9	44			
Confl. Peds. (#/hr)	4	0.40	11	0 70	0.00	11			
Peak Hour Factor	0.44	0.40	0.54	0.73	0.69	0.43			
Heavy Vehicles (%)	7%	4%	2%	2%	3%	0%			
Shared Lane Traffic (%)	475	405	400	077	004	0			
Lane Group Flow (vph)	475	425	196	677	961	0			
Turn Type	Prot	pm+ov	pm+pt	NA	NA		0		
Protected Phases	4	5	5	2	6		3		
Permitted Phases		4	2	<u> </u>	<u> </u>				
Detector Phase	4	5	5	2	6				
Switch Phase				45.0	45.0				
Minimum Initial (s)	8.0	3.0	3.0	15.0	15.0		8.0		
Minimum Split (s)	16.0	6.5	6.5	26.0	29.0		32.0		
Total Split (s)	16.0	15.0	15.0	52.0	37.0		32.0		
Total Split (%)	16.0%	15.0%	15.0%	52.0%	37.0%		32%		
Yellow Time (s)	4.5	3.0	3.0	4.5	4.5		4.0		
All-Red Time (s)	1.5	0.0	0.0	1.5	1.5		0.0		
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0				
Total Lost Time (s)	6.0	3.0	3.0	6.0	6.0				
Lead/Lag	Lag	Lead	Lead		Lag		Lead		
Lead-Lag Optimize?	Yes	Yes	Yes		Yes		Yes		
Recall Mode	None	None	None	C-Max	C-Max		None		
Act Effct Green (s)	42.0	59.1	49.0	46.0	31.9				
Actuated g/C Ratio	0.42	0.59	0.49	0.46	0.32				
v/c Ratio	0.67	0.39	0.73	0.41	0.90				
Control Delay	29.2	2.0	35.5	18.9	39.8				
Queue Delay	0.0	0.0	0.0	0.0	0.0				
Total Delay	29.2	2.0	35.5	18.9	39.8				
LOS	С	А	D	В	D				
Approach Delay	16.3			22.6	39.8				

PR AM 24-093 - Glenbard East High School - Lombard 6:45 am 04/04/2024 Projected Weekday Morning Peak Hour Synchro 11 Report BSM Page 3

01	22	20	25
----	----	----	----

	٦	\mathbf{r}	1	t	Ļ	∢_	
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø3
Approach LOS	В			С	D		
Queue Length 50th (ft)	239	0	69	146	312		
Queue Length 95th (ft)	134	0	63	147	219		
Internal Link Dist (ft)	429			295	18		
Turn Bay Length (ft)	60		95				
Base Capacity (vph)	708	1101	281	1656	1071		
Starvation Cap Reductn	0	0	0	0	0		
Spillback Cap Reductn	0	0	0	0	0		
Storage Cap Reductn	0	0	0	0	0		
Reduced v/c Ratio	0.67	0.39	0.70	0.41	0.90		
Intersection Summary							
Area Type:	Other						
Cycle Length: 100							
Actuated Cycle Length: 10							
Offset: 47 (47%), Referen	ced to phase	2:NBTL a	and 6:SB	T, Start of	Green		
Natural Cycle: 135							
Control Type: Actuated-Co	pordinated						
Maximum v/c Ratio: 0.90							

Maximum v/c Ratio: 0.90		
Intersection Signal Delay: 26.6	Intersection LOS: C	
Intersection Capacity Utilization 50.0%	ICU Level of Service A	
Analysis Period (min) 15		

Splits and Phases: 2: Main Street & Signalized Access Drive



<u>Capacity Analysis Summary Reports</u> Weekday Afternoon Peak Hour – Projected Conditions

01/22/20	25
----------	----

	٨	+	\mathbf{F}	4	+	•	1	1	1	1	ţ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ኘ	4Î		5	eî 👘		۲	A⊅		5	A	
Traffic Volume (vph)	88	42	105	84	39	57	68	743	64	62	666	33
Future Volume (vph)	88	42	105	84	39	57	68	743	64	62	666	33
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	100		0	165		0	150		0	110		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	100			200			110			90		-
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Ped Bike Factor	1.00	0.95		0.97	0.99		1.00	1.00		1.00	1.00	
Frt		0.886			0.927			0.985			0.992	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1582	0	1787	1682	0	1805	3439	0	1719	3495	0
Flt Permitted	0.647			0.331			0.225			0.218		-
Satd. Flow (perm)	1203	1582	0	603	1682	0	426	3439	0	394	3495	0
Right Turn on Red			Yes			Yes		• • • • •	Yes			Yes
Satd. Flow (RTOR)		182			55			13			6	
Link Speed (mph)		25			30			35			35	
Link Distance (ft)		291			419			247			516	
Travel Time (s)		7.9			9.5			4.8			10.1	
Confl. Peds. (#/hr)	2	1.0	48	48	0.0	2	9		6	6	10.1	9
Peak Hour Factor	0.48	0.69	0.55	0.70	0.57	0.89	0.85	0.87	0.67	0.62	0.74	0.66
Heavy Vehicles (%)	2%	0%	2%	1%	4%	4%	0%	3%	4%	5%	2%	7%
Shared Lane Traffic (%)			270									
Lane Group Flow (vph)	183	252	0	120	132	0	80	950	0	100	950	0
Turn Type	pm+pt	NA		pm+pt	NA		pm+pt	NA		pm+pt	NA	
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4			8			2			6		
Detector Phase	7	4		3	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	3.0	8.0		3.0	8.0		3.0	15.0		3.0	15.0	
Minimum Split (s)	6.5	33.0		6.5	33.0		6.5	31.0		6.5	31.0	
Total Split (s)	12.0	34.0		12.0	34.0		13.0	31.0		13.0	31.0	
Total Split (%)	13.3%	37.8%		13.3%	37.8%		14.4%	34.4%		14.4%	34.4%	
Yellow Time (s)	3.0	4.0		3.0	4.0		3.0	4.0		3.0	4.0	
All-Red Time (s)	0.5	2.0		0.5	2.0		0.5	2.0		0.5	2.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	3.5	6.0		3.5	6.0		3.5	6.0		3.5	6.0	
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag		Lead	Lag	
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes		Yes	Yes	
Recall Mode	None	None		None	None		None	C-Max		None	C-Max	
Act Effct Green (s)	23.3	12.3		22.8	12.1		53.2	44.6		54.2	45.1	
Actuated g/C Ratio	0.26	0.14		0.25	0.13		0.59	0.50		0.60	0.50	
v/c Ratio	0.50	0.67		0.46	0.49		0.22	0.56		0.29	0.54	
Control Delay	29.4	20.5		28.8	26.6		15.7	28.6		9.9	18.5	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	29.4	20.5		28.8	26.6		15.7	28.6		9.9	18.5	
LOS	С	С		С	С		В	С		А	В	
Approach Delay		24.3			27.7			27.6			17.7	
Approach LOS		С			С			С			В	

PR PM 24-093 - Glenbard East High School - Lombard 2:30 pm 04/04/2024 Projected Weekday Afternoon Peak Hour Synchro 11 Report BSM Page 1

01/22/2025

	۶	-	\mathbf{r}	1	←	•	1	Ť	۲	1	ţ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 50th (ft)	83	37		52	41		22	250		19	185	
Queue Length 95th (ft)	60	51		64	43		60	354		32	231	
Internal Link Dist (ft)		211			339			167			436	
Turn Bay Length (ft)	100			165			150			110		
Base Capacity (vph)	365	617		266	561		404	1711		384	1755	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.50	0.41		0.45	0.24		0.20	0.56		0.26	0.54	
Intersection Summary												
Area Type: C	Other											
Cycle Length: 90												
Actuated Cycle Length: 90												
Offset: 49 (54%), Referenced	d to phase	2:NBTL a	and 6:SB	FL, Start o	of Green							
Natural Cycle: 80												
Control Type: Actuated-Coor	dinated											
Maximum v/c Ratio: 0.67												
Intersection Signal Delay: 23	.3			In	tersectior	n LOS: C						
Intersection Capacity Utilizat	ion 67.7%			IC	U Level o	of Service	С					
Analysis Period (min) 15												

Splits and Phases: 1: Main Street & Wilson Avenue

Ø1	Ø2 (R)	√ Ø3	<u></u> 04
13 s	31 s	12 s	34 s
▲ ø5	Ø6 (R)		↓ Ø8
13 s	31 s	12 s	34 s

	٦	$\mathbf{\hat{v}}$	1	Ť	Ļ	~		
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø3	
Lane Configurations	ሻ	1	ሻ	† †	∱ ⊅			
Traffic Volume (vph)	65	112	23	804	751	51		
Future Volume (vph)	65	112	23	804	751	51		
Ideal Flow (vphpl)	1900	1900	1900	2000	1900	1900		
Lane Width (ft)	12	12	11	11	11	11		
Storage Length (ft)	60	0	95			0		
Storage Lanes	1	1	1			0		
Taper Length (ft)	55	•	110			Ŭ		
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	0.95		
Ped Bike Factor	0.98	1.00	1.00	0.00	0.99	0.00		
Frt	0.00	0.850			0.988			
Flt Protected	0.950	0.000	0.950		0.000			
Satd. Flow (prot)	1805	1583	1601	3566	3362	0		
Flt Permitted	0.950	1000	0.208	0000	0002	U		
Satd. Flow (perm)	1776	1583	350	3566	3362	0		
Right Turn on Red	1770	Yes	550	5500	JJ02	Yes		
Satd. Flow (RTOR)		175			9	163		
Link Speed (mph)	15	175		35	35			
Link Distance (ft)	509			375	98			
Travel Time (s)	23.1			7.3	90 1.9			
Confl. Peds. (#/hr)	23.1		25	1.5	1.9	25		
Peak Hour Factor	0.36	0.64	0.52	0.85	0.79	0.64		
Heavy Vehicles (%)	0.30	2%	9%	3%	2%	2%		
	0 %	Ζ70	9%	370	Ζ 70	Ζ 70		
Shared Lane Traffic (%)	181	175	44	946	1031	0		
Lane Group Flow (vph)					NA	0		
Turn Type	Prot	pm+ov	pm+pt	NA			2	
Protected Phases	4	5	5	2	6		3	
Permitted Phases	4	4	2	0	6			
Detector Phase	4	5	5	2	6			
Switch Phase	0.0	2.0	2.0	45.0	45.0		F 0	
Minimum Initial (s)	8.0	3.0	3.0	15.0	15.0		5.0	
Minimum Split (s)	16.0	6.5	6.5	26.0	29.0		32.0	
Total Split (s)	16.0	13.0	13.0	42.0	29.0		32.0	
Total Split (%)	17.8%	14.4%	14.4%	46.7%	32.2%		36%	
Yellow Time (s)	4.5	3.0	3.0	4.5	4.5		4.0	
All-Red Time (s)	1.5	0.0	0.0	1.5	1.5		0.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0			
Total Lost Time (s)	6.0	3.0	3.0	6.0	6.0			
Lead/Lag	Lag	Lead	Lead		Lag		Lead	
Lead-Lag Optimize?	Yes	Yes	Yes		Yes		Yes	
Recall Mode	None	None	None	C-Max	C-Max		None	
Act Effct Green (s)	17.4	29.7	63.6	60.6	51.3			
Actuated g/C Ratio	0.19	0.33	0.71	0.67	0.57			
v/c Ratio	0.52	0.27	0.13	0.39	0.54			
Control Delay	37.7	4.3	5.5	7.4	15.6			
Queue Delay	0.0	0.0	0.0	0.0	0.0			
Total Delay	37.7	4.3	5.5	7.4	15.6			
LOS	D	А	А	А	В			
Approach Delay	21.3			7.4	15.6			

PR PM 24-093 - Glenbard East High School - Lombard 2:30 pm 04/04/2024 Projected Weekday Afternoon Peak Hour Synchro 11 Report BSM Page 3

	٦	\mathbf{i}	•	Ť	Ļ	1	
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø3
Approach LOS	C	LDIX	NDL	A	B	ODIX	00
Queue Length 50th (ft)	92	0	7	112	252		
Queue Length 95th (ft)	55	9	10	151	286		
Internal Link Dist (ft)	429			295	18		
Turn Bay Length (ft)	60		95				
Base Capacity (vph)	348	697	386	2401	1919		
Starvation Cap Reductn	0	0	0	0	0		
Spillback Cap Reductn	0	0	0	0	0		
Storage Cap Reductn	0	0	0	0	0		
Reduced v/c Ratio	0.52	0.25	0.11	0.39	0.54		
Intersection Summary							
Area Type:	Other						
Cycle Length: 90							
Actuated Cycle Length: 9							
Offset: 5 (6%). Reference	ed to phase 2:	NBTL and	16:SBT. 3	Start of G	reen		

Offset: 5 (6%), Referenced to phase 2:NBTL and 6:SBT, Sta	int of Green
Natural Cycle: 85	
Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 0.54	
Intersection Signal Delay: 13.0	Intersection LOS: B
Intersection Capacity Utilization 39.2%	ICU Level of Service A
Analysis Period (min) 15	

Splits and Phases: 2: Main Street & Signalized Access Drive

